

DEM, dams and dikes

Catherine O'Sullivan

DEM8 recognises dikes

DEM8

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DEM 8

8th International Conference
on Discrete Element Methods

21st - 26th July 2019

The Netherlands



The boy with his finger in the dike



Kinderdijk



742 years old

To drain the polder, a system of 19 windmills was built around 1740

UNESCO world heritage site

Kinderdijk



Dutch Dikes

Total land area Netherlands: 41,528 km²

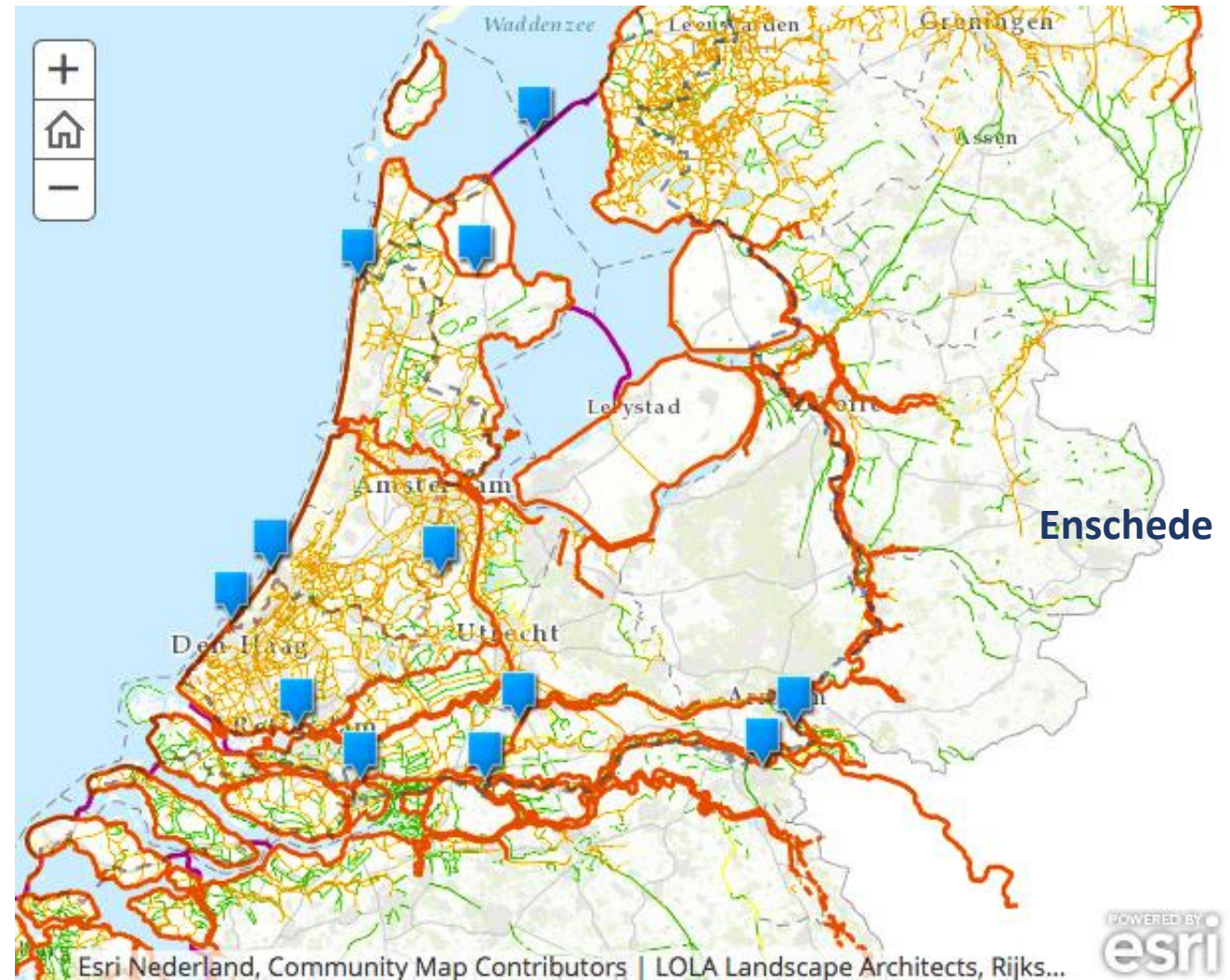
26% below mean sea level (NAP)

66% of the area is flood prone

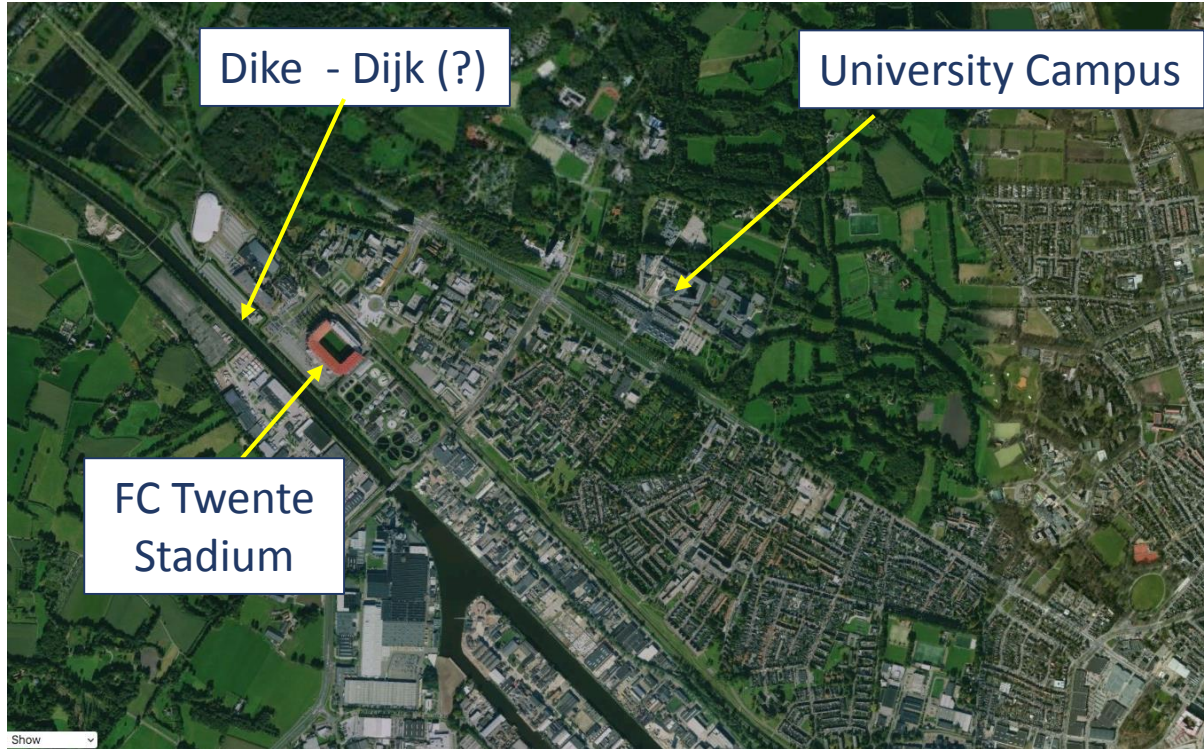
9 million people live in these low areas

70% of GNP is earned in flood-vulnerable area

The Dutch dike network extends for over 22,000 kilometres



Dikes near here



Strengthening of Twente Canal (Twentekanaal)

The International Levee Handbook - 2013

The International Levee Handbook



- Joint research project of CIRIA (UK), French Ministry for Ecology and US Army Corps of Engineers
- Funding from France, UK, USA, Ireland and the Netherlands



**US Army Corps
of Engineers®**

International perspective: US

30,000 documented miles of levee in the US



Breach in 17th Street Canal levee in New Orleans, Louisiana, on August 31, 2005

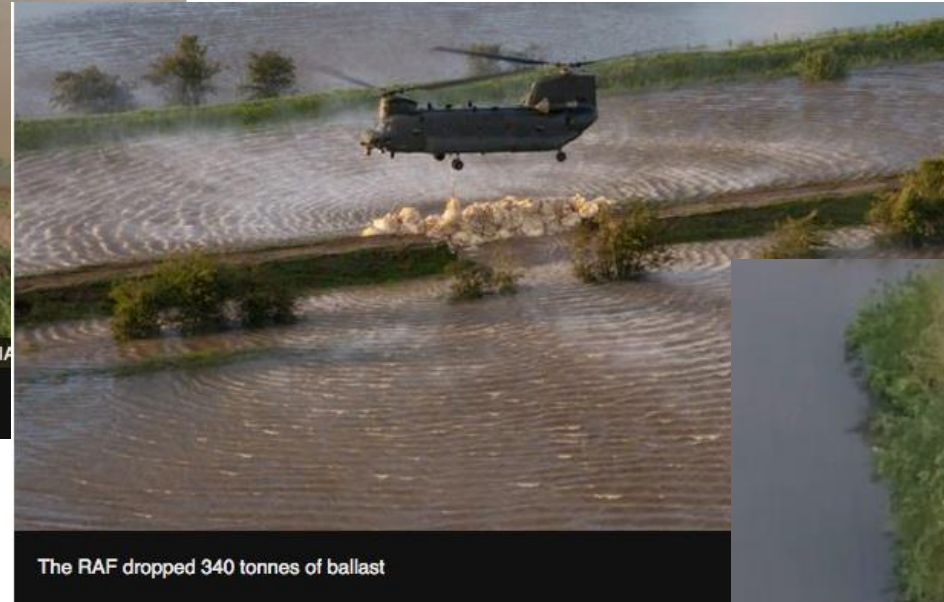


American River Levees California

<http://www.watereducation.org/tour/bay-delta-tour-2018-0>

International perspective: United Kingdom

UK Environment Agency responsible for 9,000 km of flood embankment



580 homes in the Wainfleet area were evacuated after the River Steeping burst its banks this June

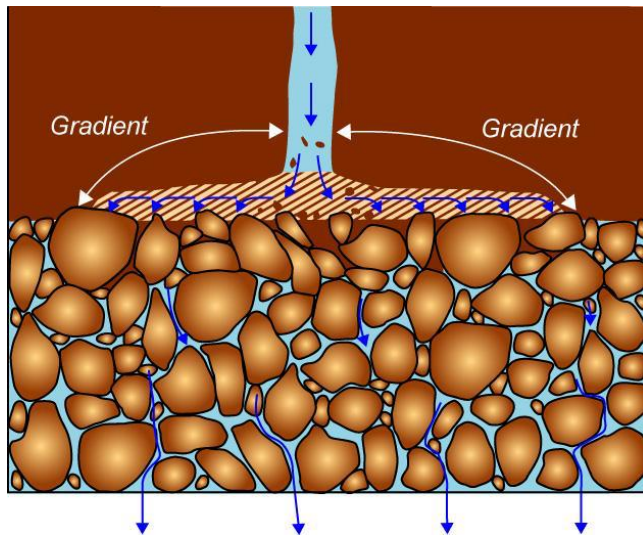


<https://www.bbc.co.uk/news/uk-england-lincolnshire-48646801>

<https://www.bbc.co.uk/news/uk-england-lincolnshire-48707396>

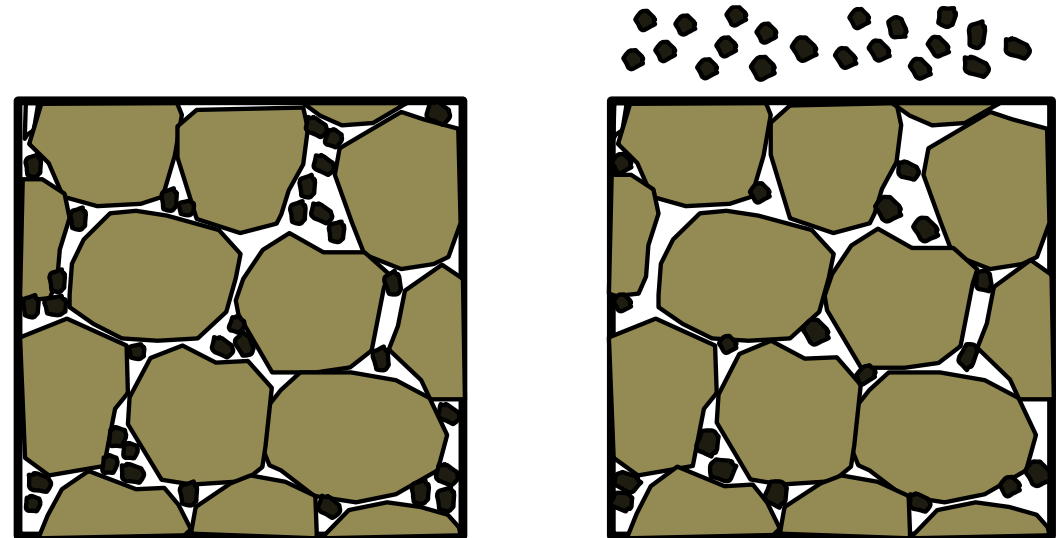
Design issues requiring a particulate perspective

1. Base – filter compatibility



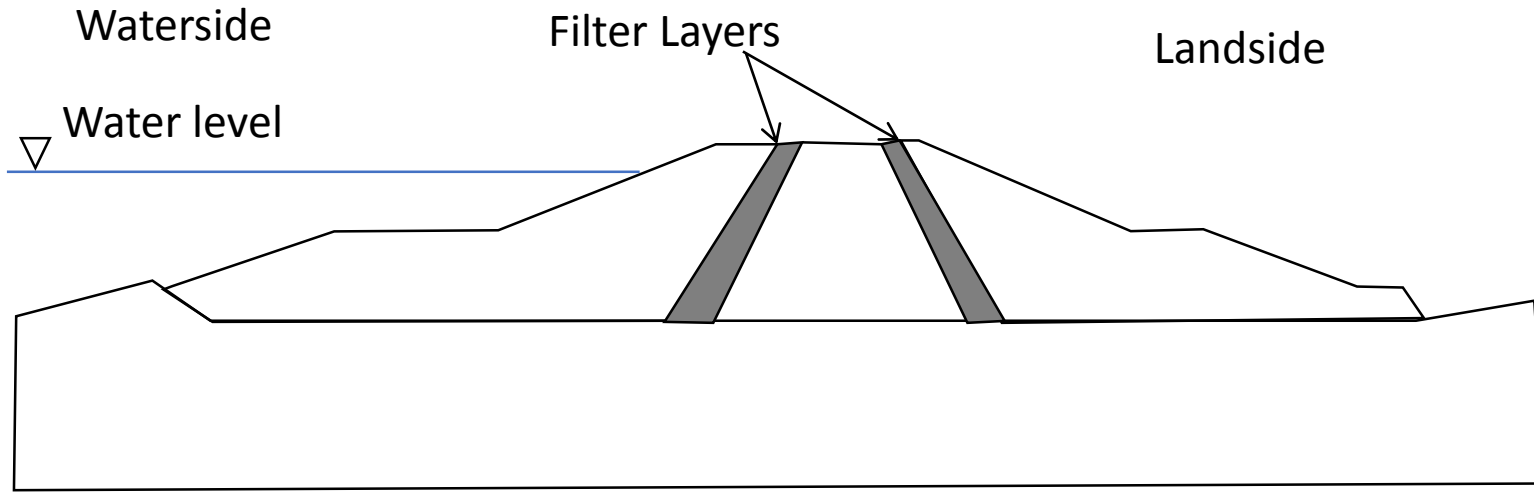
(FEMA, 2011)

2. Internal instability / suffusion

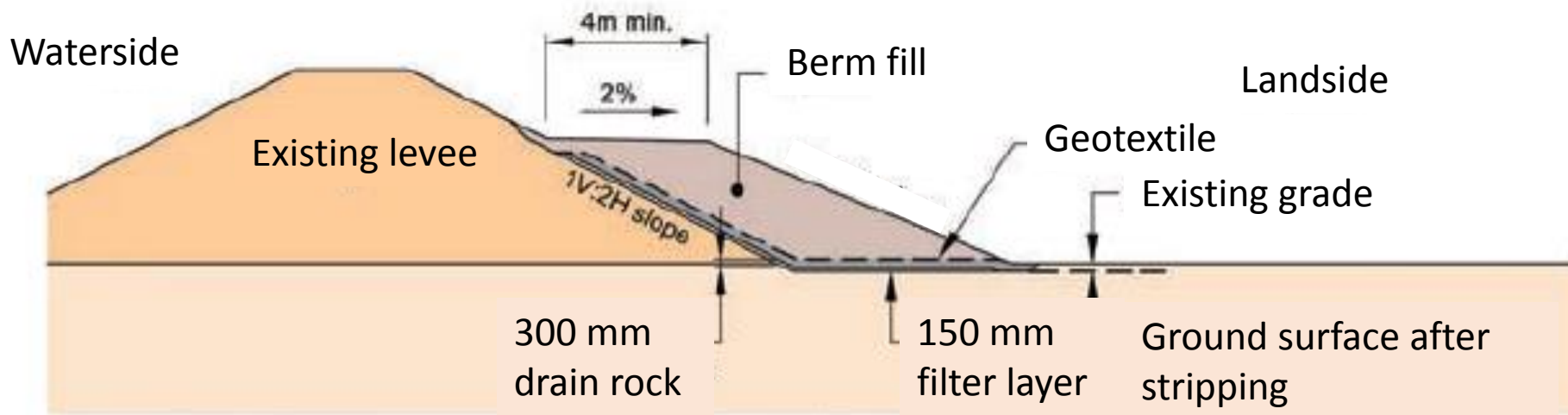


After Slangen and Fannin

Filters: Dikes



Engineered levee

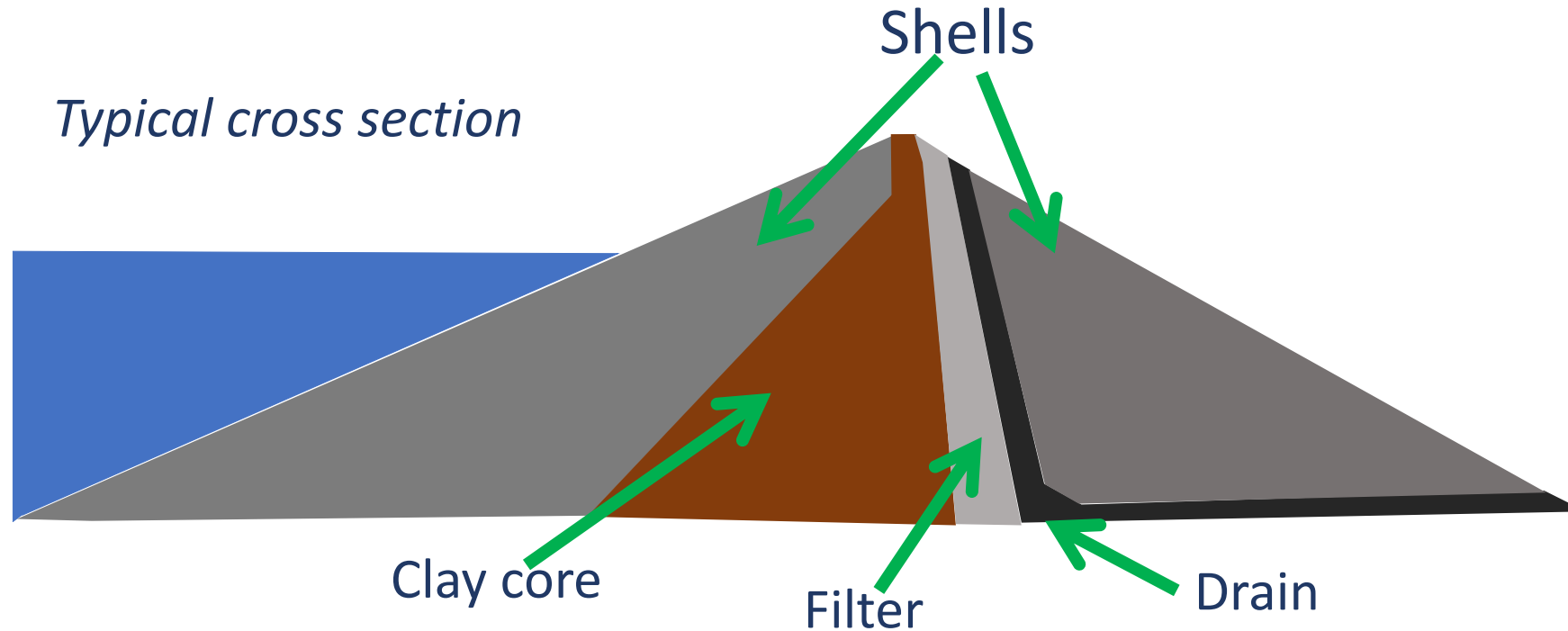


Retrofit of existing levee

Drained Stability Berm

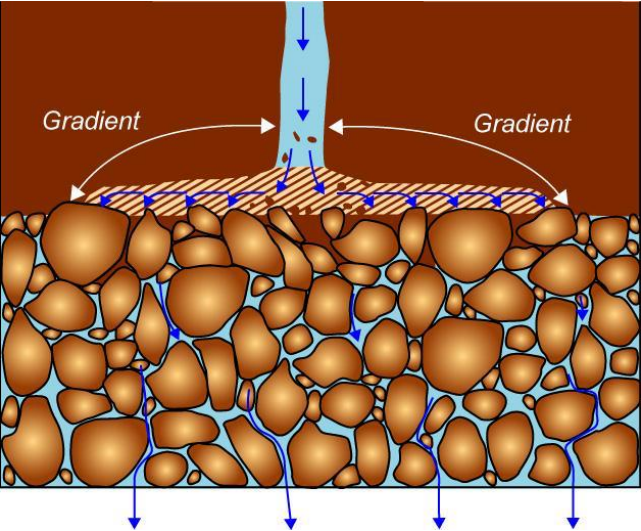
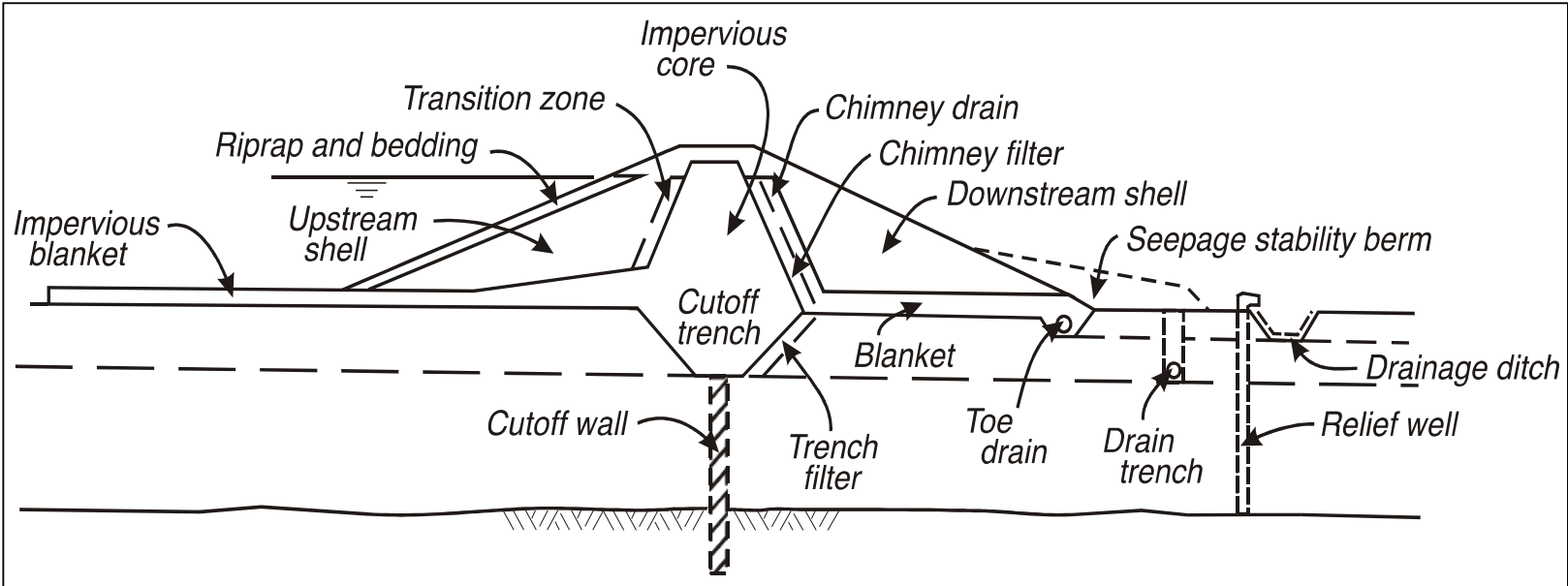
(International Levee Handbook, 2013)

Filters: Embankment dams



- Dams can be over 100 m high
- Water seeps through dam continuously
- Seeping water can preferentially erode fines
- In the UK about 2,500 dams retain reservoirs exceeding 25,000 m³
- In the US there are about 90,580 dams

Filters: Embankment dams



(FEMA, 2011)



Downstream protection

Core

Sandwich filter

Paraperios dam - May 26 2010

Role of filters

Filters are designed and constructed to achieve specific goals such as preventing internal soil movement and controlling drainage (FEMA, 2011)

Five filter functions govern the capability of providing control for internal erosion.

1. Retention.
2. Self Filtration or stability.
3. No cohesion
4. Drainage.
5. Strength. ICOLD (2015)

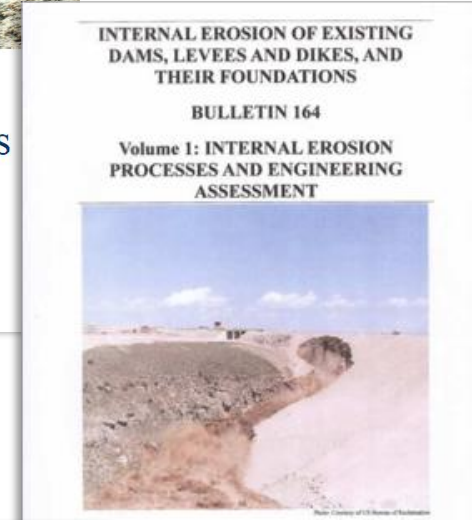
Filter layers help to promote filtration, by preventing soil from migrating especially from the impervious core. (International Levee Handbook, 2013)



Filters for Embankment Dams

Best Practices for Design and Construction

October 2011



INTERNAL EROSION OF EXISTING DAMS, LEVEES AND DIKES, AND THEIR FOUNDATIONS

BULLETIN 164

Volume 1: INTERNAL EROSION PROCESSES AND ENGINEERING ASSESSMENT



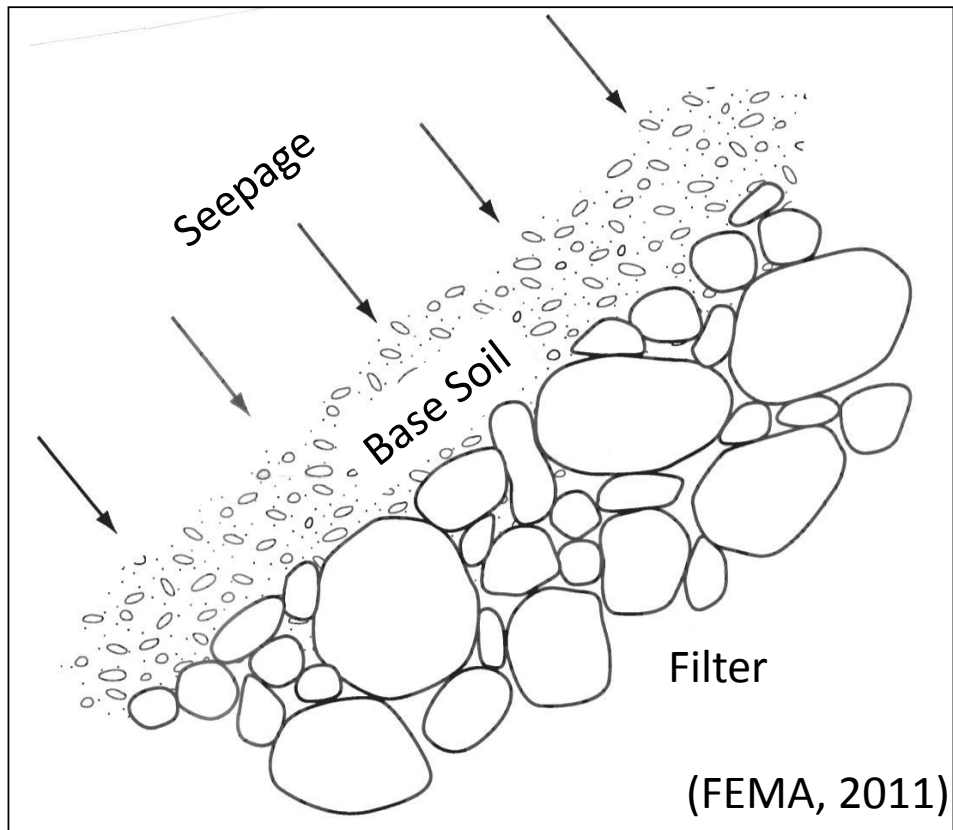
19 February 2015

The International Levee Handbook



Retention

The voids in the filter should be sufficiently small to prevent erosion of the base soil (ICOLD,2015)

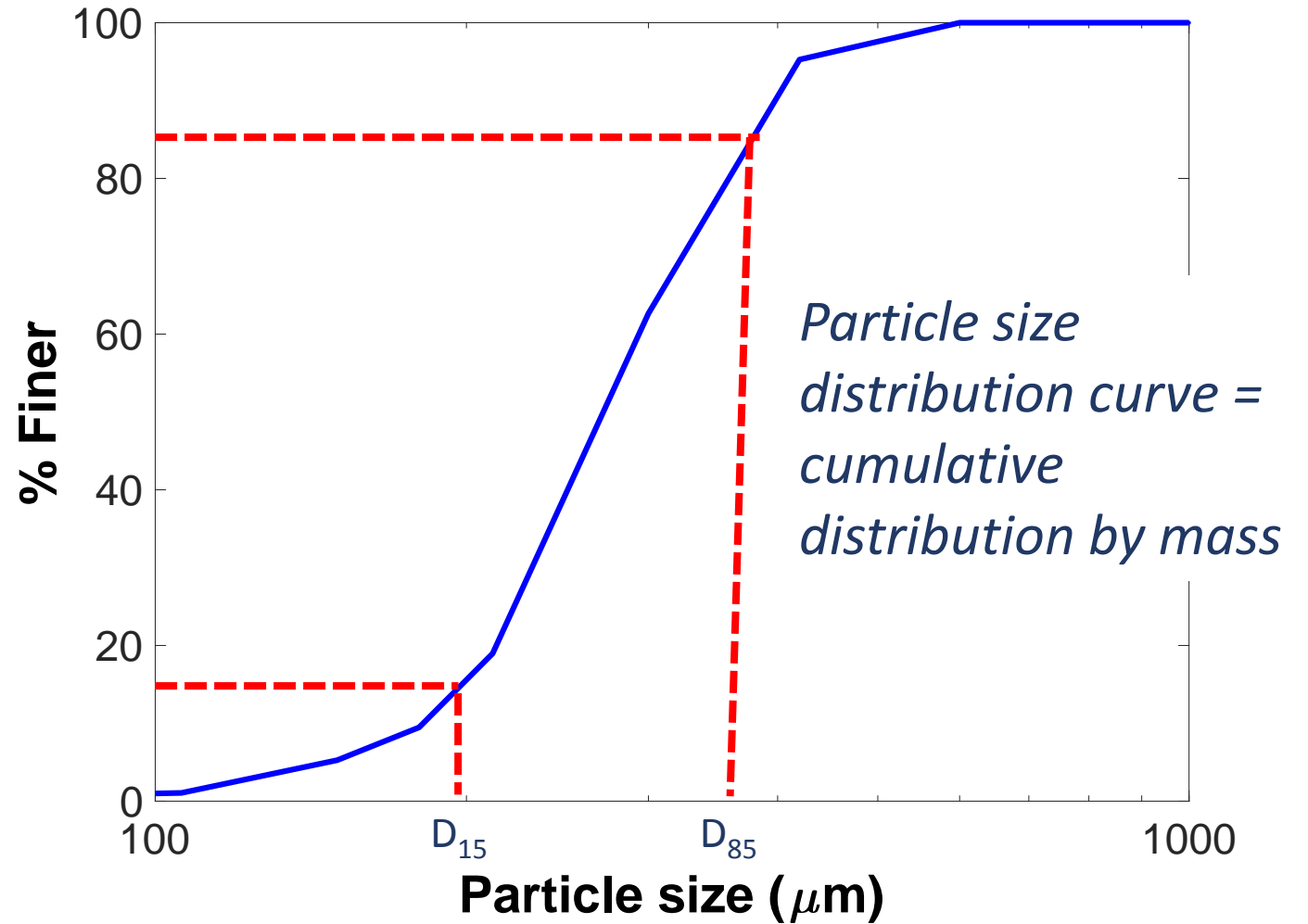


Empirical rules used in design:

- Consider particle size distribution
- Terzaghi's filter rule / Sherard & Dunnigan (1989)
 - D_{15F} of filter
 - D_{85B} of base
 - For retention $D_{15F} < 4 D_{85B}$
- Controlling constriction size – largest particle that can pass through filter

(ICOLD,2015)

Filter particle size distribution



15% of particles by mass are smaller than D_{15}

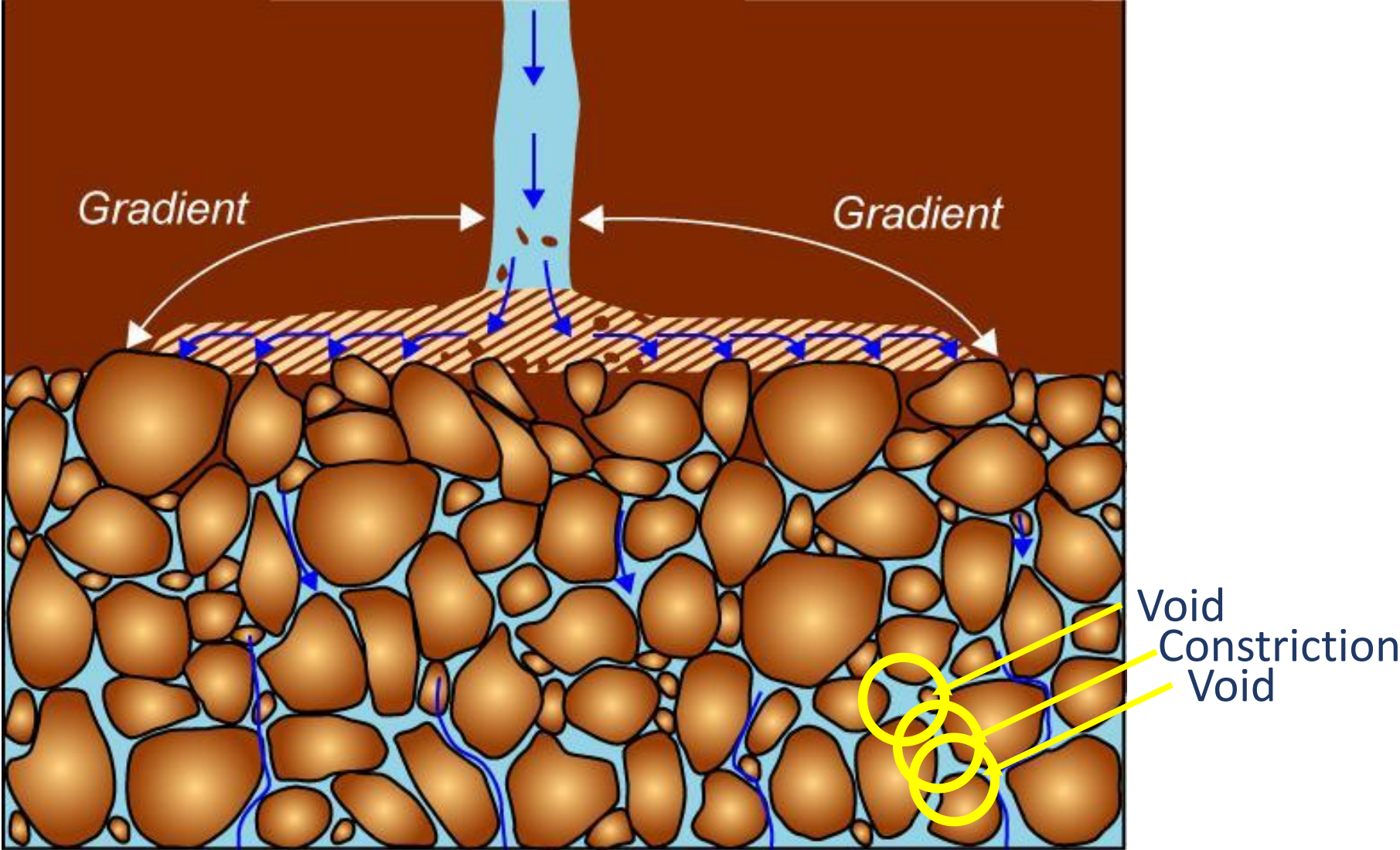
85% of particles by mass are smaller than D_{85}

Filter retention $D_{15F} < 4 D_{85B}$

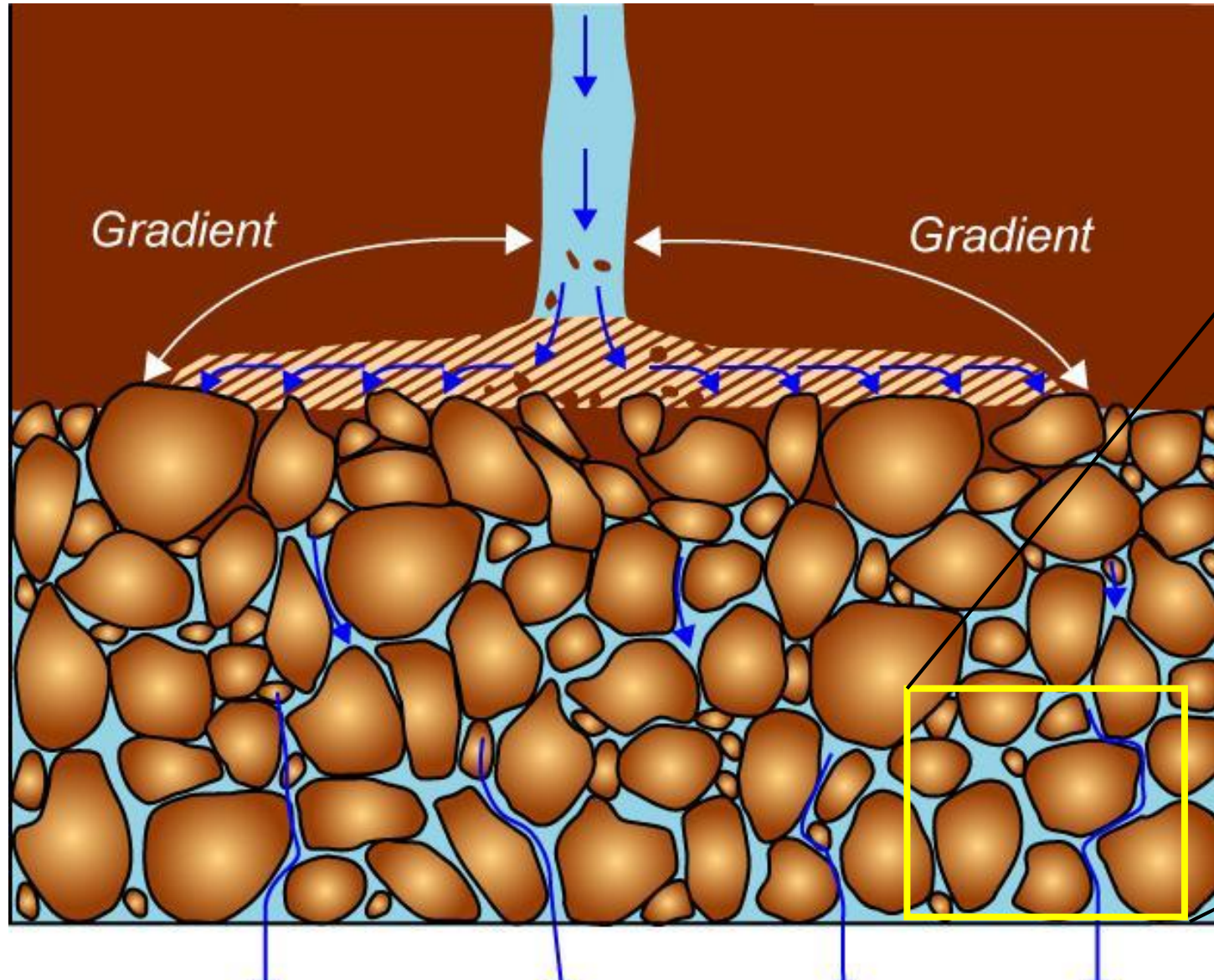
Research questions: Filter retention

- What is the relationship between the size of constrictions and D_{15F} ?
- Does particle scale analysis support use of the ratio D_{15F} / D_{85B} in design?

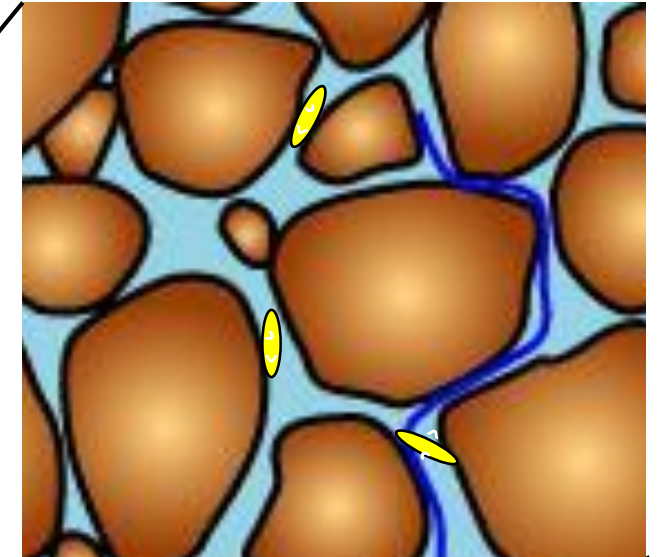
Retention



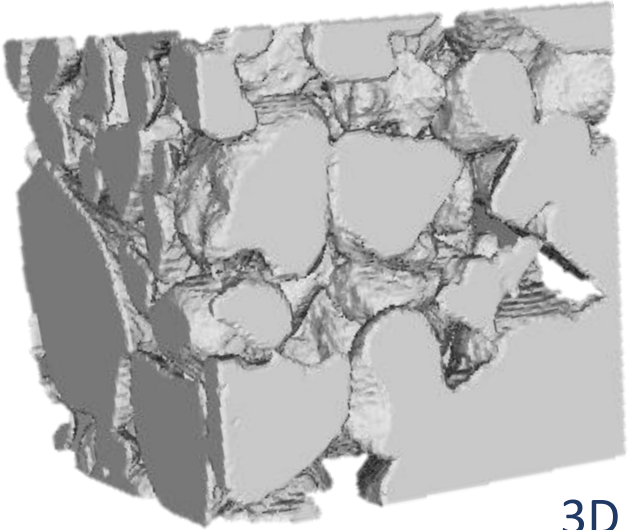
Retention



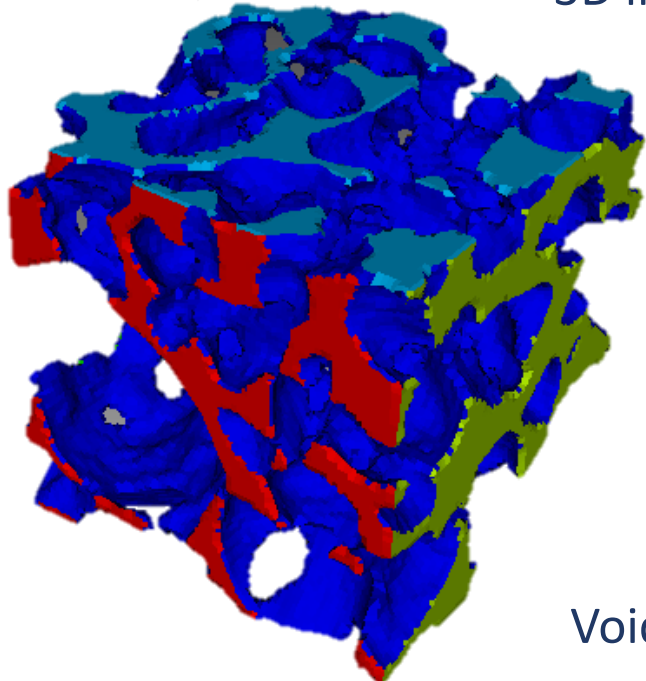
Fine core particles get trapped in constrictions



Quantifying constriction size & frequency



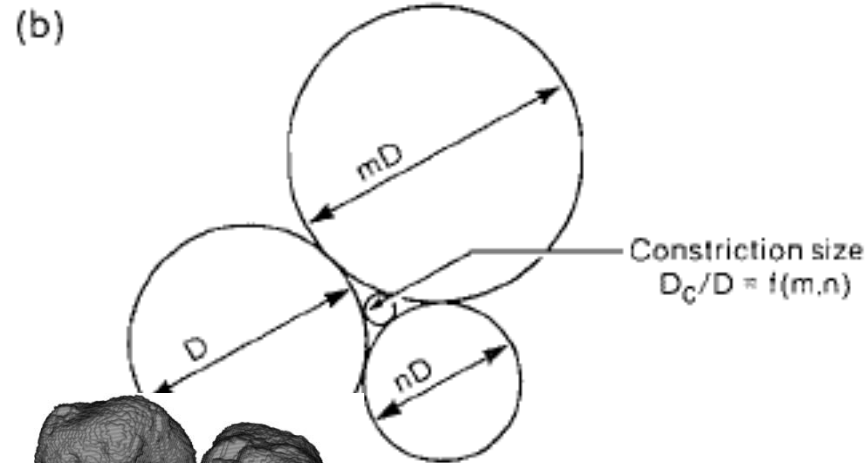
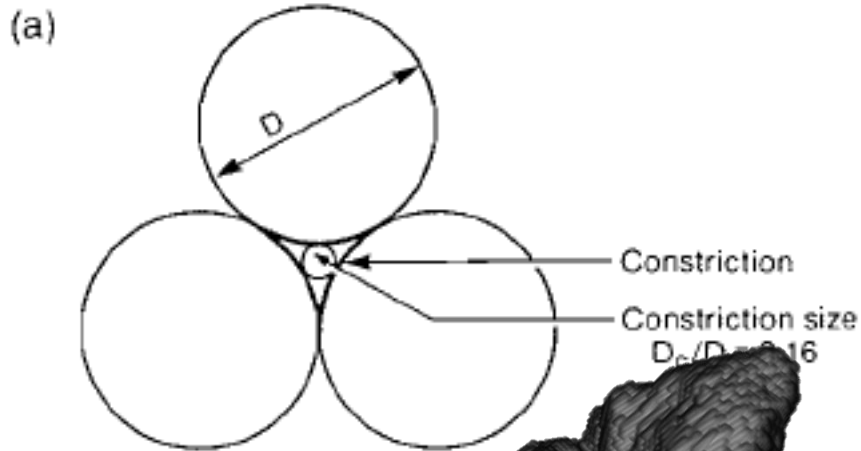
3D image of sand from micro CT



Void space from microCT

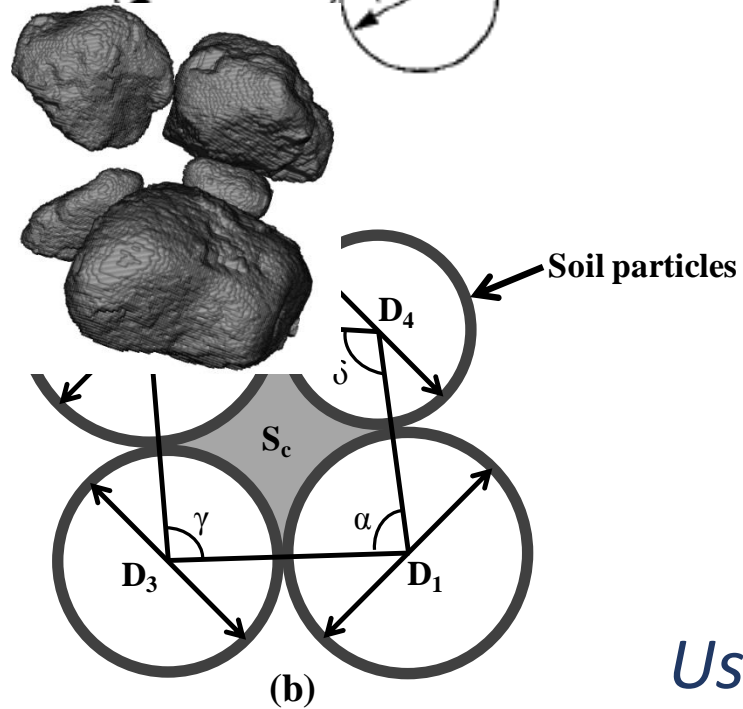
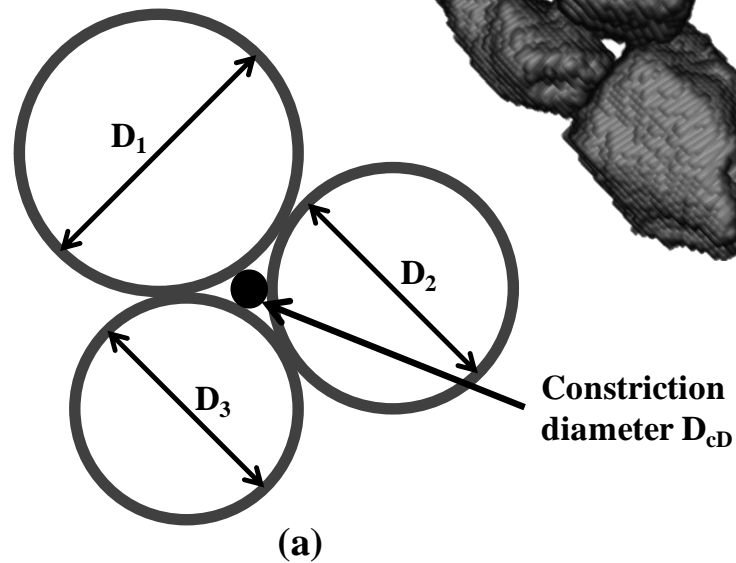
- Ability to image pore space is a recent advancement
- Pore space topology is complex
- Pore space is continuous
- Division between individual voids / pores is subjective

Analytical approach to quantify constriction sizes



Coplanar spheres

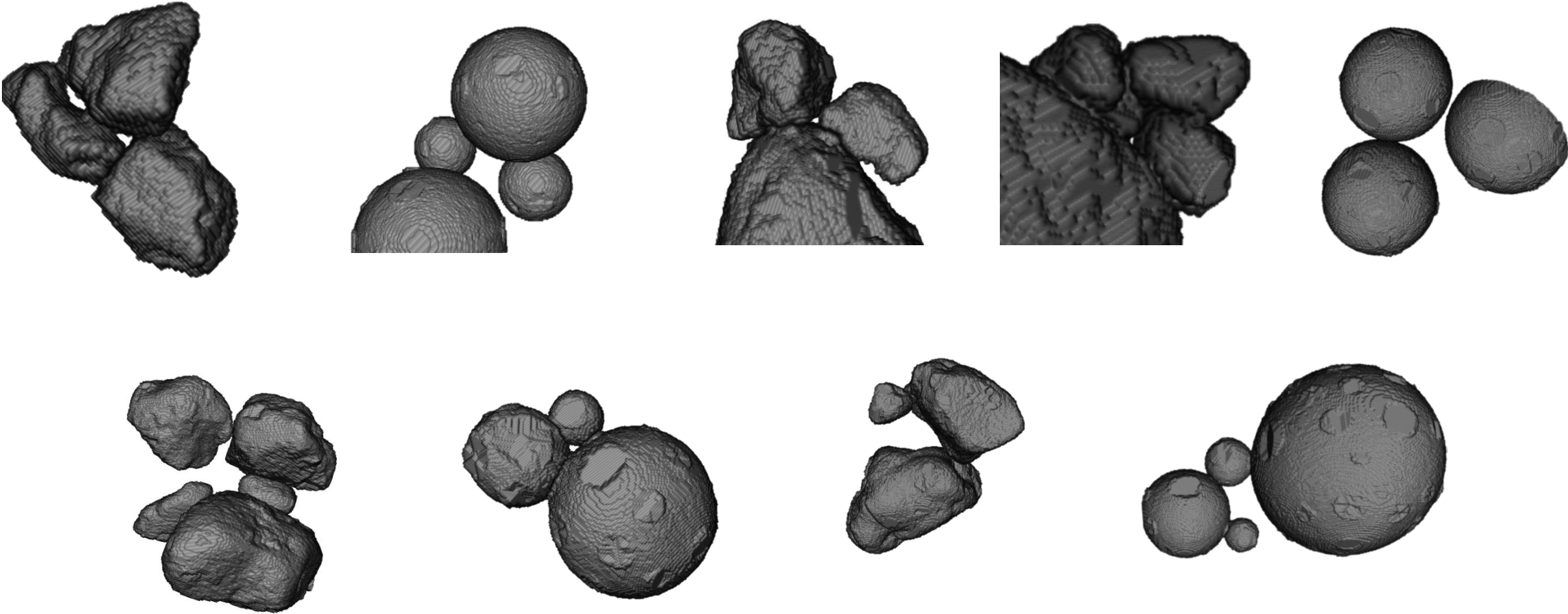
Kenney et al. (1985)



Silvera et al. (1985)

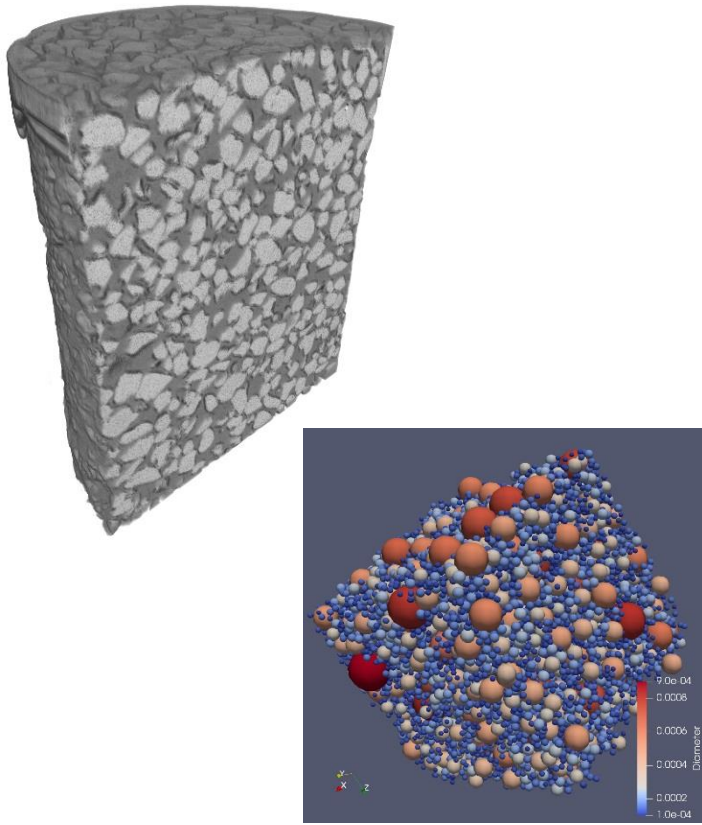
Used to justify design criteria

Real constrictions from microCT data

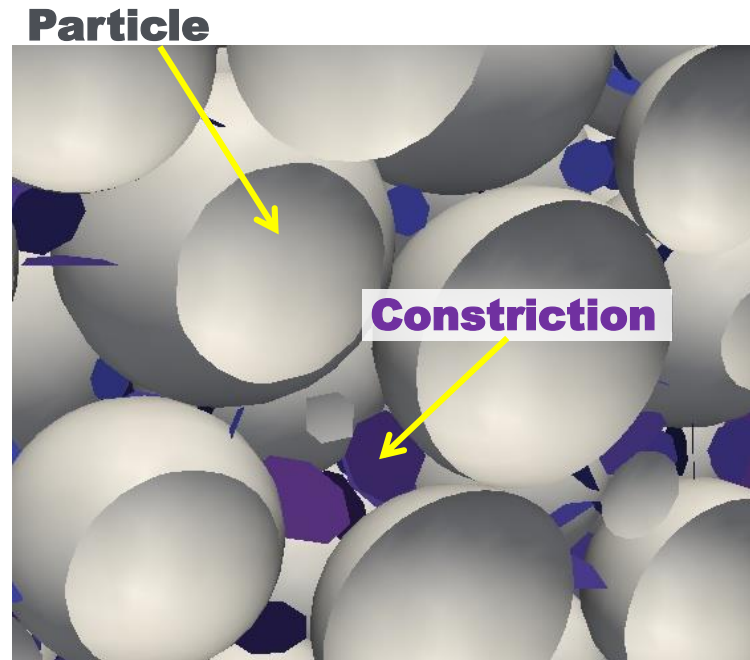


Determining constriction size distribution

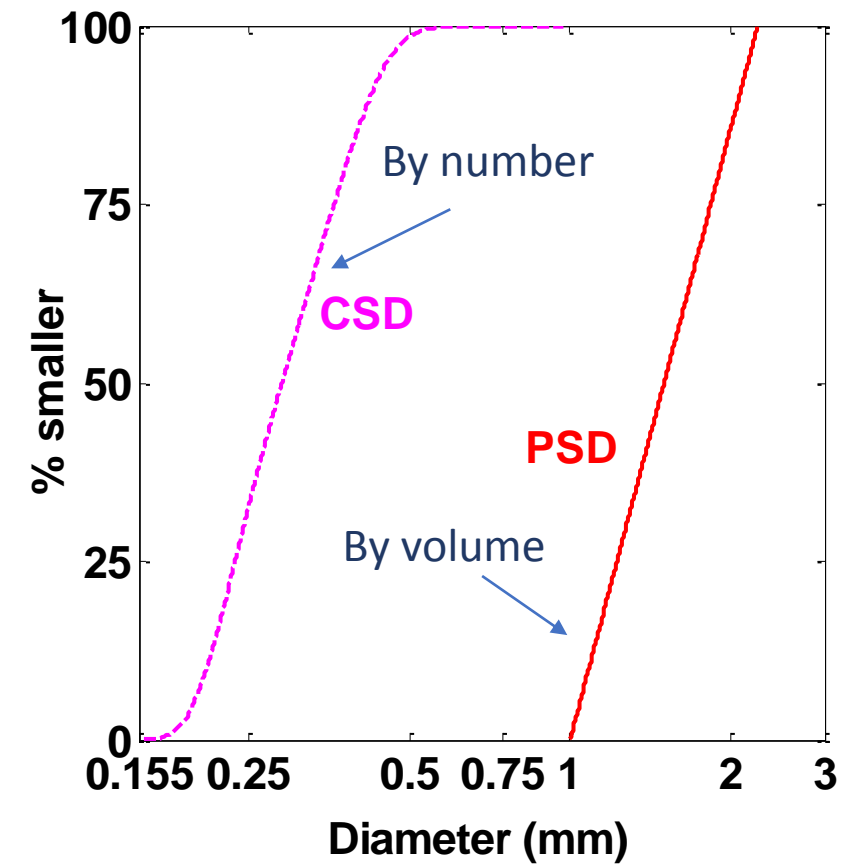
Generate particle scale data



Apply void partitioning algorithm

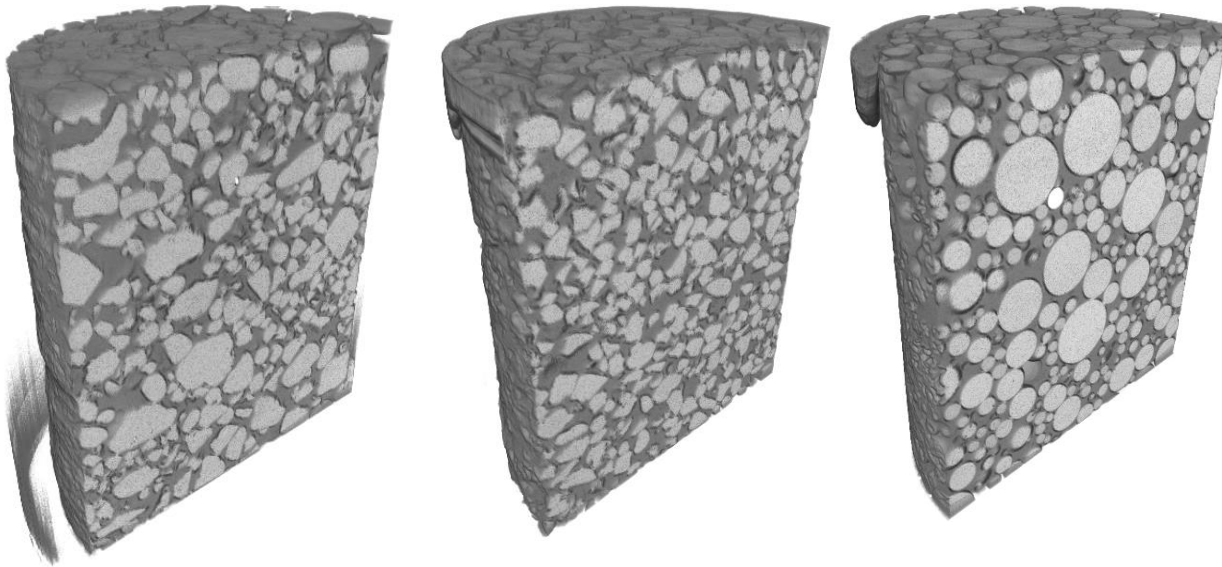


Calculate Constriction Size Distribution



Samples considered to study retention

Micro Computed Tomography

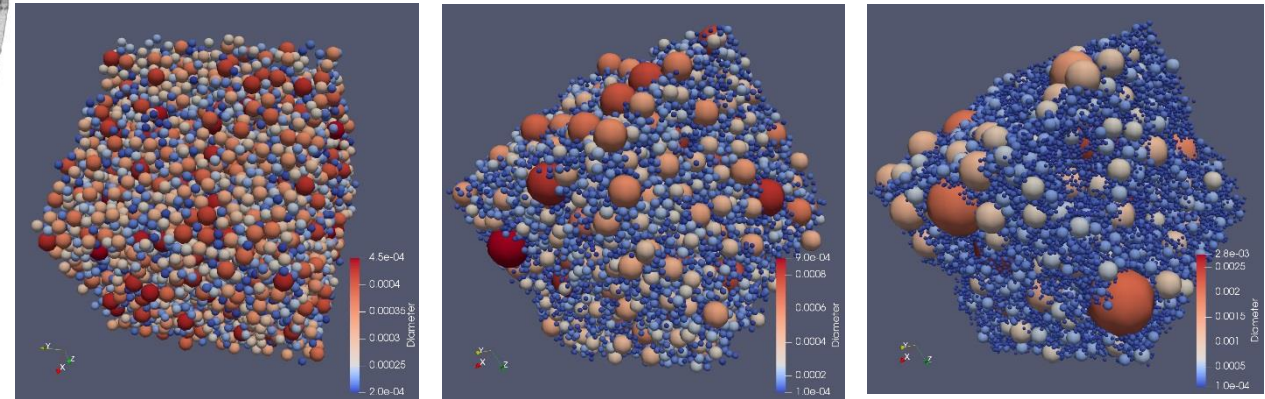


Leighton Buzzard
Sand
 $C_u=3$

Leighton Buzzard
Sand
 $C_u=1.5$

Glass Beads
 $C_u=3$

DEM Simulations



Spheres
 $C_u=1.2$

Spheres
 $C_u=3.0$

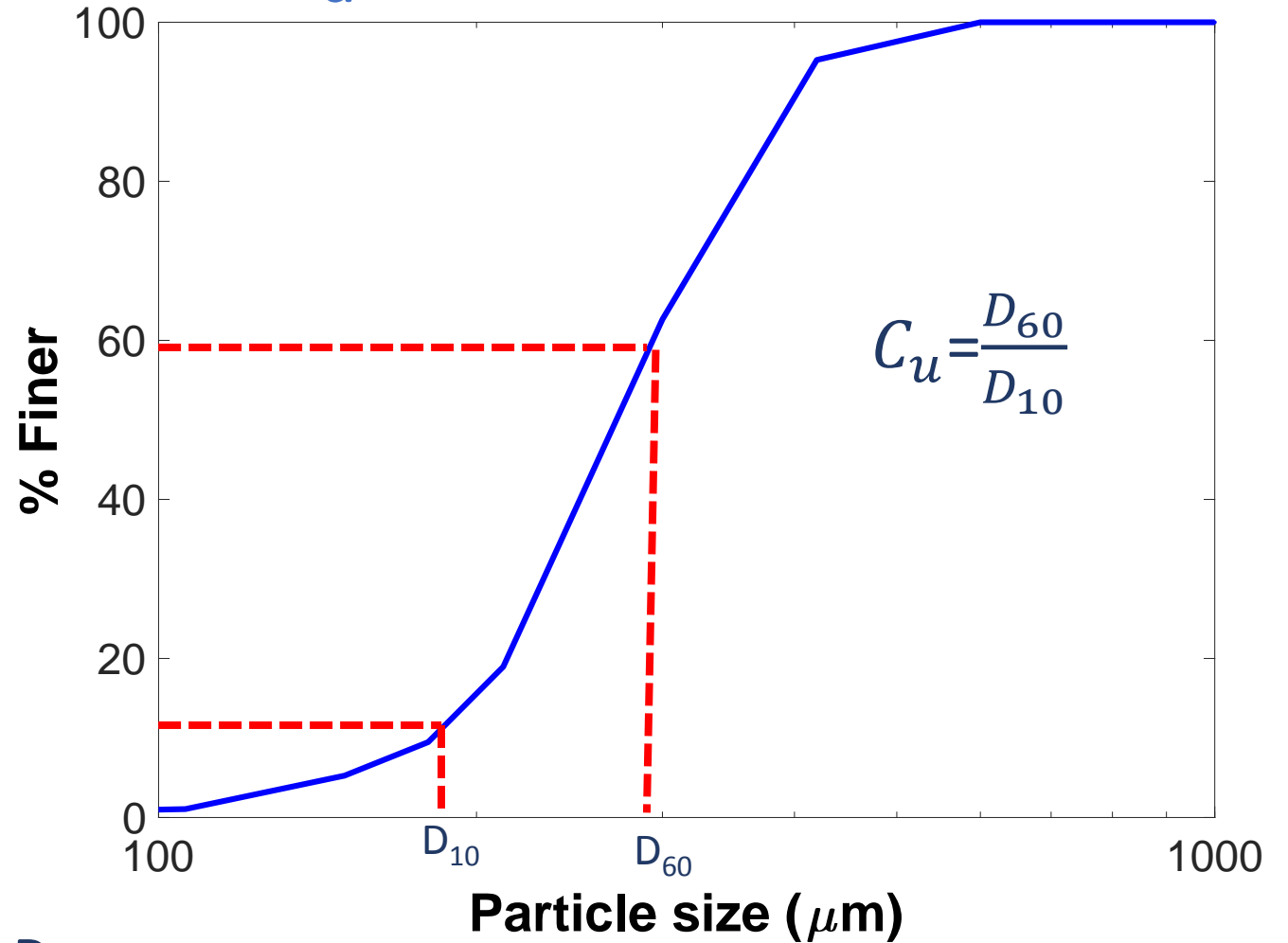
Spheres
 $C_u=6.0$

(Taylor, 2017)

(Shire, 2018)

Do not consider possibility of filters containing fines

Coefficient of uniformity, C_u



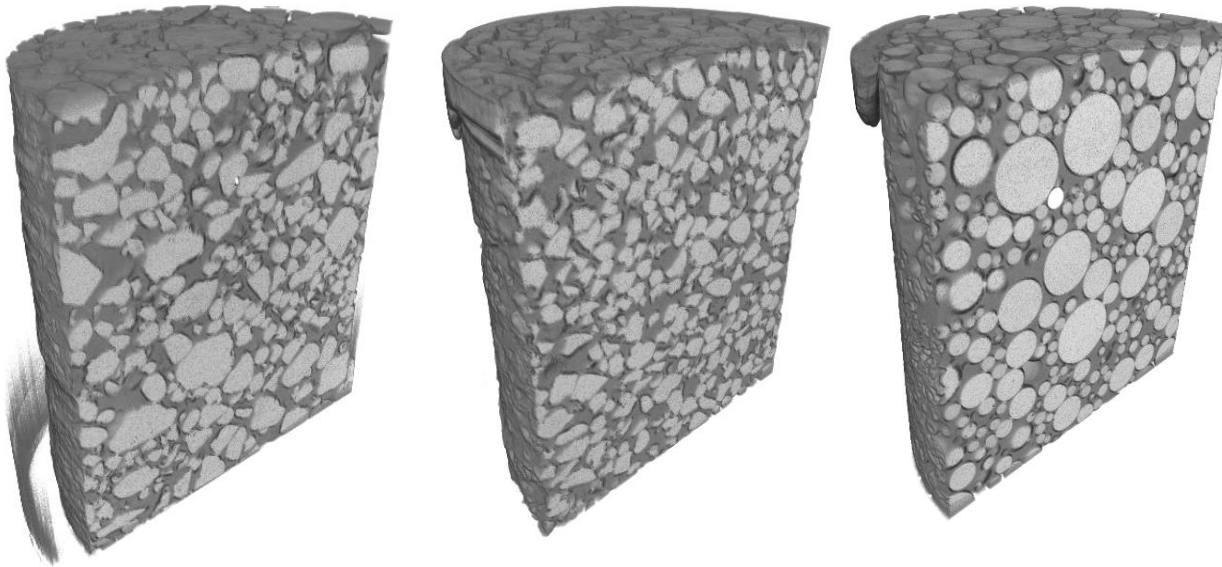
10% of particles by mass are smaller than D_{10}

60% of particles by mass are smaller than D_{60}

Cumulative distribution by volume / mass

Samples considered to study retention

Micro Computed Tomography

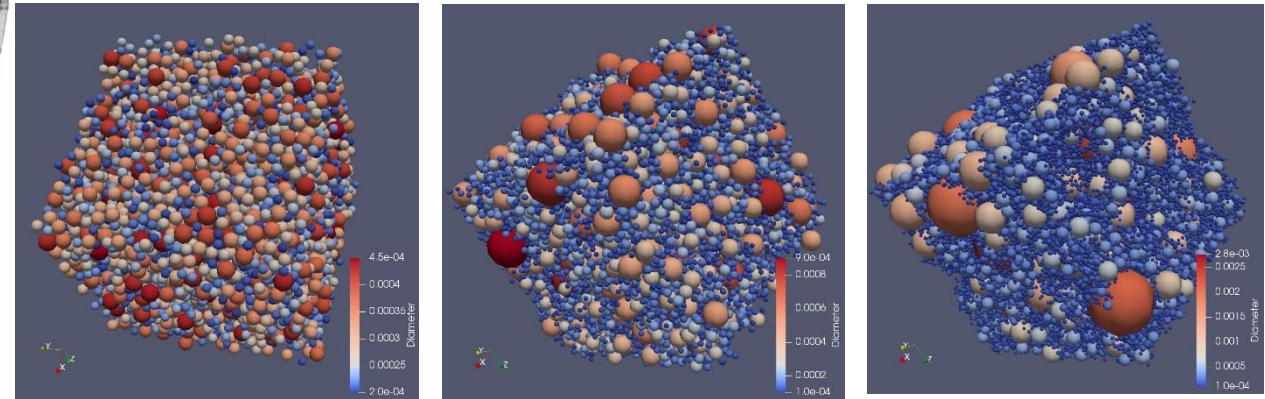


Leighton Buzzard
Sand
 $C_u=3$

Leighton Buzzard
Sand
 $C_u=1.5$

Glass Beads
 $C_u=3$

DEM Simulations



Spheres
 $C_u=1.2$

Spheres
 $C_u=3.0$

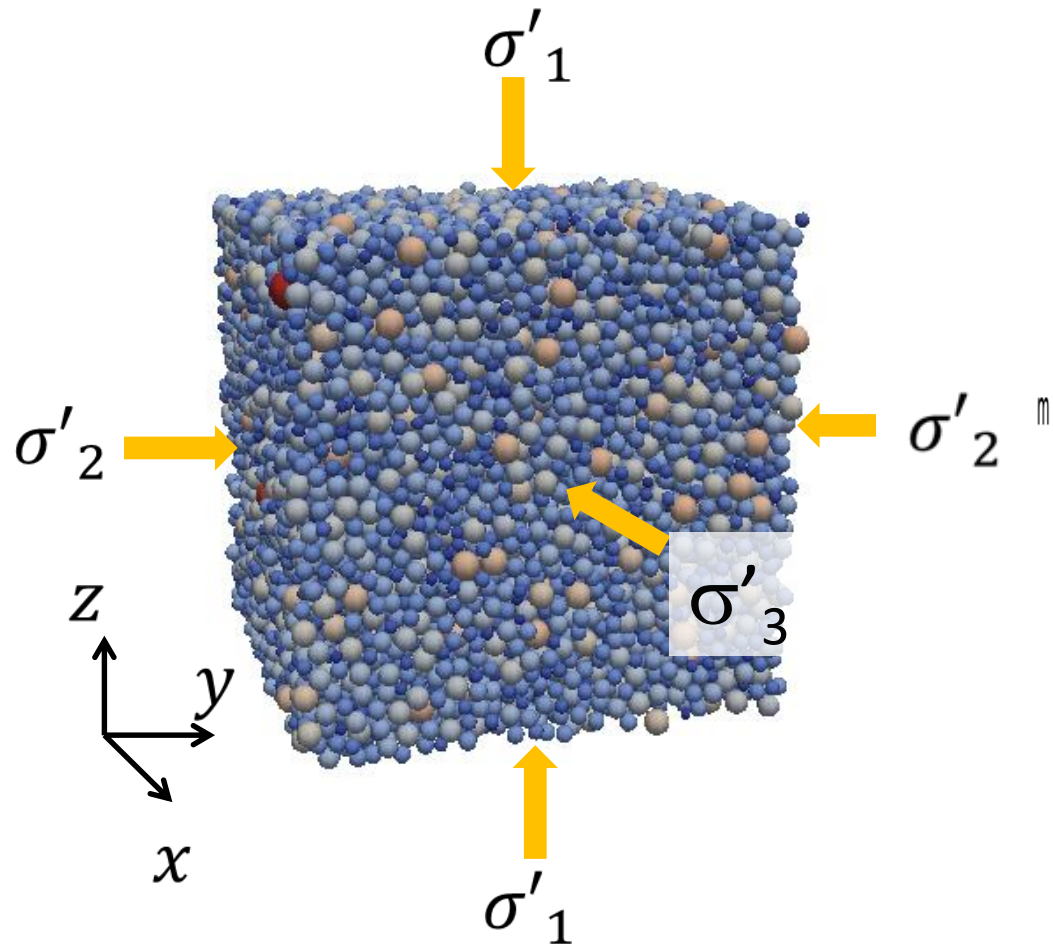
Spheres
 $C_u=6.0$

(Taylor, 2017)

(Shire, 2018)

Do not consider possibility of filters containing fines

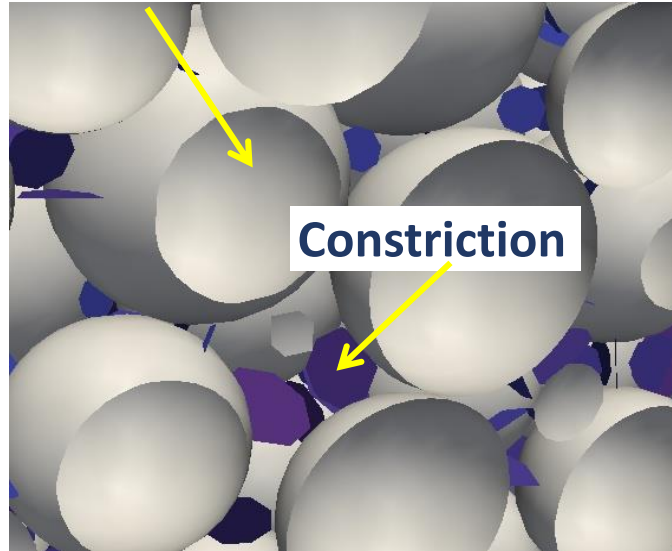
DEM simulations



- LAMMPS (Plimpton, 1995; Sandia National Laboratories)
- Development and testing by Dr. Kevin Hanley (formerly Imperial College, now Edinburgh)
- Validation using lattice packings / Benchmarking against PFC
- Periodic boundaries, Hertz-Mindlin contact model
- Used Imperial College HPC clusters
- Isotropic compression of to 60,000 spherical particles

DEM constrictions: Triangulation method

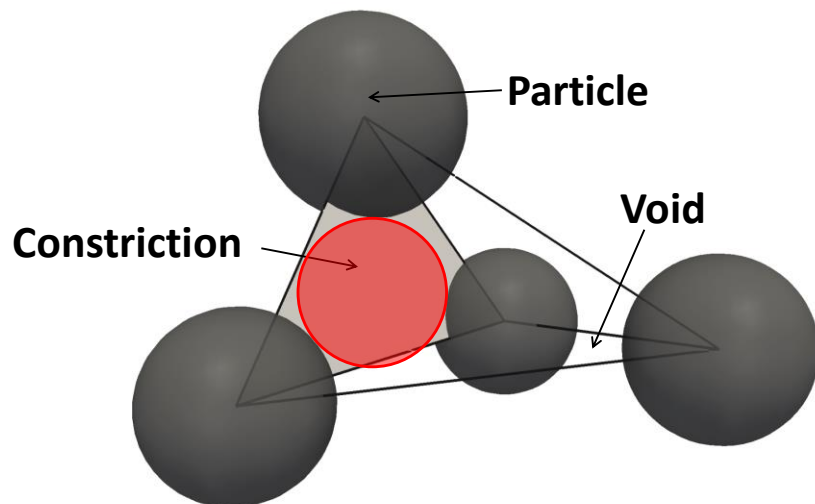
Particle



Triangulation of particle centres weighted by particle radii

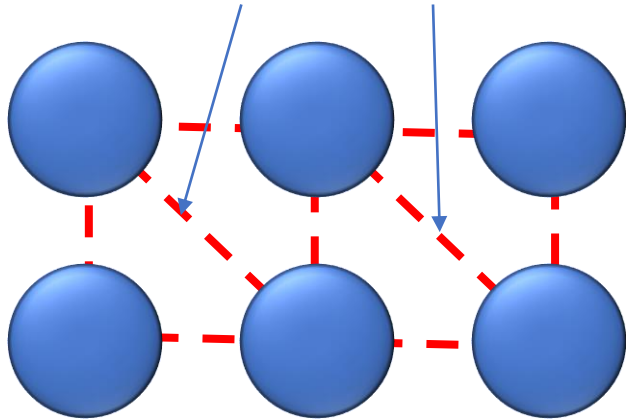
Tetrahedra faces define void boundaries

Constrictions located on tetrahedra faces

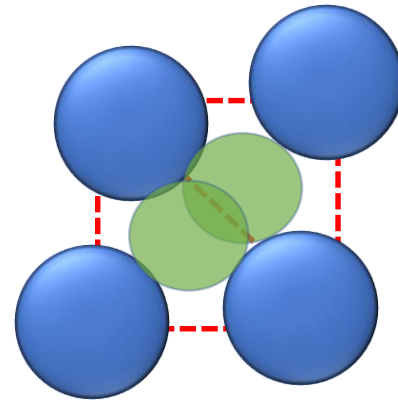


DEM constrictions: Triangulation method

False identification of constrictions due to over-segmentation

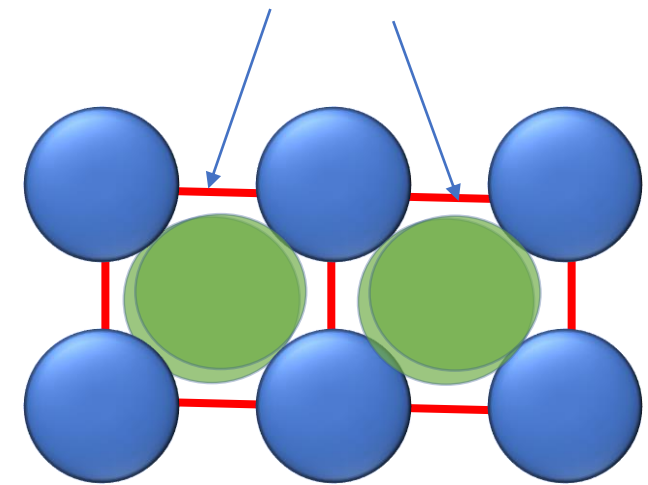


Delaunay triangulation based on particle centroids



Identify spheres tangent to particles forming Delaunay cell

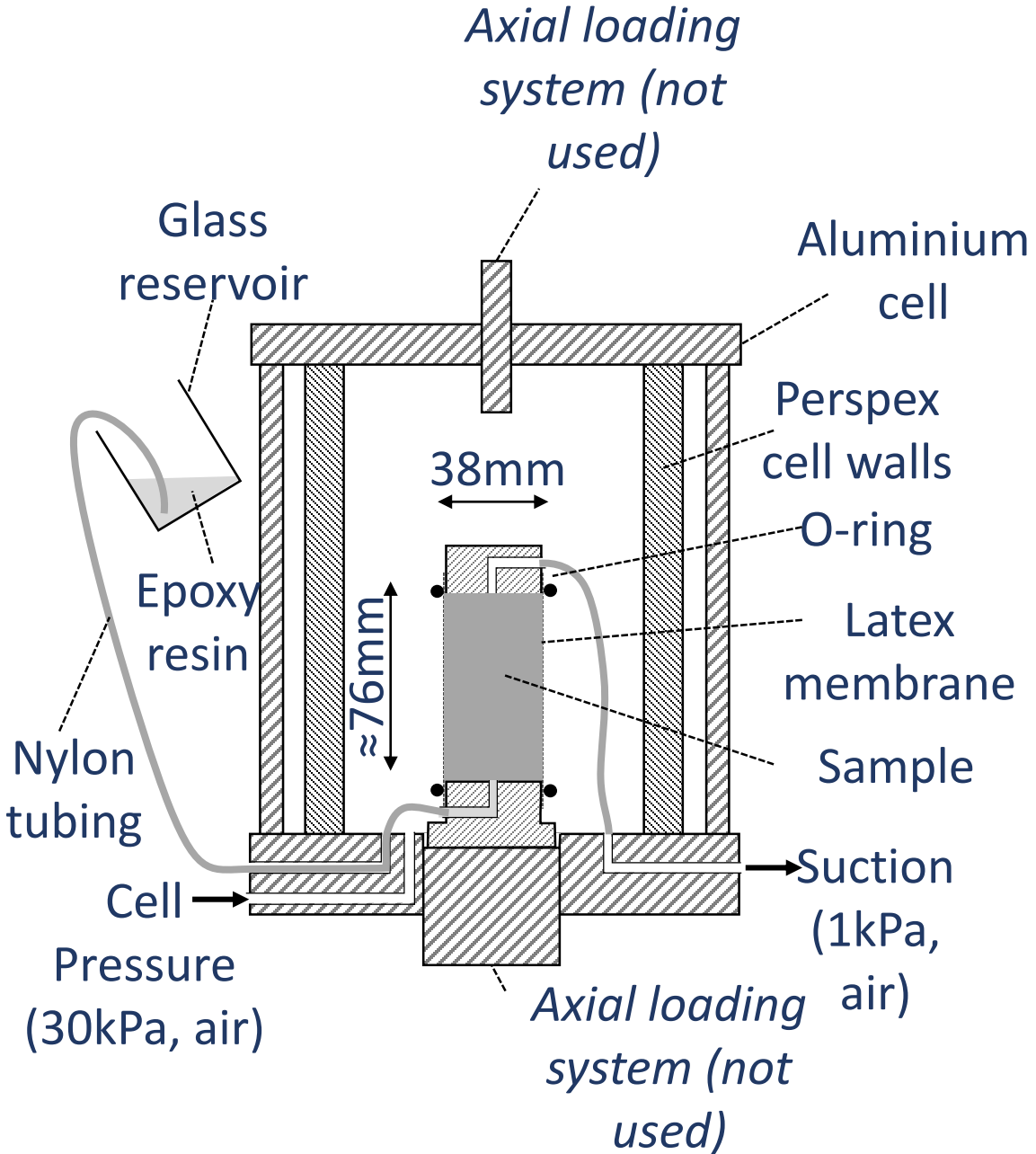
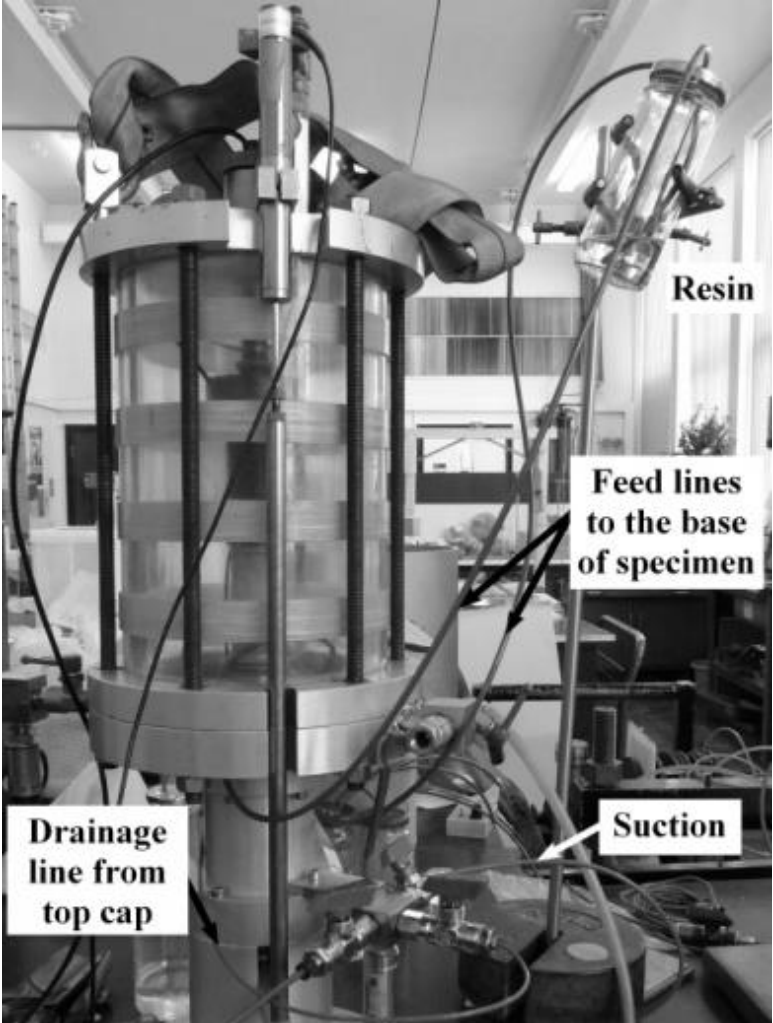
Valid constrictions



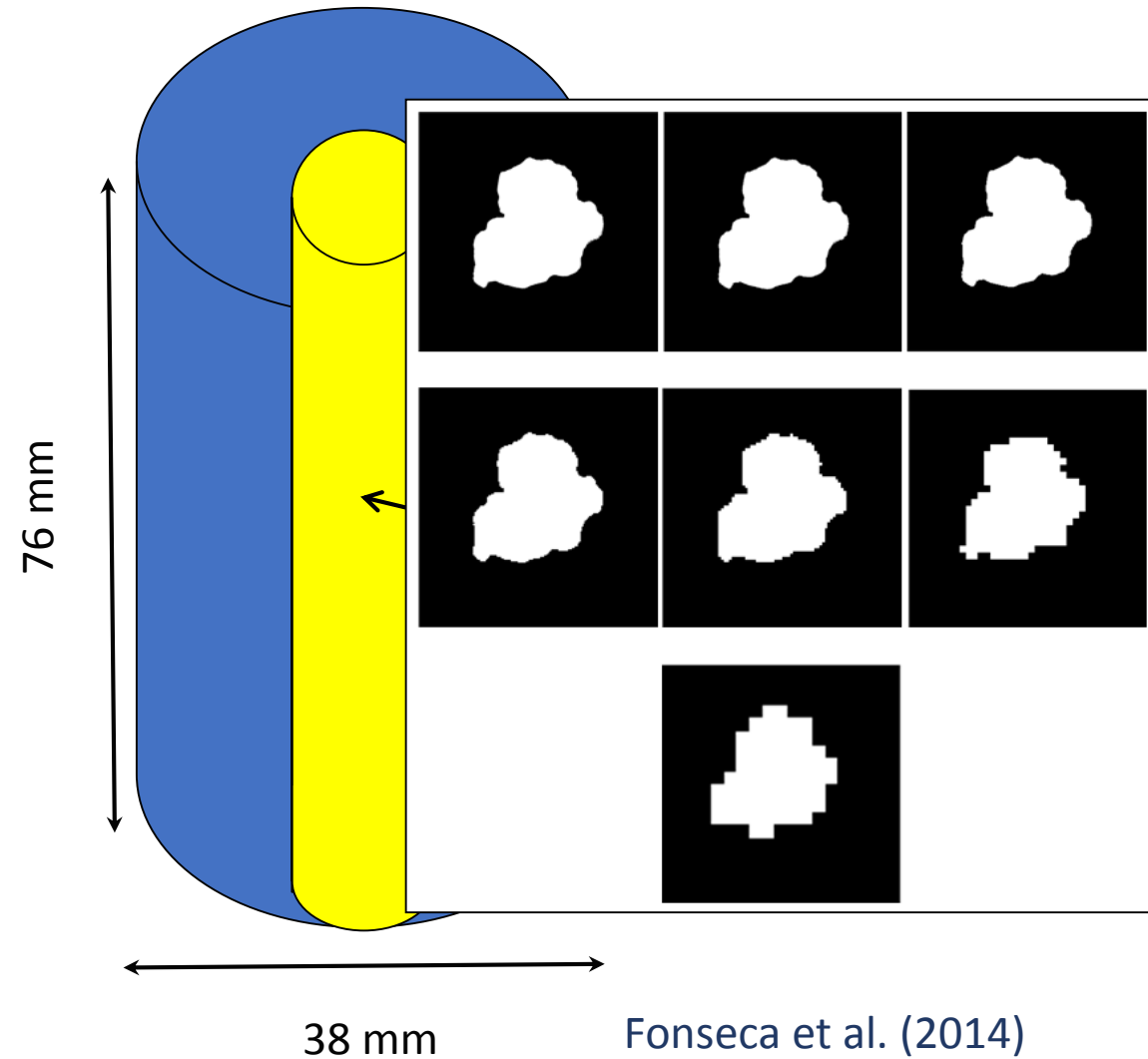
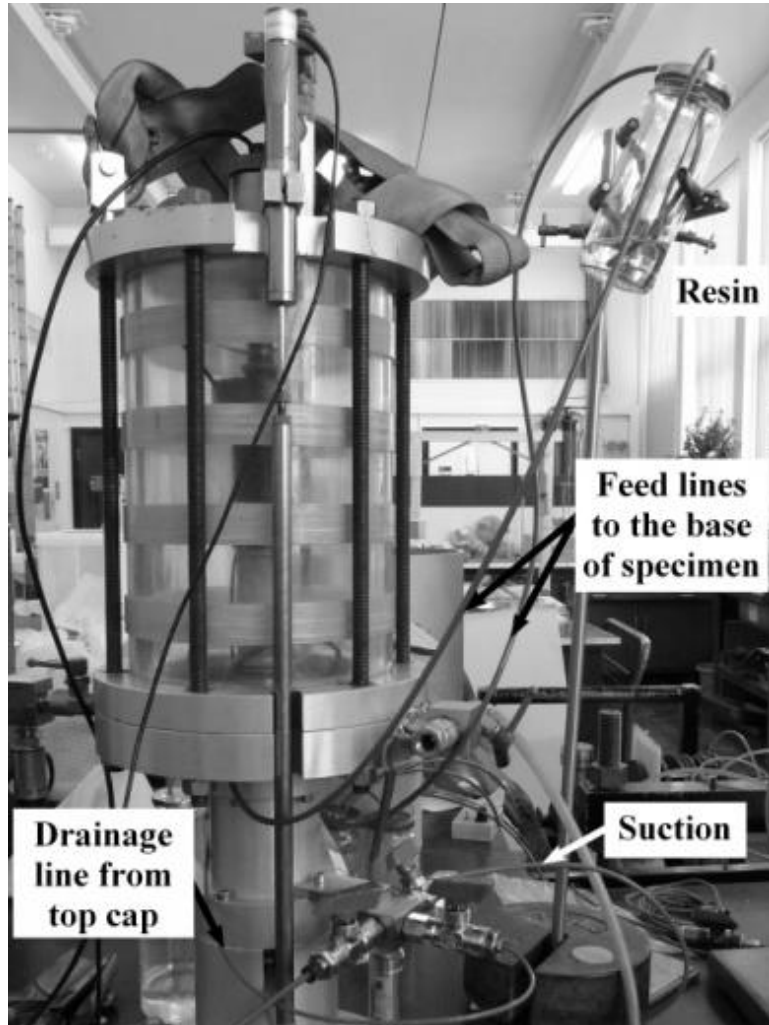
Where tangent spheres overlap Delaunay cells are merged

User decides magnitude of overlap

Experimental approach

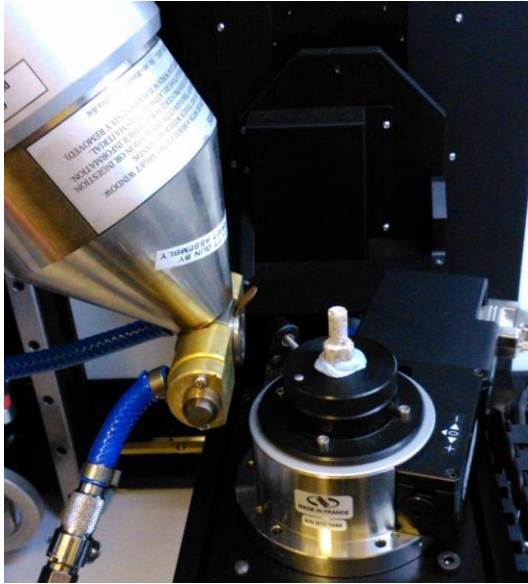


Experimental approach

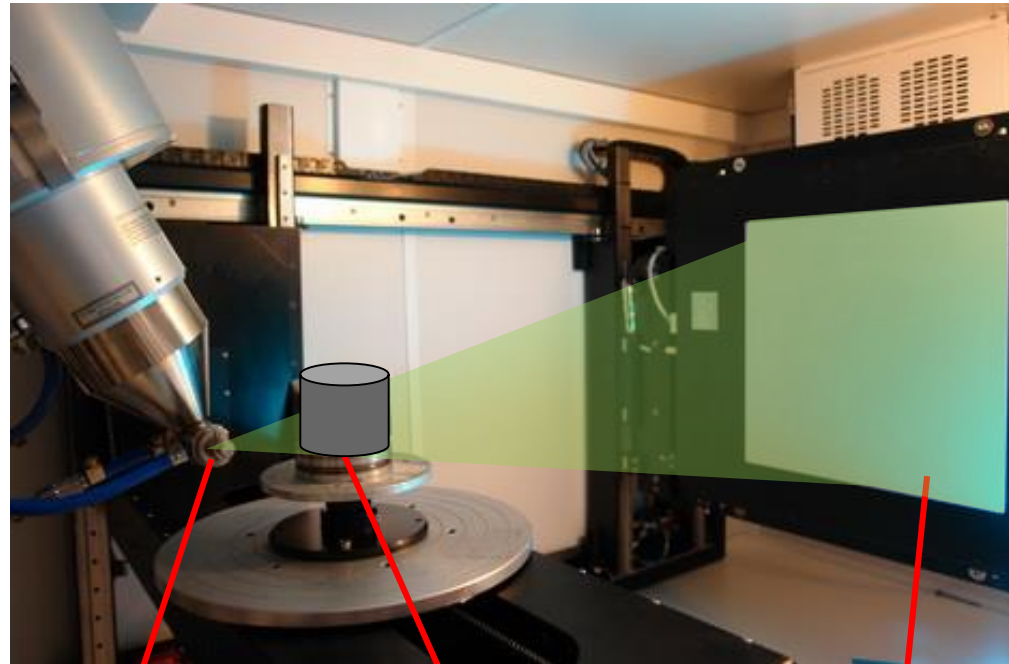


Fonseca et al. (2014)
Géotechnique

Micro Computed Tomography (microCT)



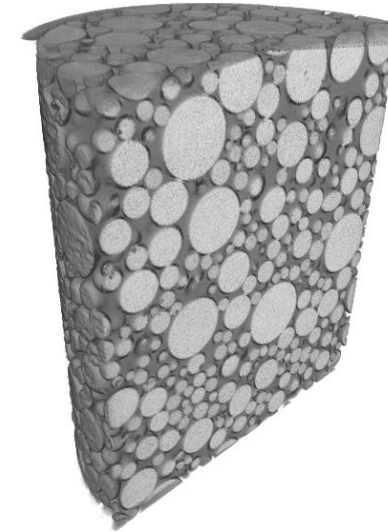
Nikon XT-H-224 scanner
Voxel size $\approx 10 \times 10 \times 10 \mu\text{m}^3$



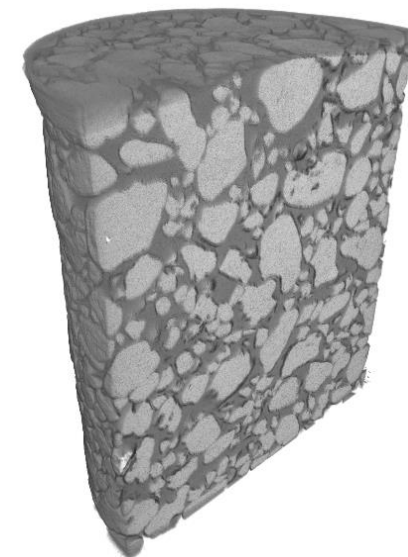
X-ray
source

Sample

Detector

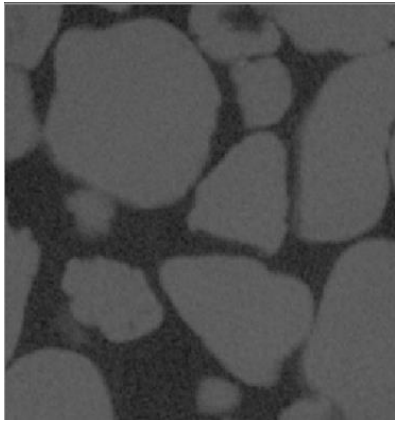


Cu3 - Glass Beads
 $e = 0.46$
(medium \rightarrow dense)



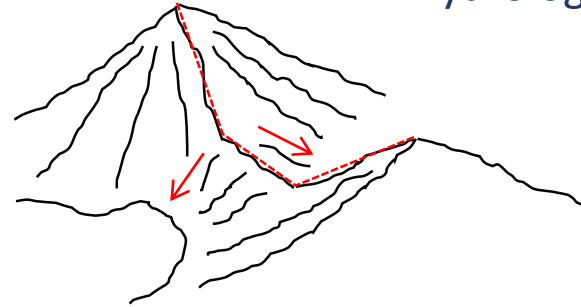
Cu3 - Sand
 $e = 0.51$
(medium density)

Segmentation of void space

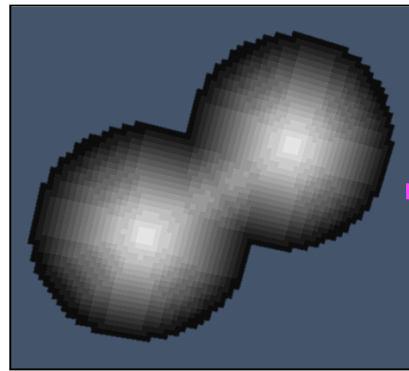
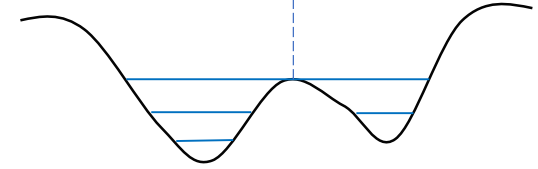


Threshold to identify gray level differentiating void space and particles

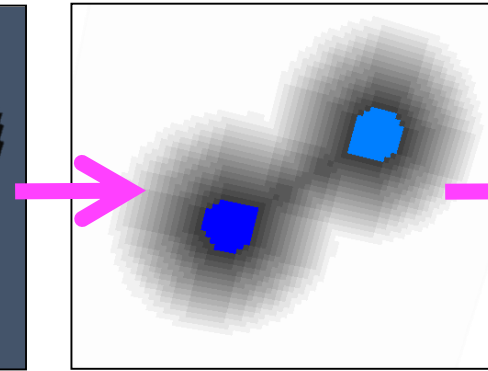
Watershed concept in hydrology



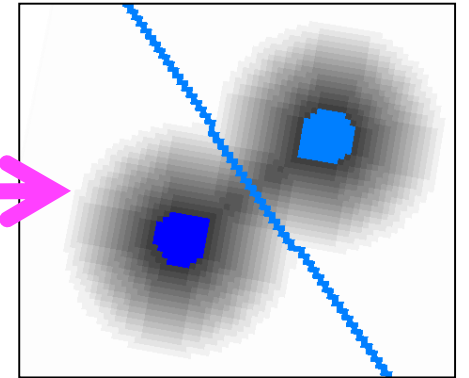
Watershed boundary



Distance map



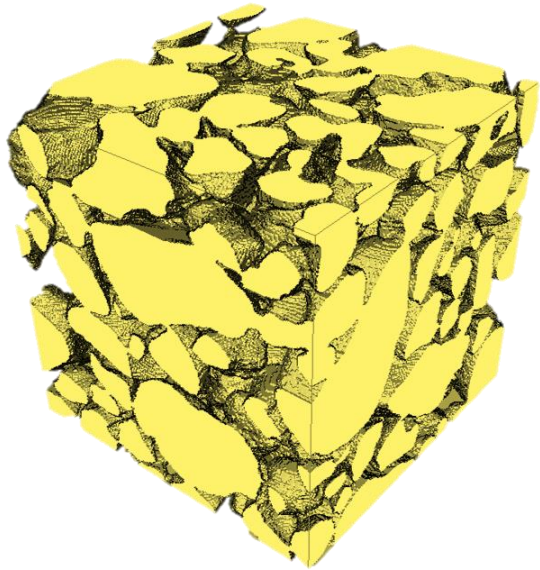
Centre locations



Watershed boundary

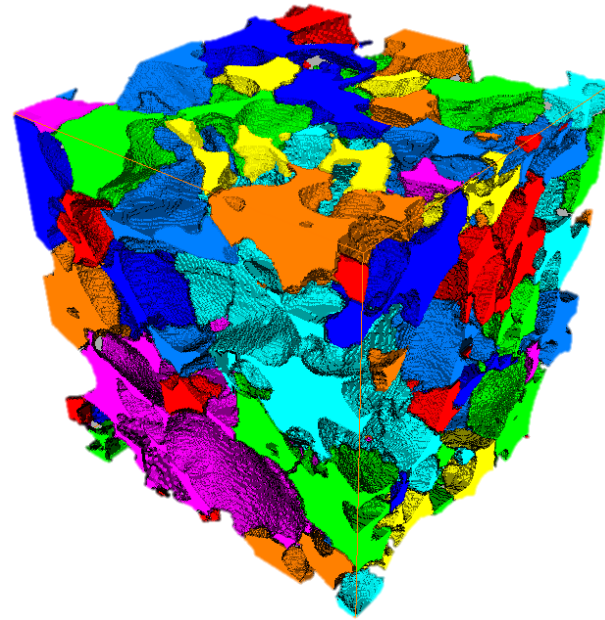
Performed using "Avizo Fire" software

Identifying constrictions in μ CT

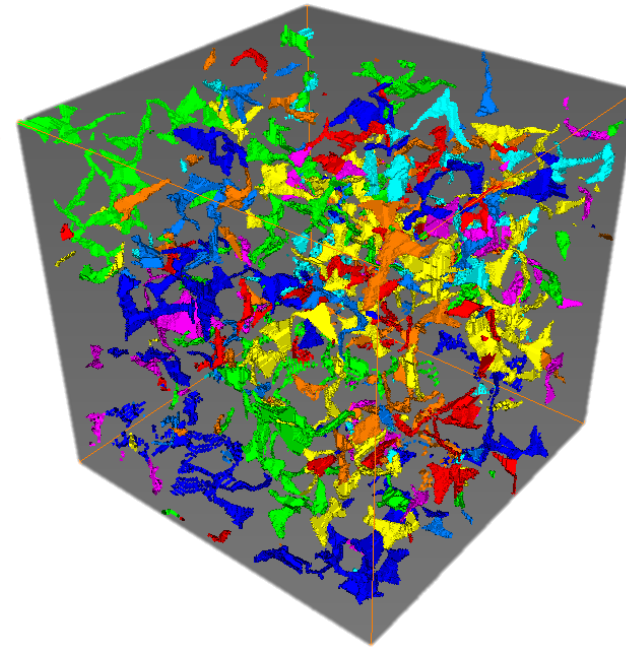


μ CT image

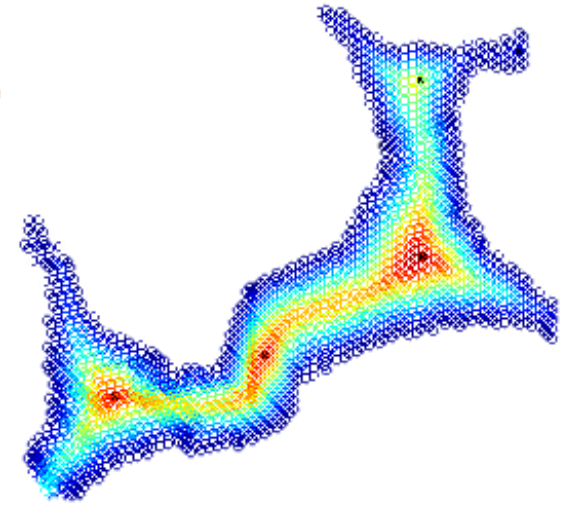
Watershed
Segmentation 



Voids

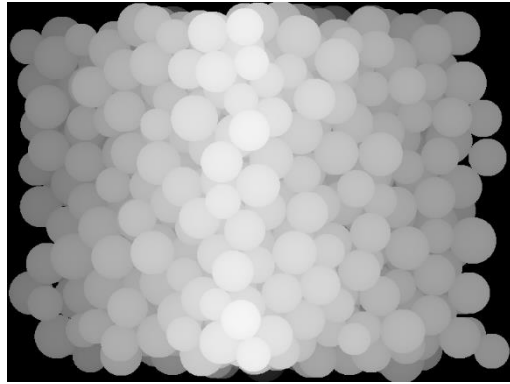


Void Boundaries

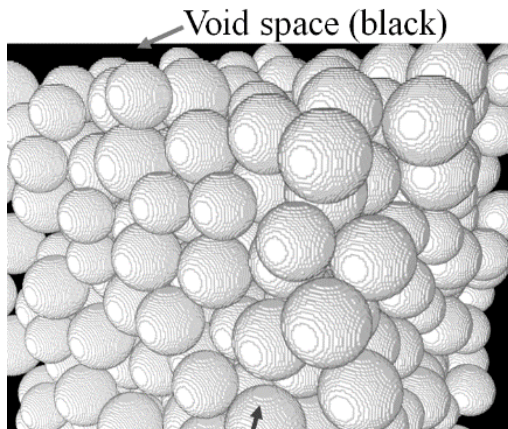


Constrictions local
maxima of distances
to particles

Comparison of constriction size distributions

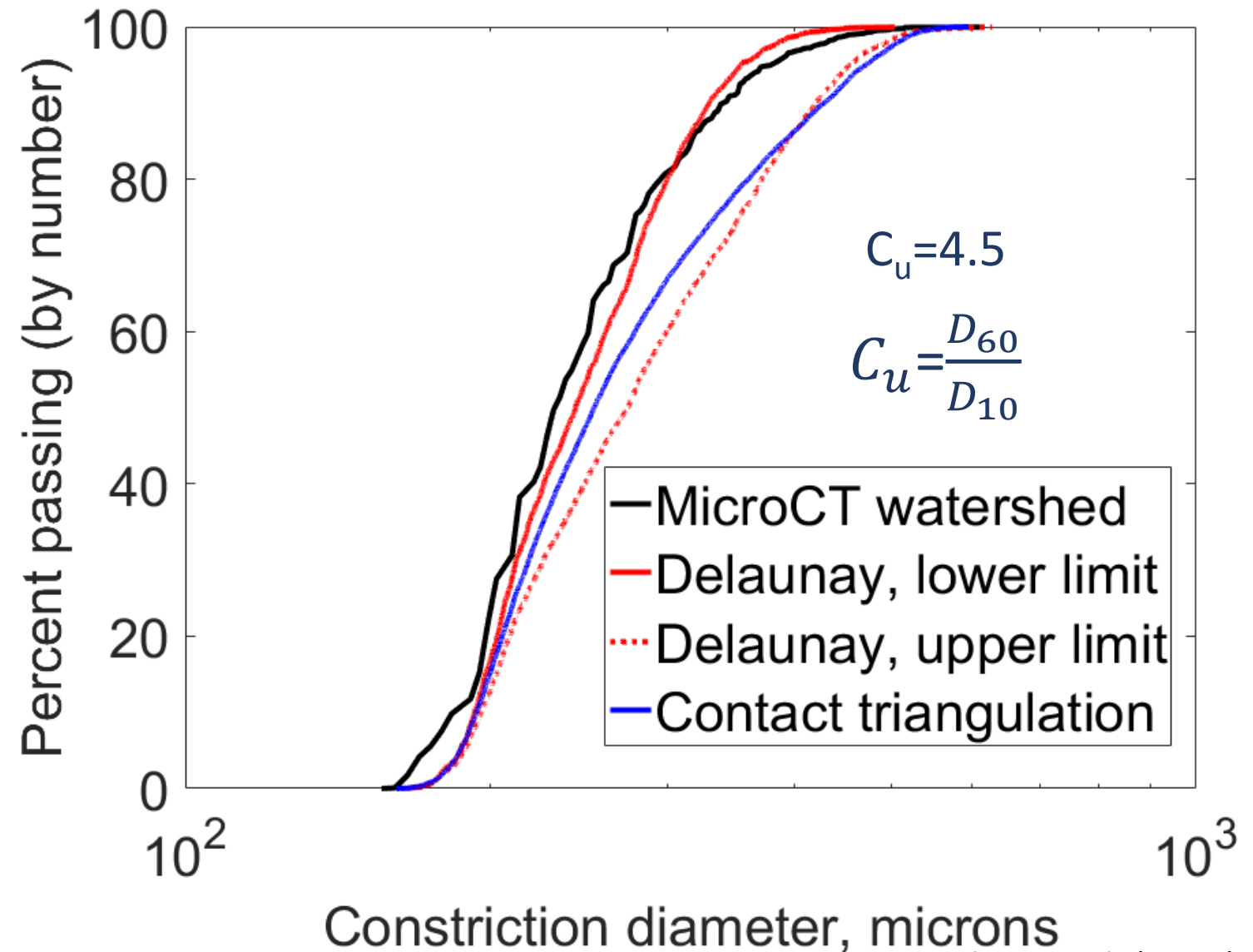


DEM Sample



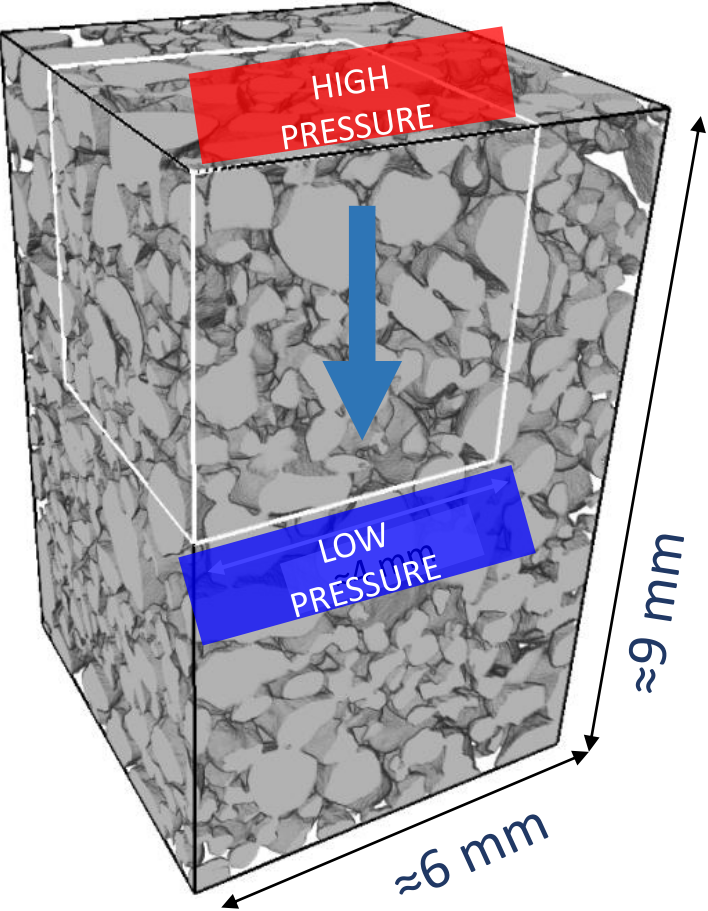
Voxelized filter particles

Voxelized DEM Sample:

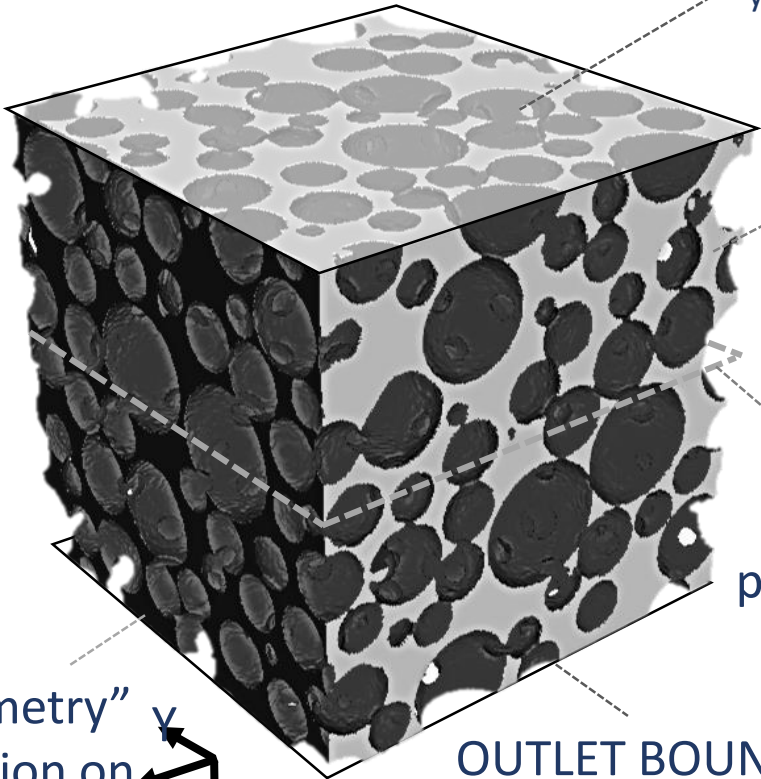


Fluid flow simulations

micro-CT
image (Cu3)



CFD Analysis



INLET BOUNDARY:

$$P_{in} = 0.001 \text{ kPa}$$
$$V_x = 0$$
$$V_y = 0$$

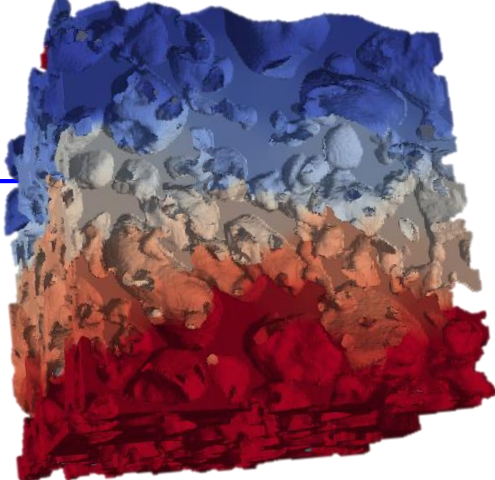
“No slip”
condition
on particle
surfaces

Slice,
perpendicular
to flow
direction

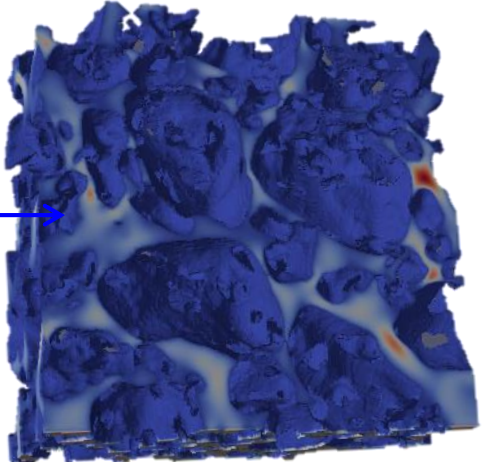
OUTLET BOUNDARY:

$$P_{out} = 0 \text{ kPa}$$
$$V_x = 0$$
$$V_y = 0$$

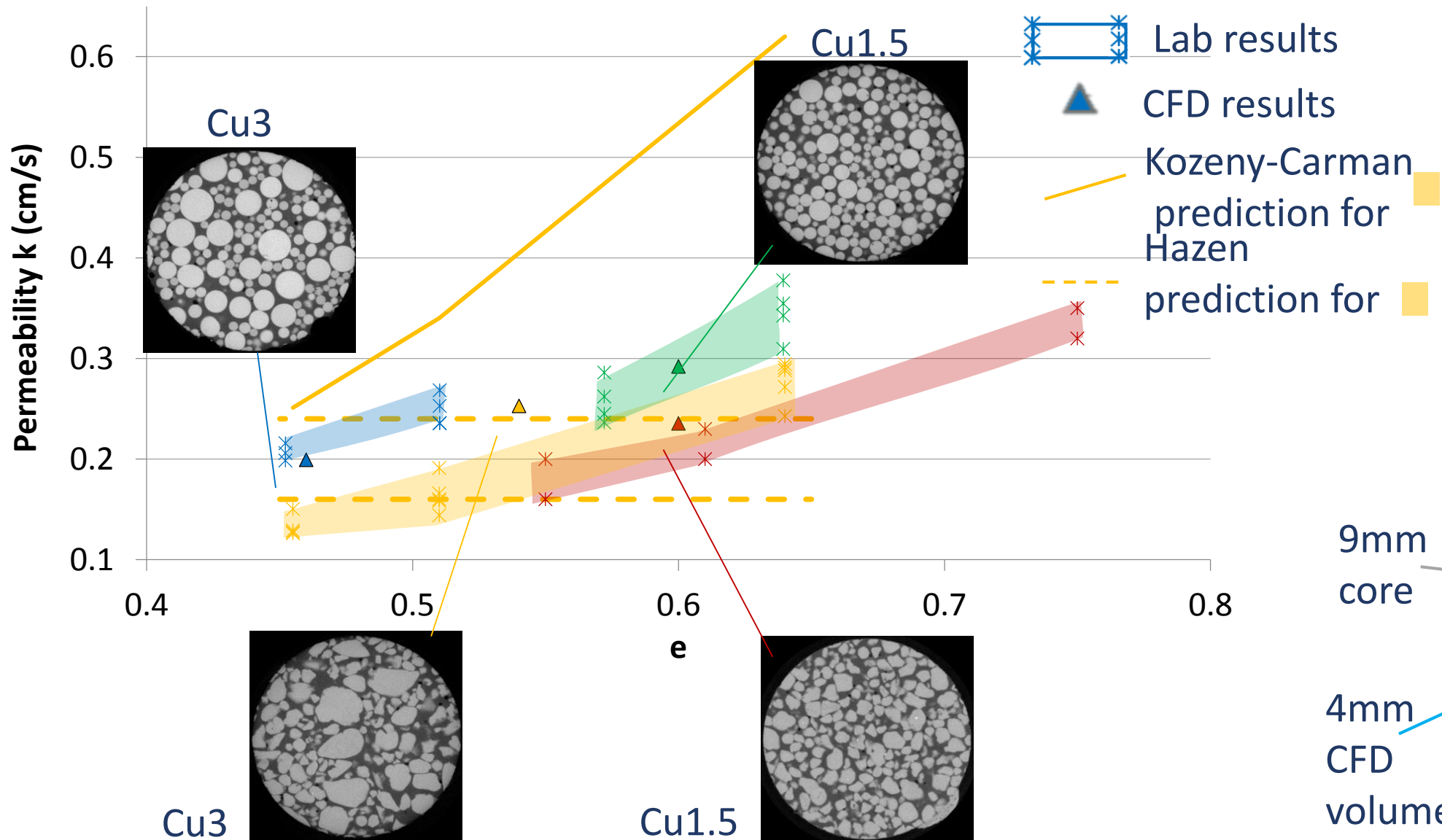
Pressures



Velocities



Comparison of CFD and permeameter data

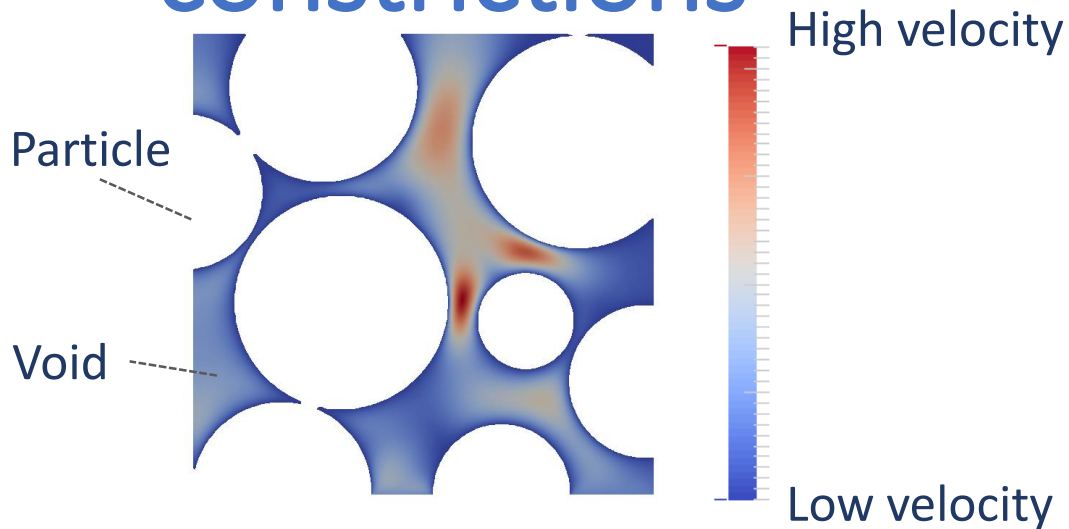


75mm permeameter

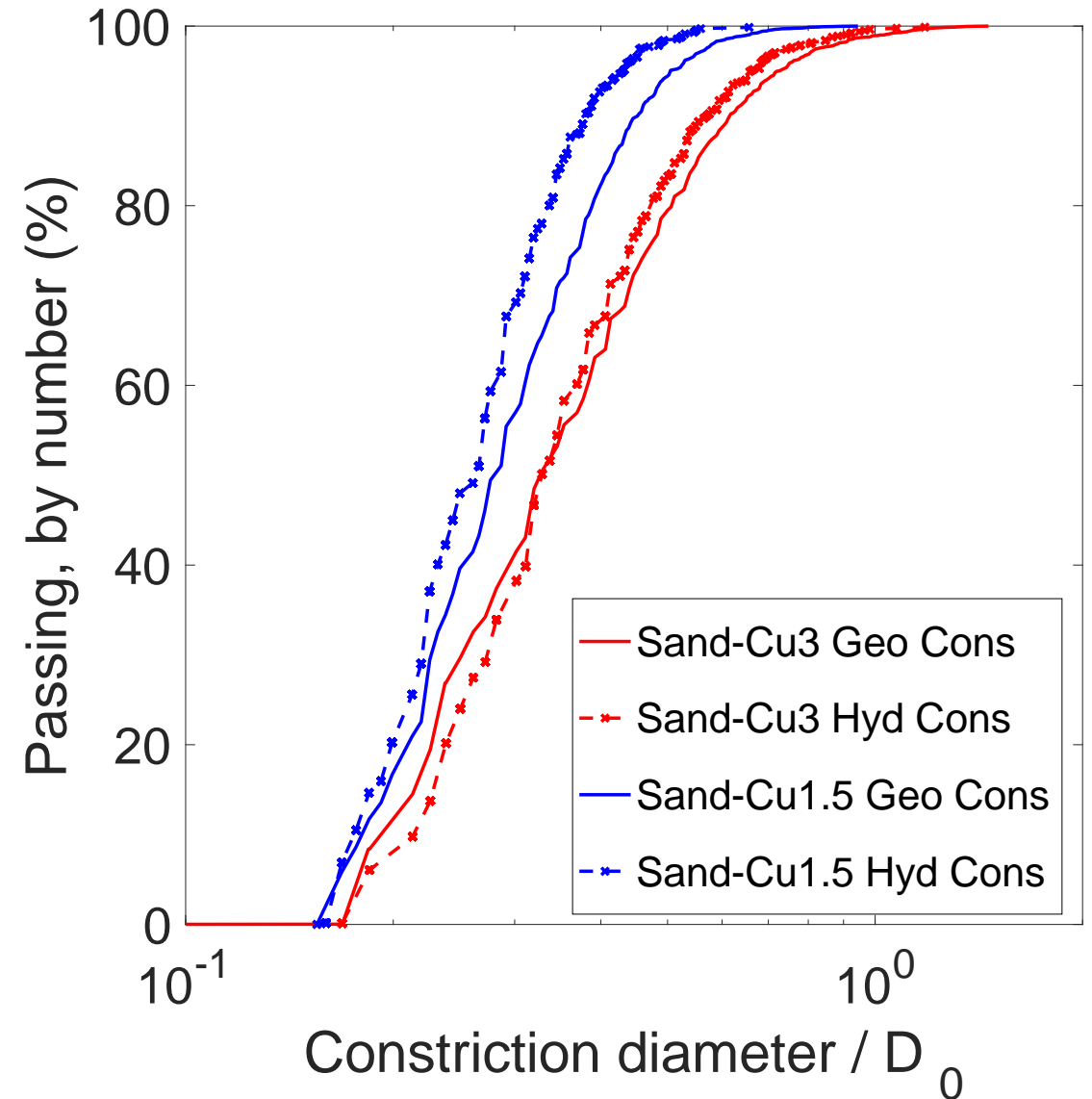
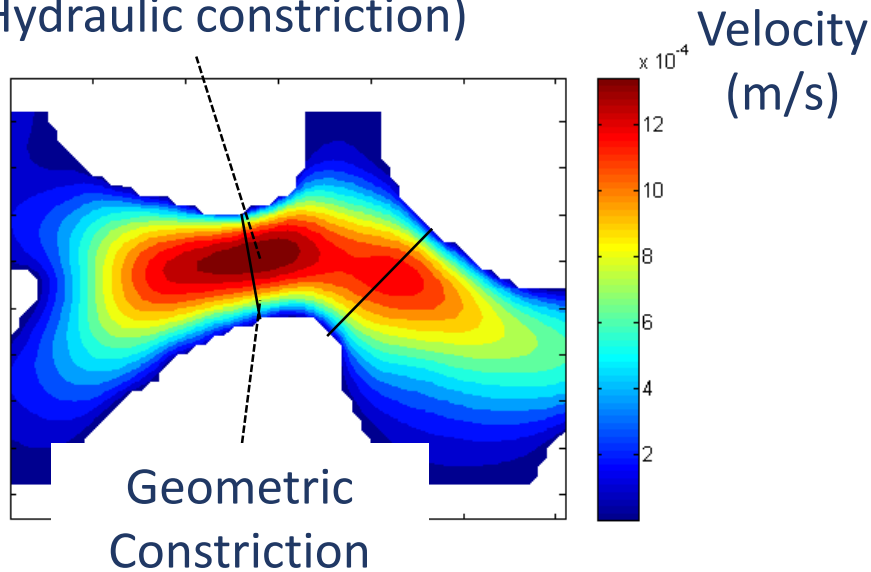


9mm core
4mm CFD volume
38mm triaxial

Comparison of geometric and hydraulic constrictions

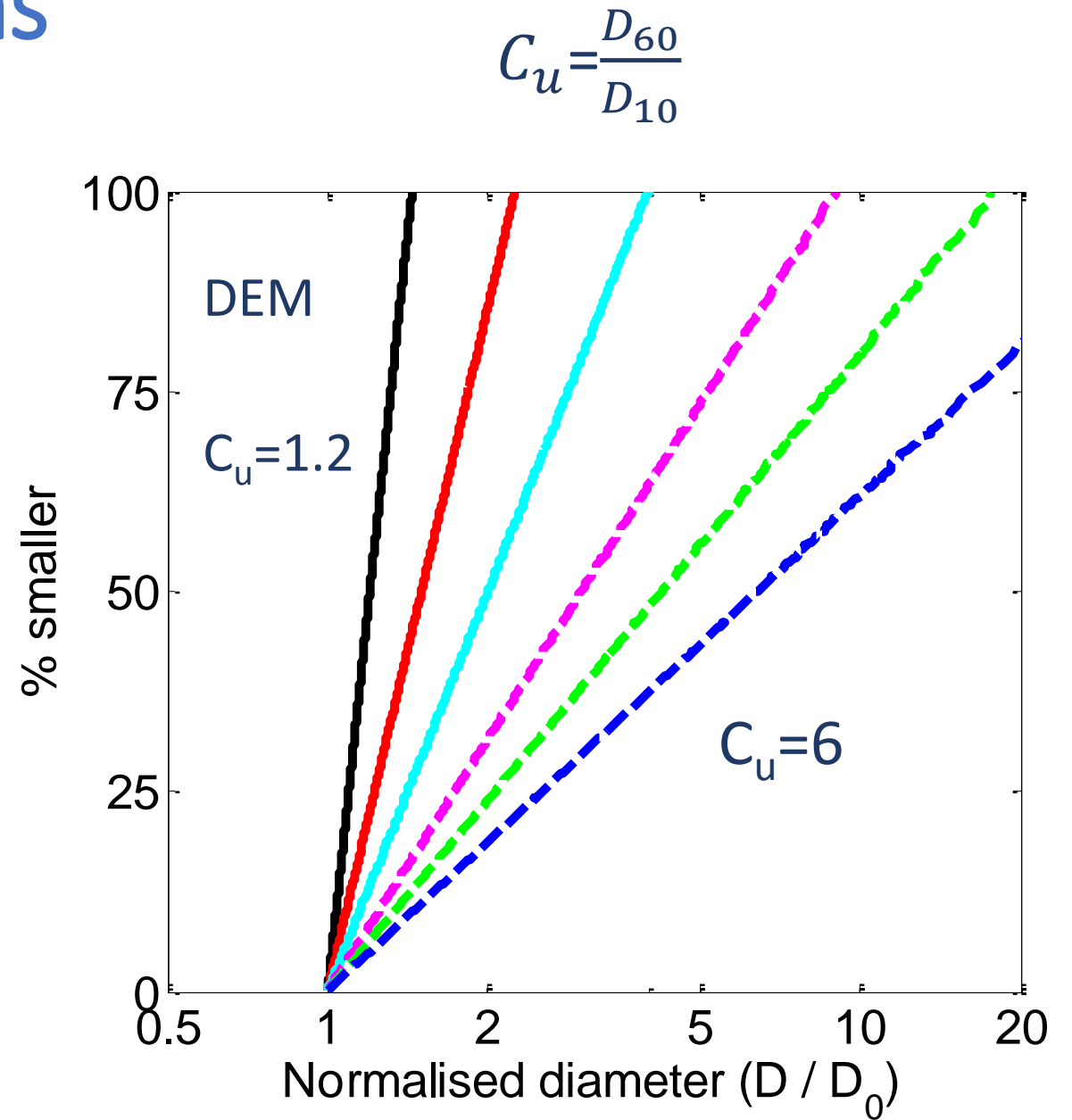
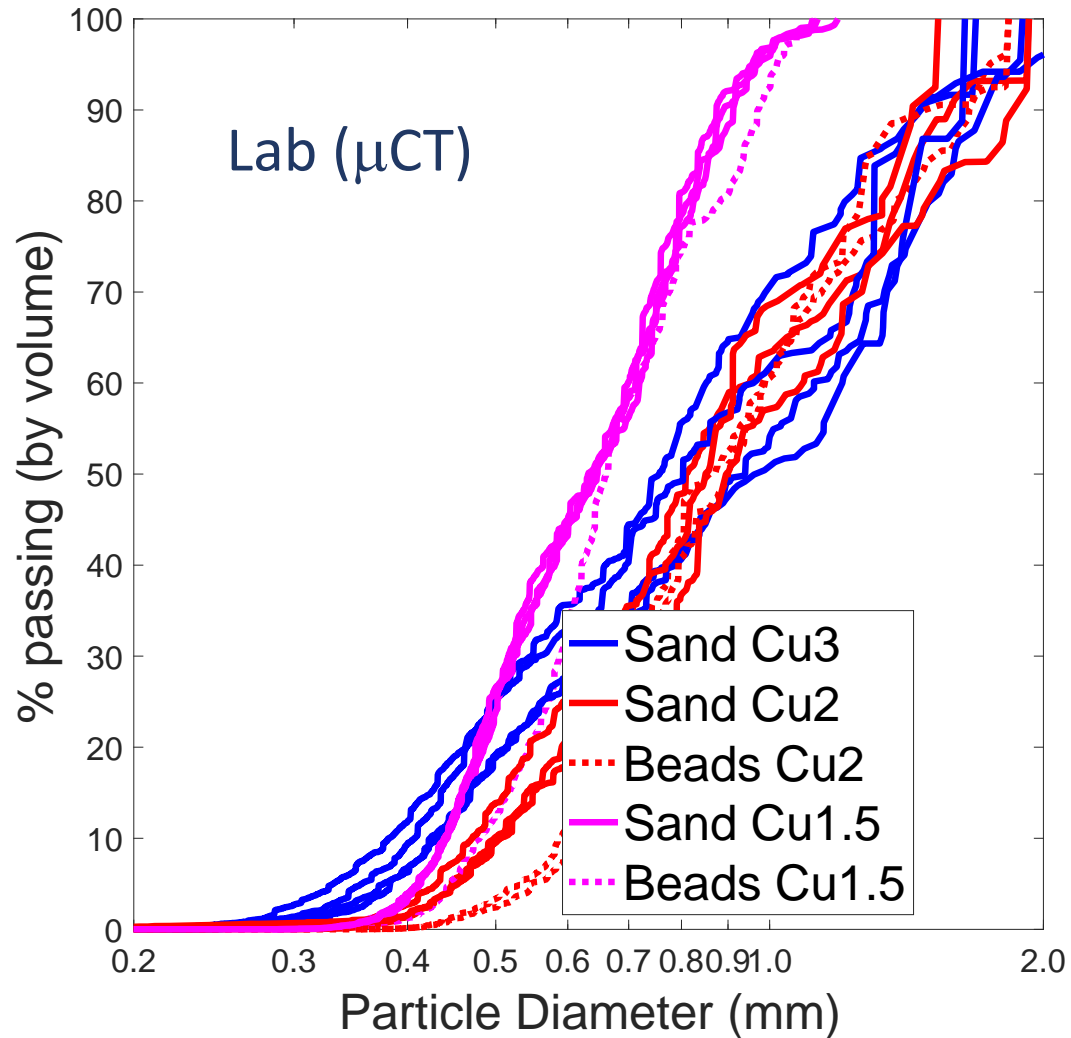


Velocity maximum
(= Hydraulic constriction)

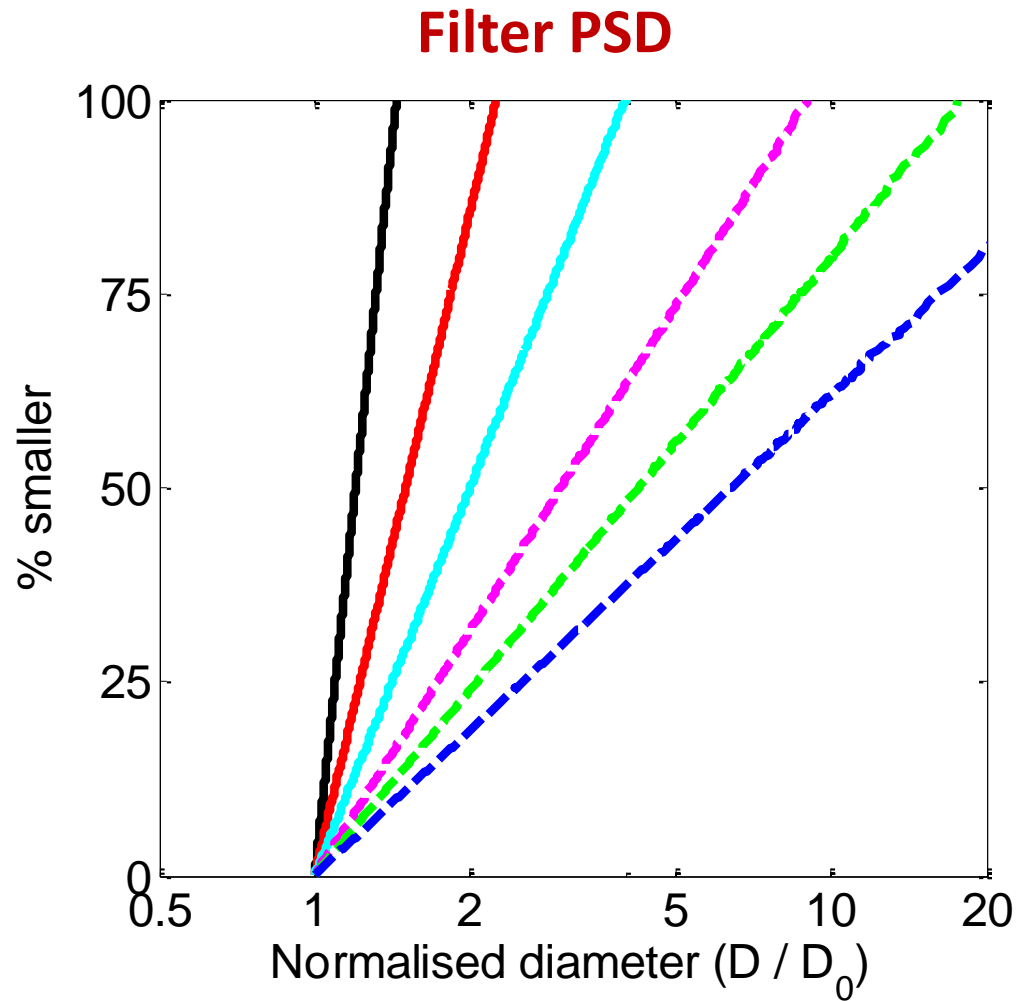


Taylor et al. (2017)

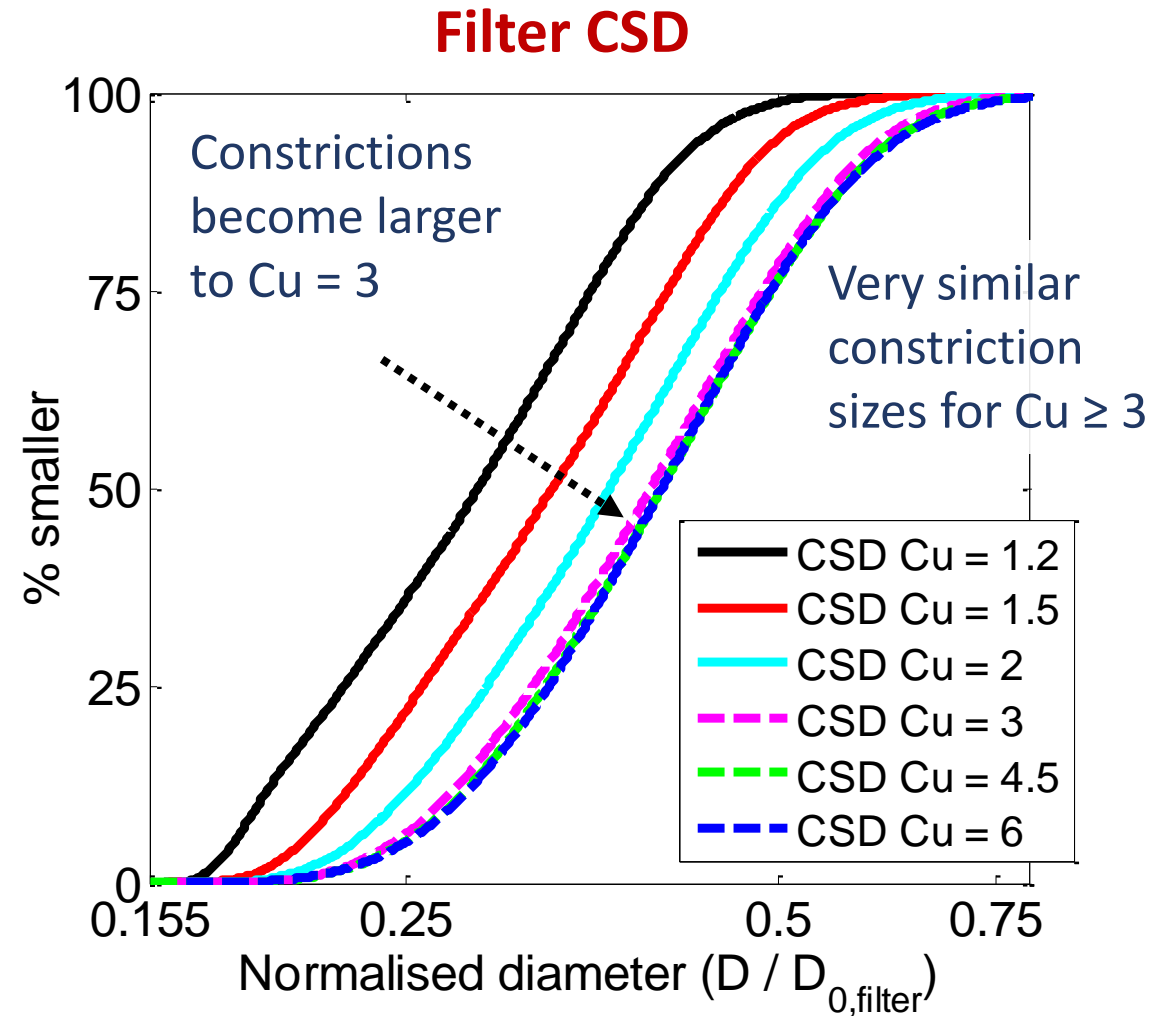
Particle Size Distributions



Constriction Size Distributions (DEM)



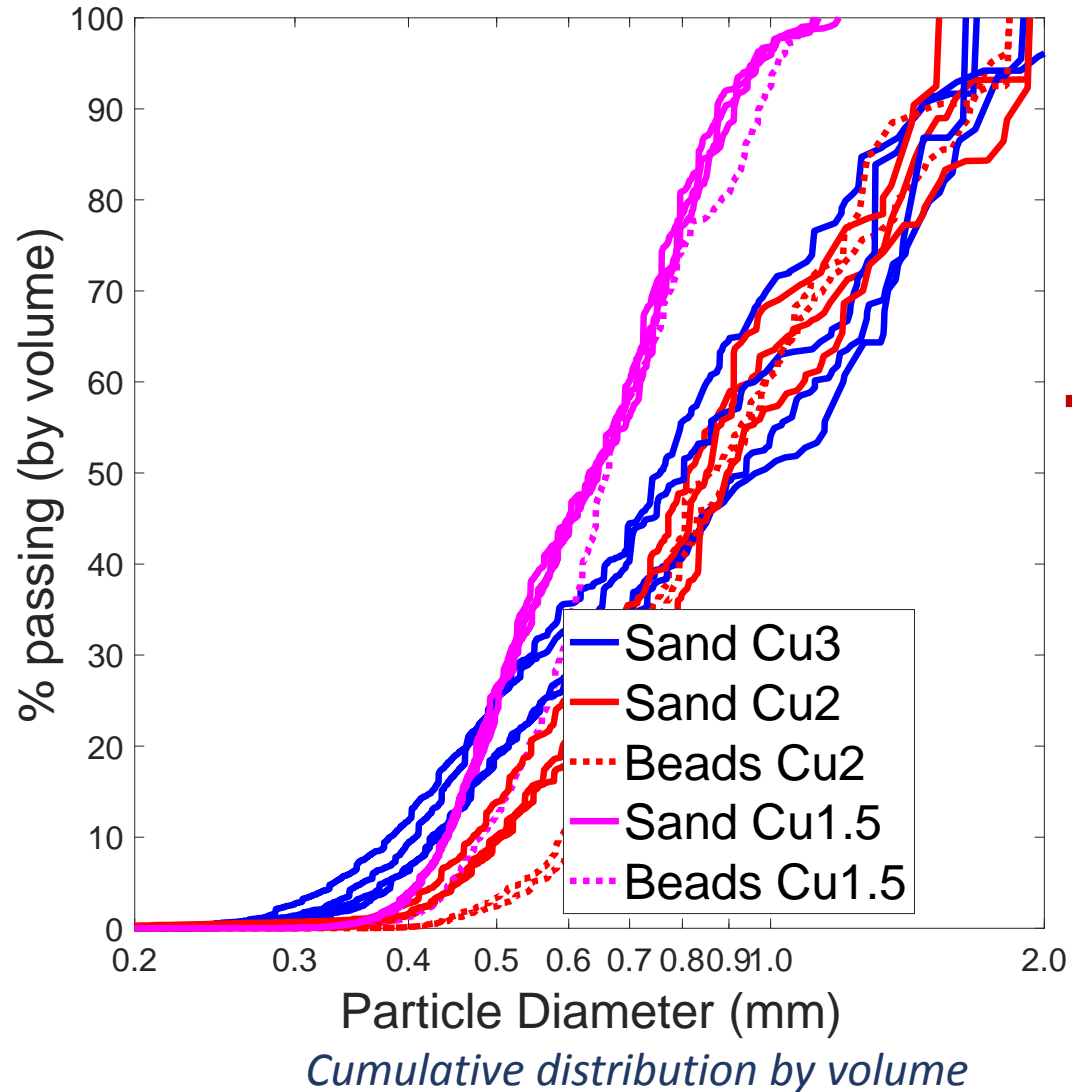
Cumulative distribution by volume



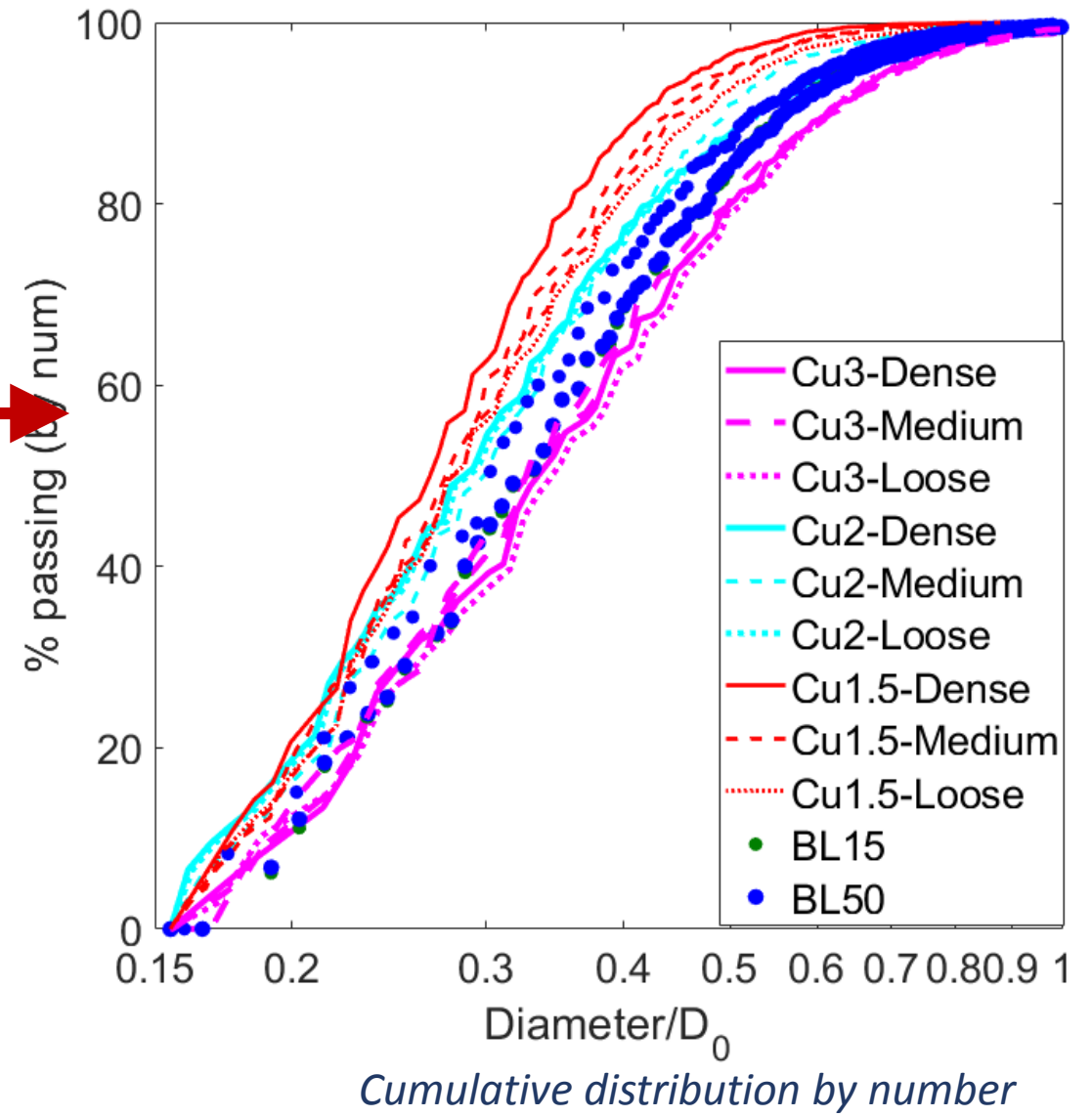
Cumulative distribution by number

Constriction size distributions (CSDs) μ CT

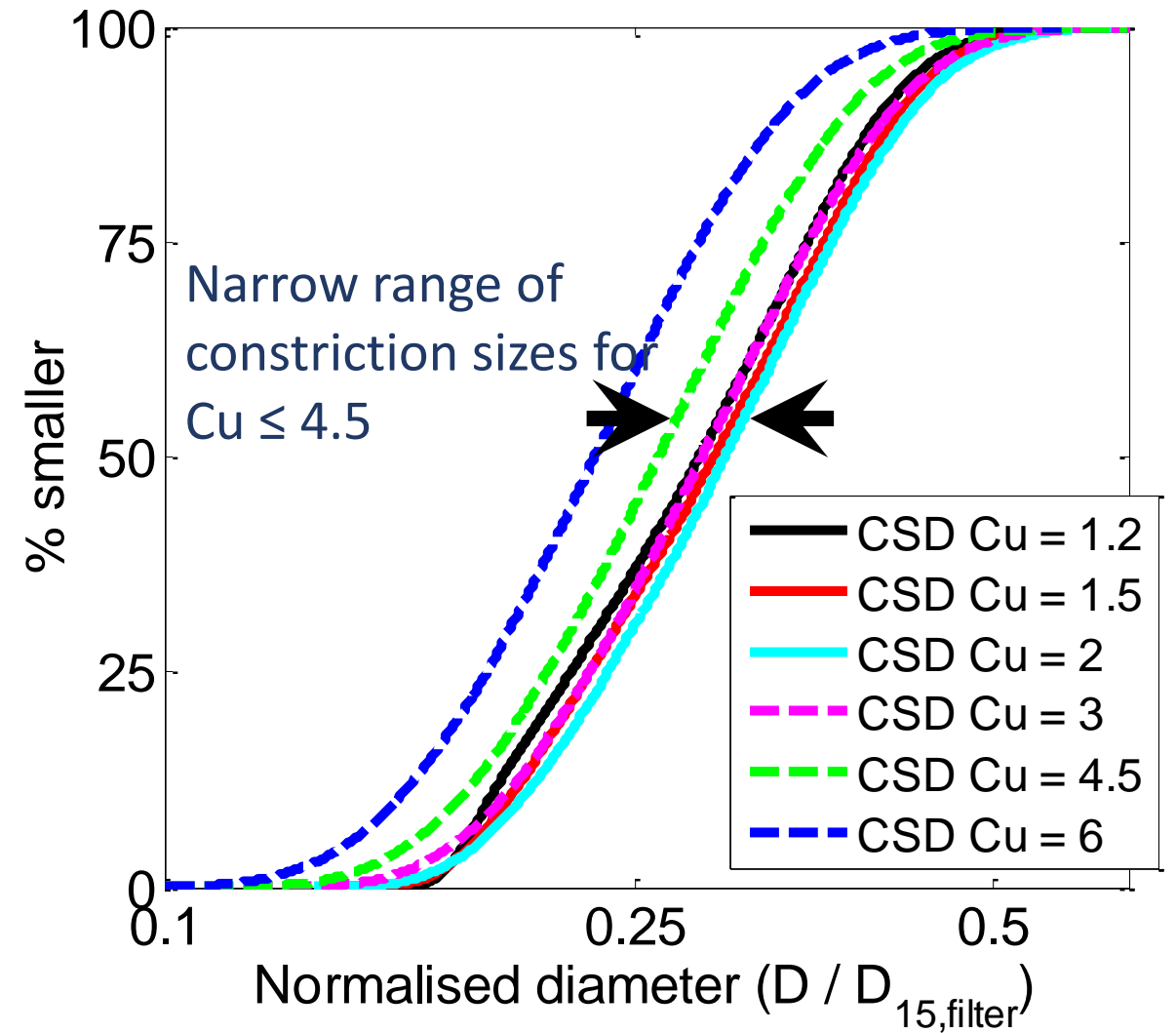
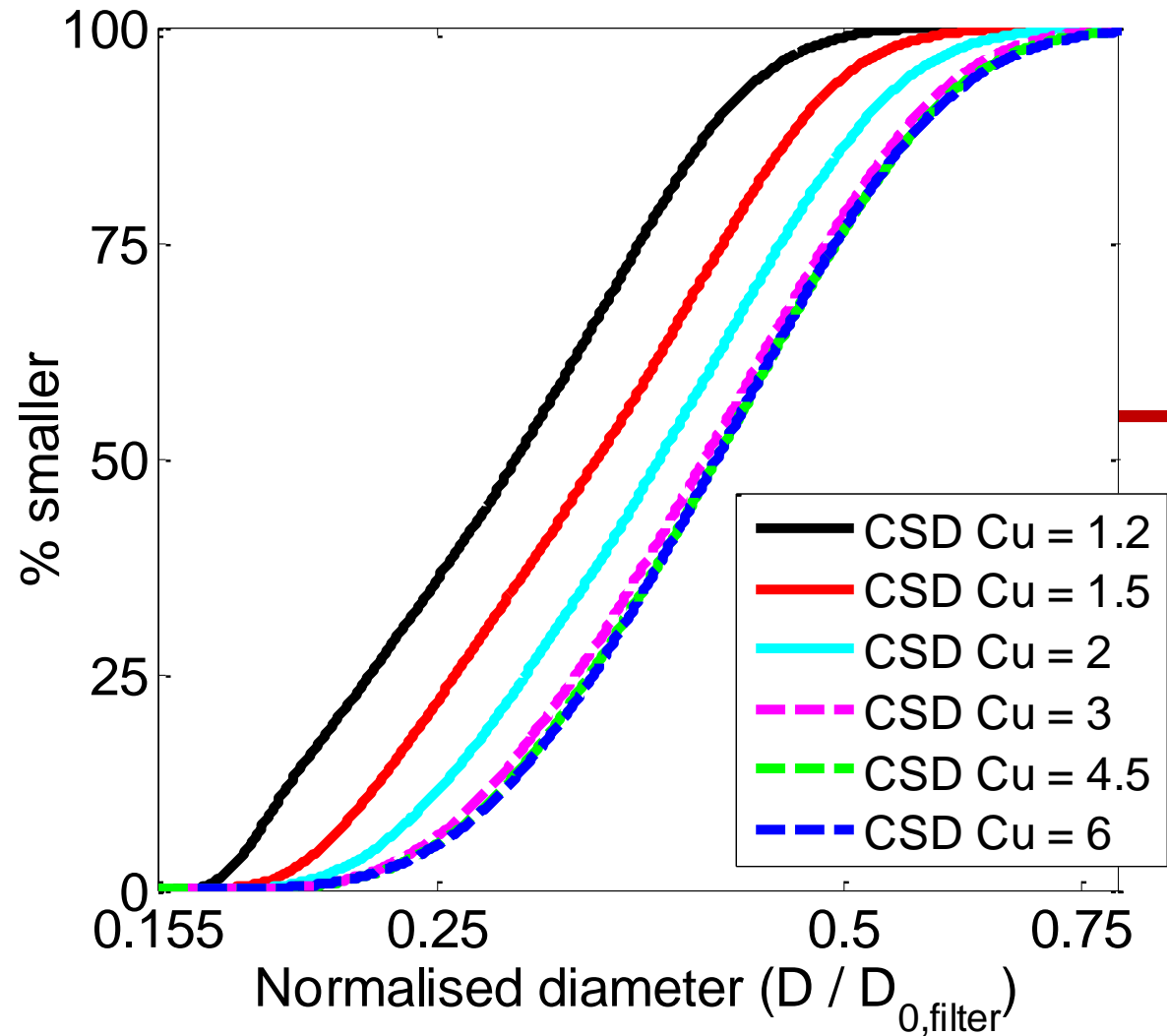
Filter PSD



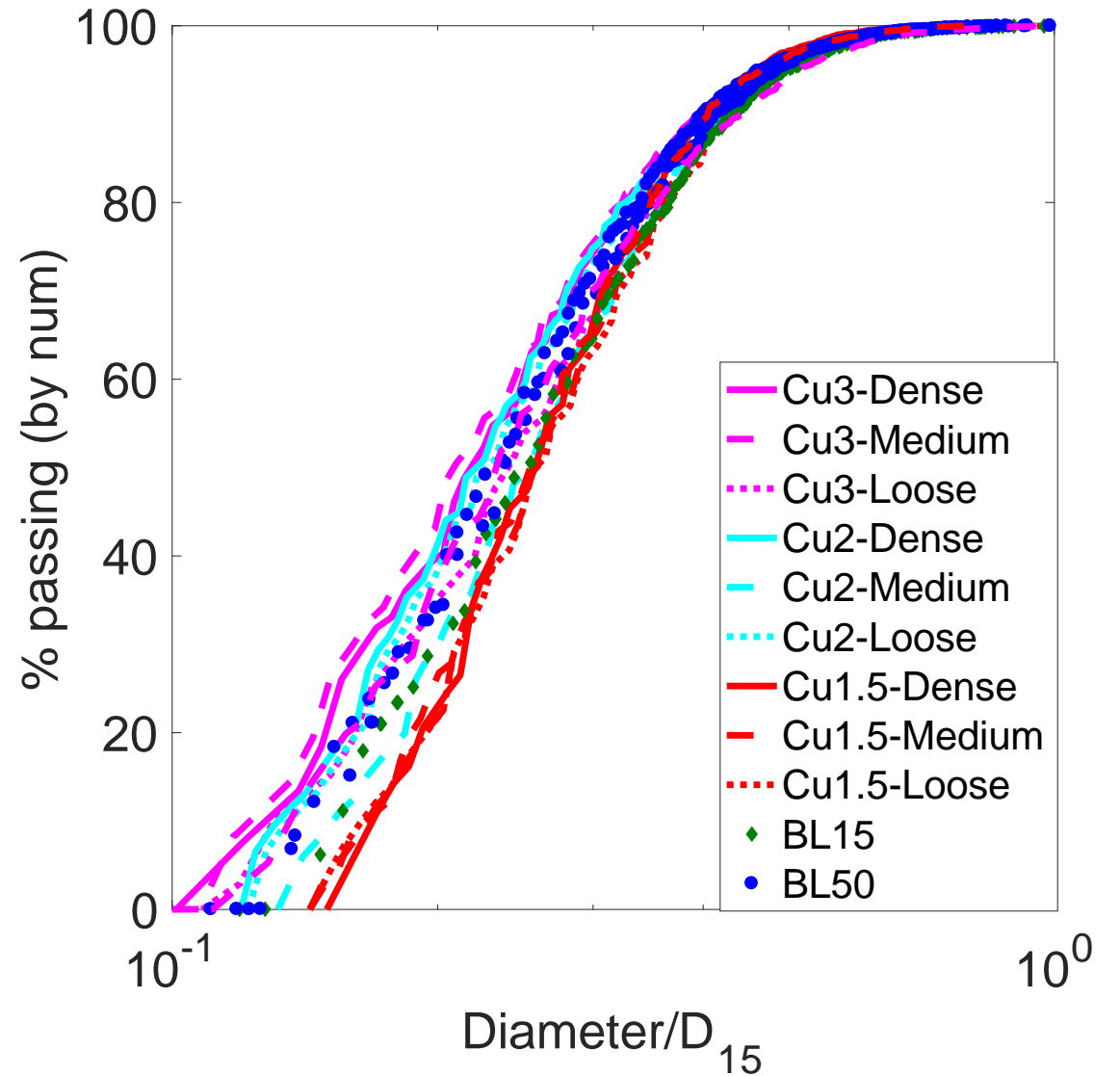
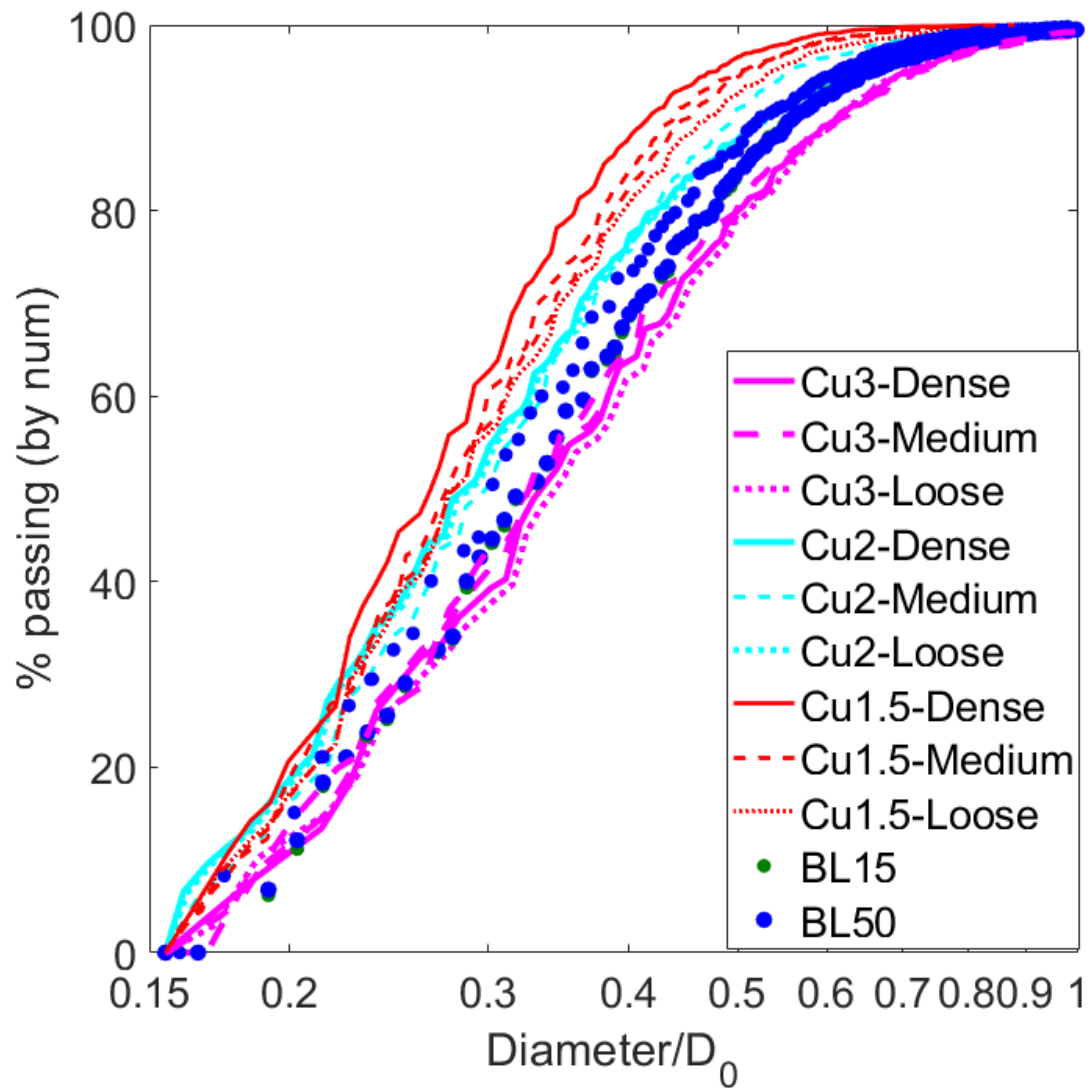
Filter CSD



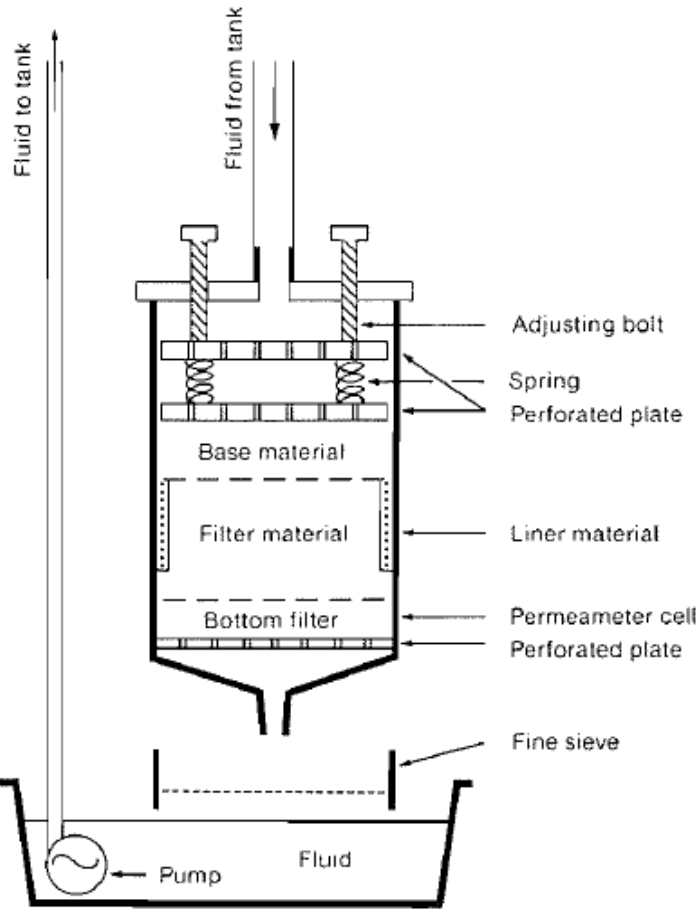
Constriction Size Distributions (DEM)



Constriction Sizes μ CT

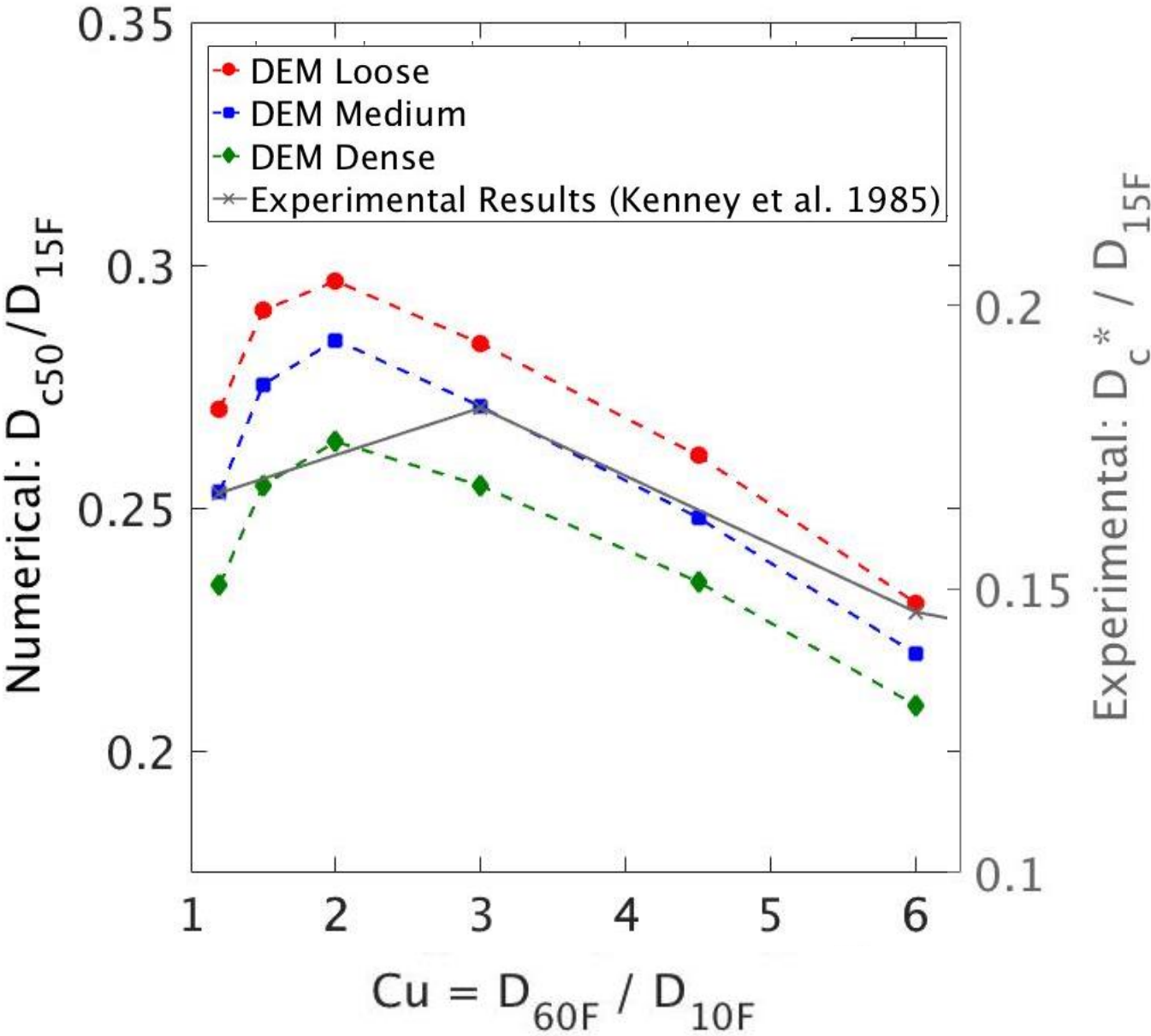


Controlling constriction size

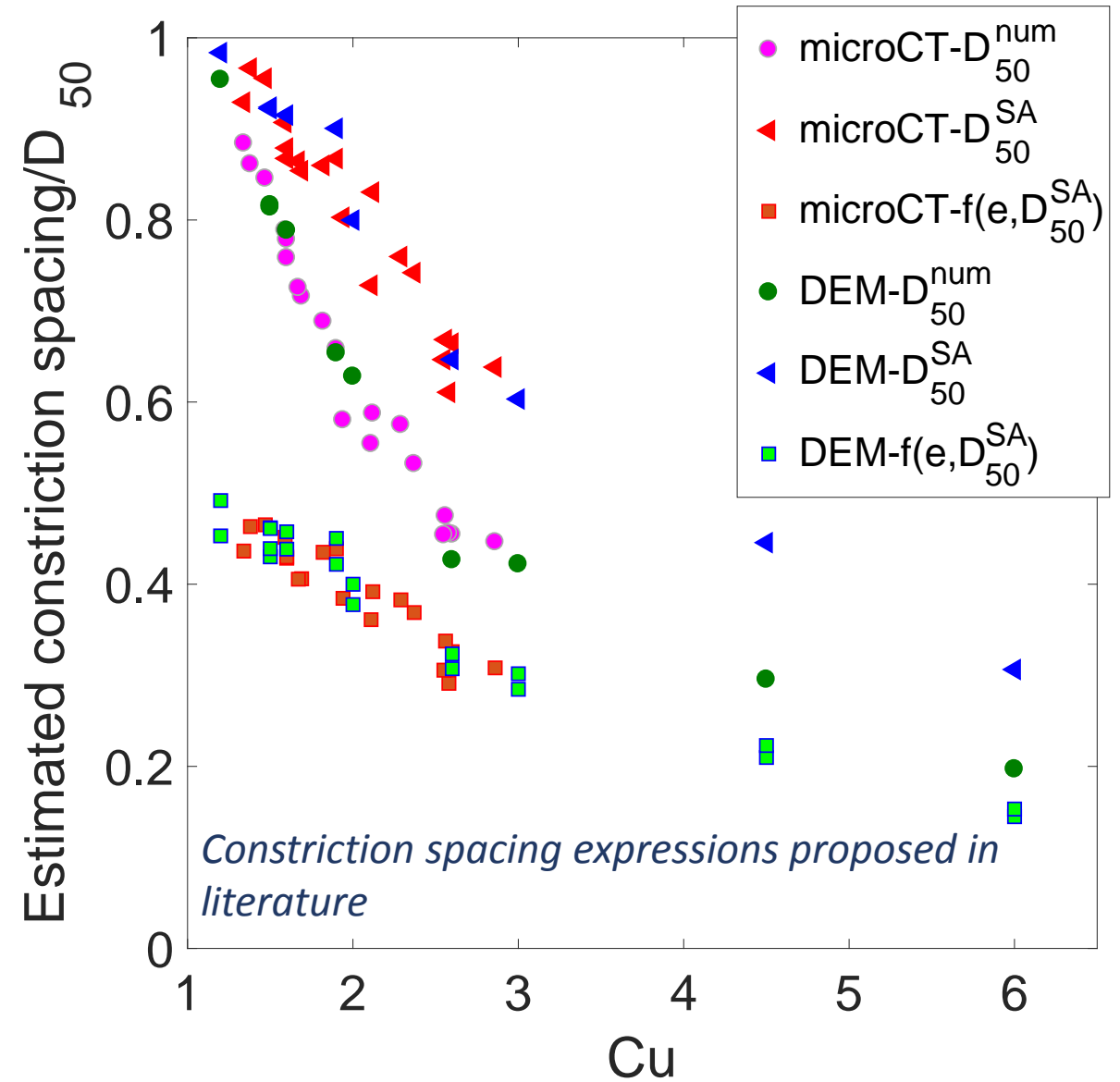
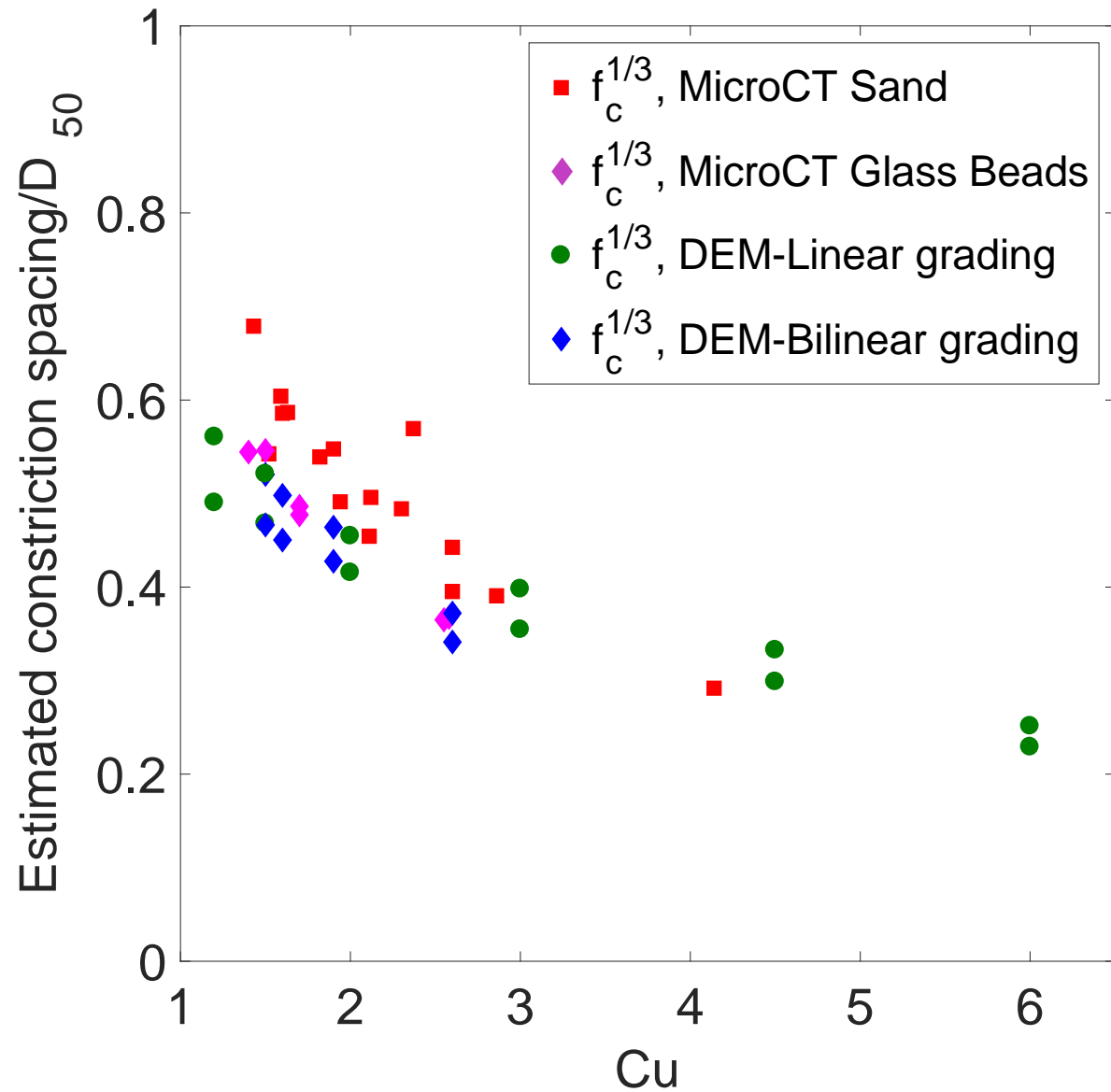


Kenney et al (1985): Base- Filter tests: Base-filer tests

D_c^* = controlling constriction diameter = largest particle that can pass through filter



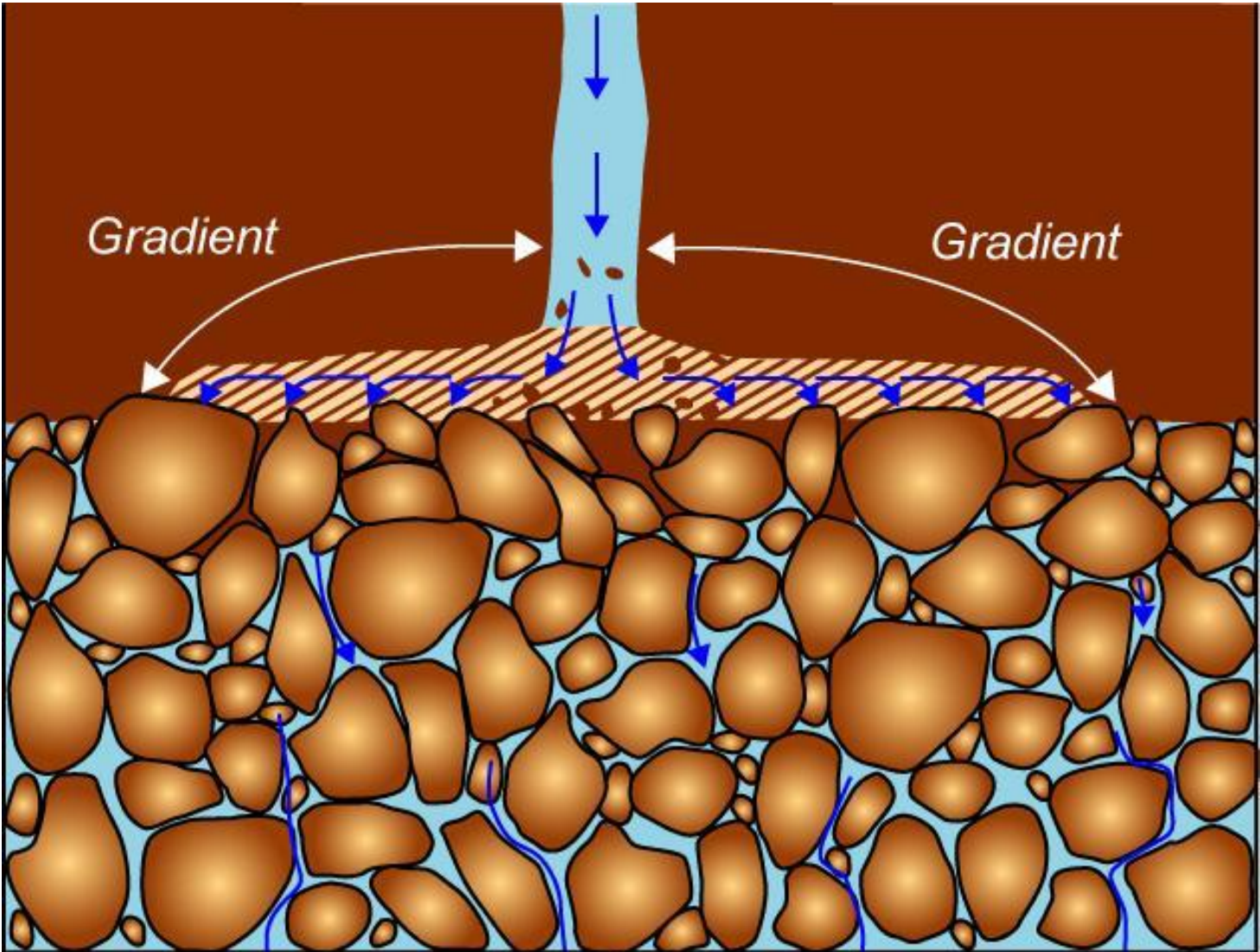
Filtration – Constriction Density / Spacing



Research questions: Filter retention

- What is the relationship between the size of constrictions and D_{15F} ?
- **Does particle scale analysis support use of the ratio D_{15F}/D_{85B} in design?**

Retention of Base



Base material:
clay or silt with
small particle
size

Filter – large
particle size to
achieve
drainage

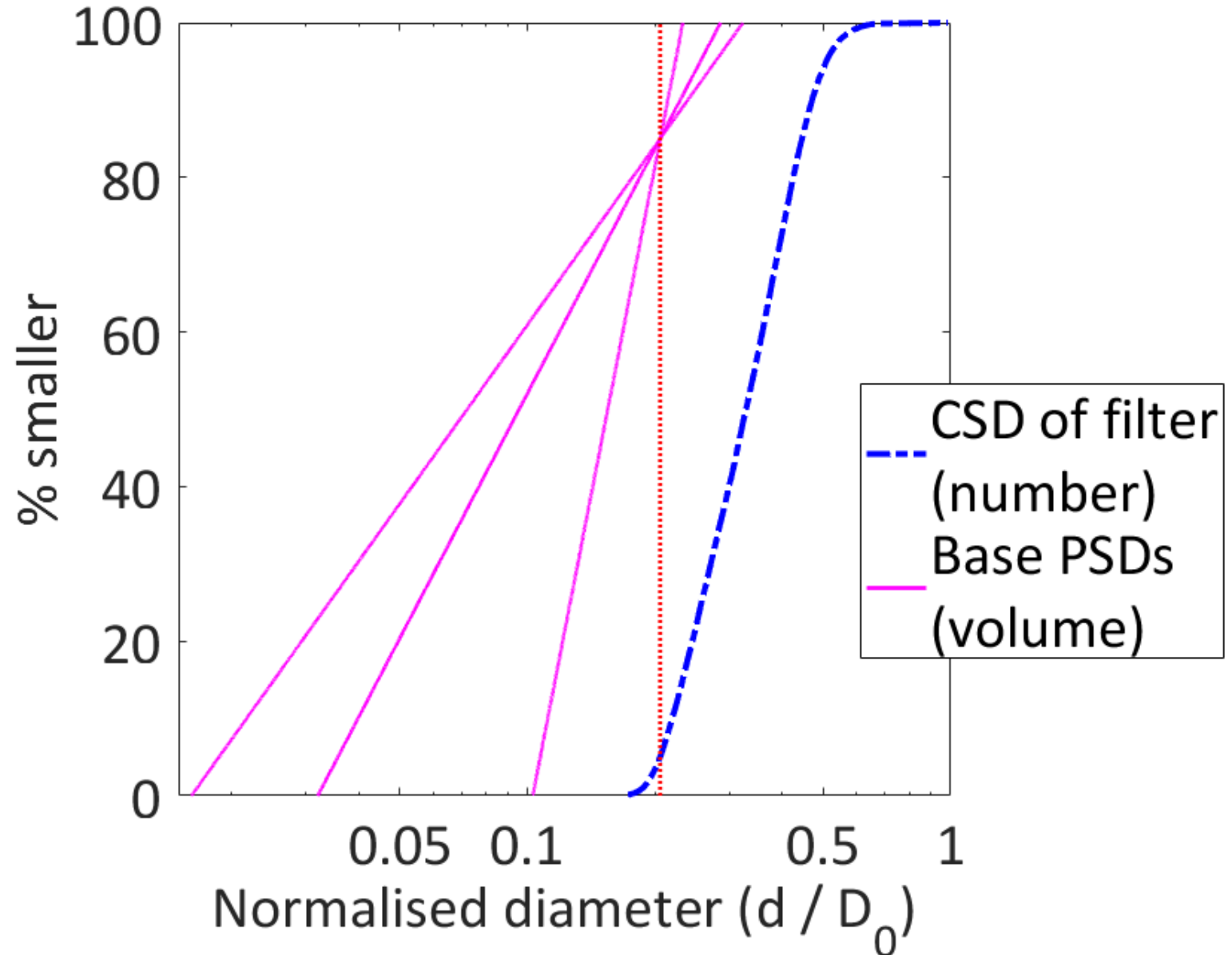
(FEMA, 2011)

Retention – Network model

Can't judge a filter's effectiveness simply by visual comparison of the CSD of the filter and the PSD of the base material to be retained

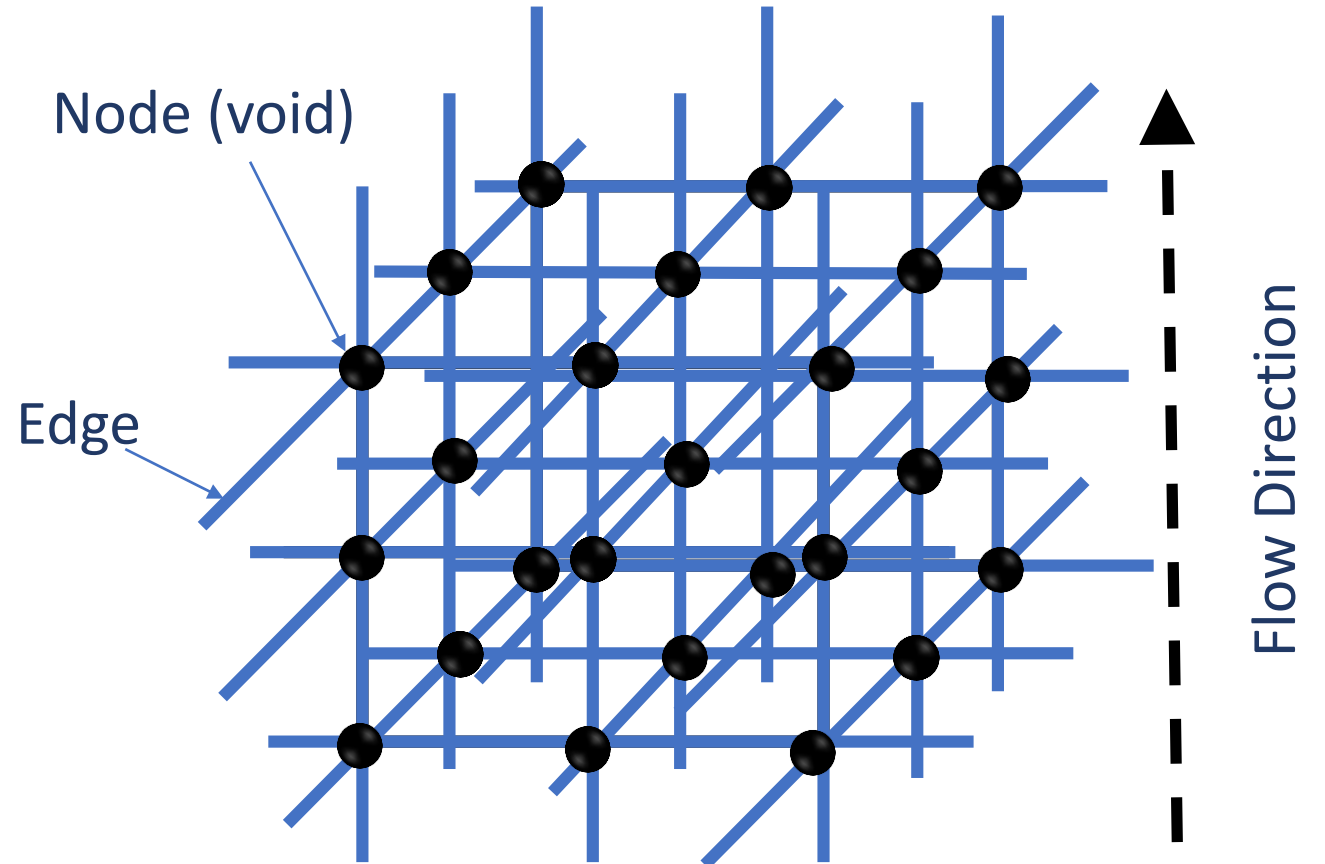
CSD – cumulative distribution by number

PSD – cumulative distribution by volume



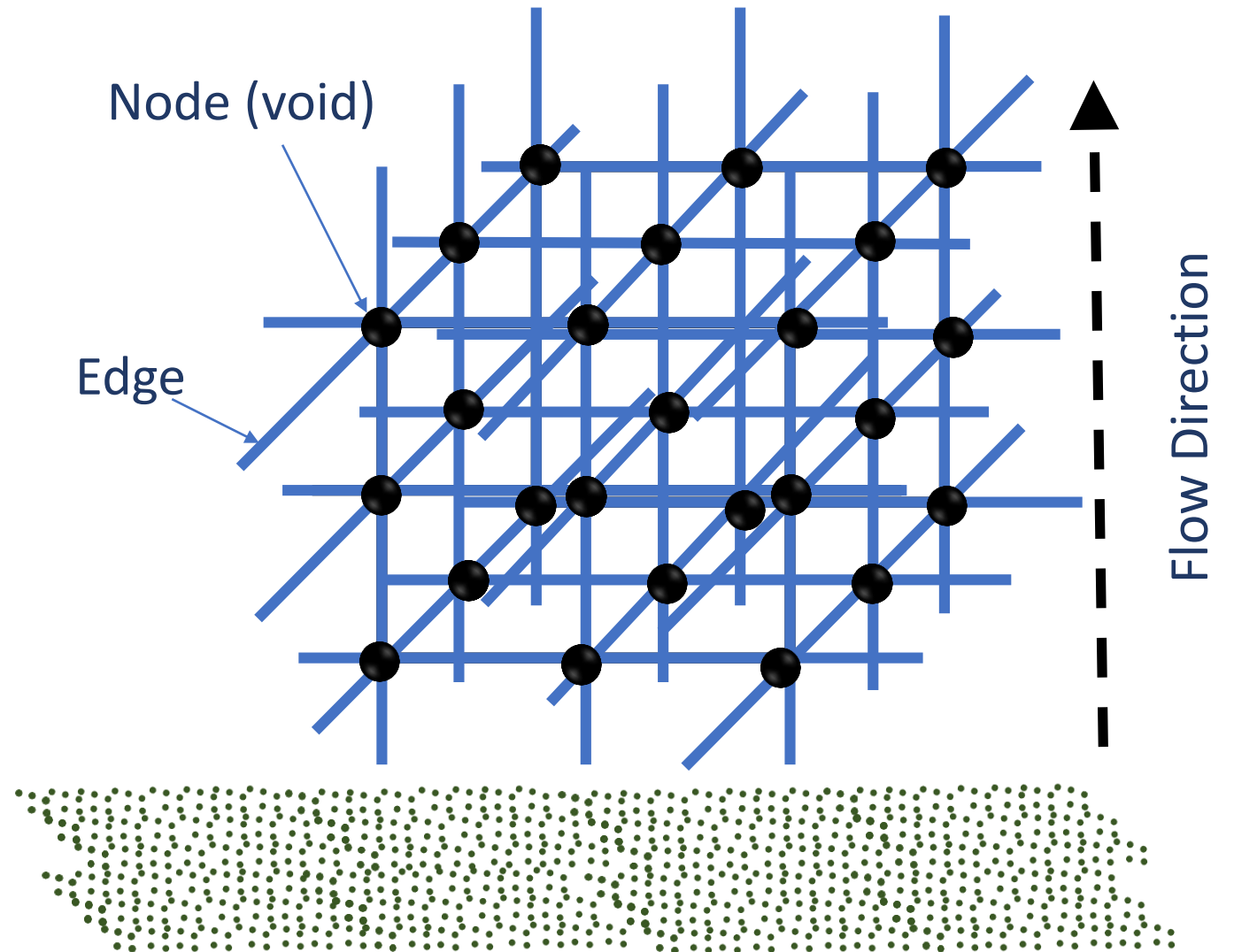
Filtration – Network model

- Network model – lattice topology
- Nodes = individual voids
- Edges = inter void connections
- Edge diameters = constriction diameters



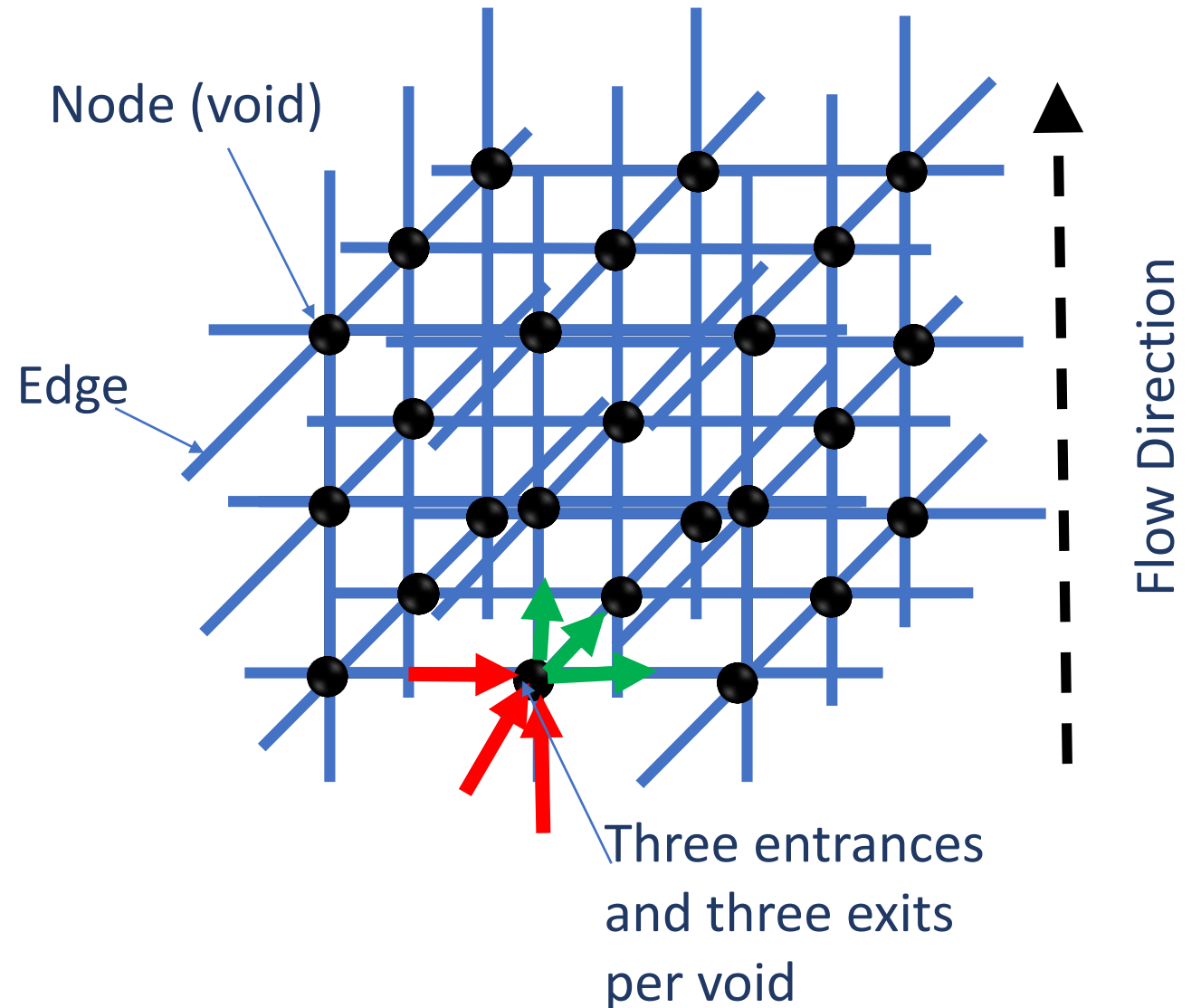
Filtration – Network model

- Simulates migration of finer base particles through network
- Fluid flow not explicitly considered
- Simple algorithm means up to 400 million base particles could be considered on a desktop pc

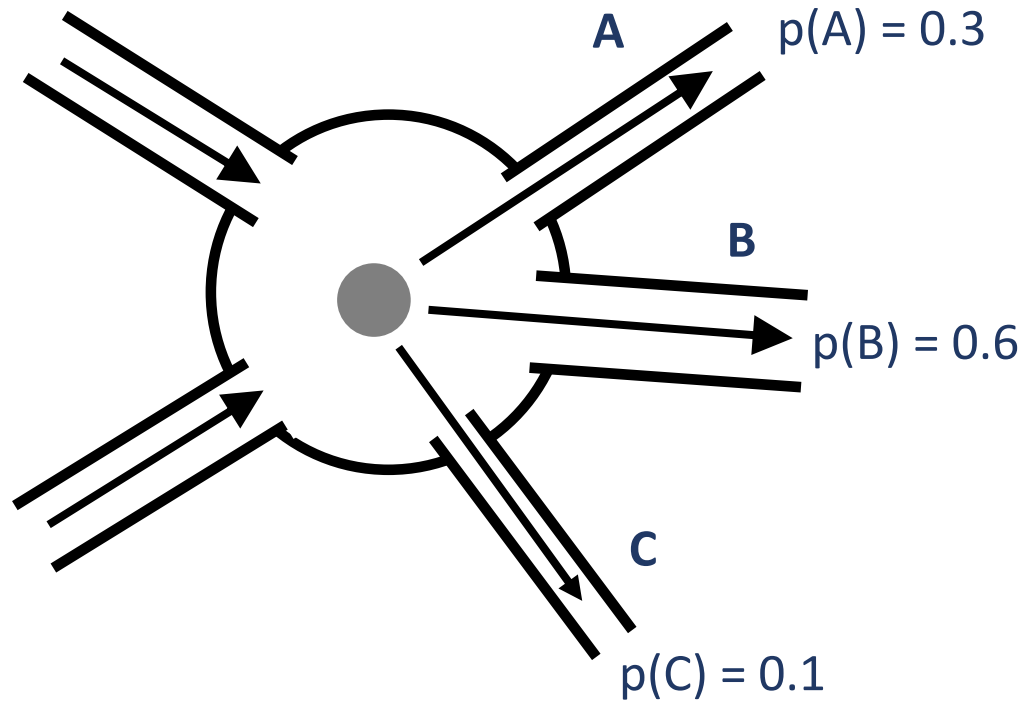


Filtration – Network model

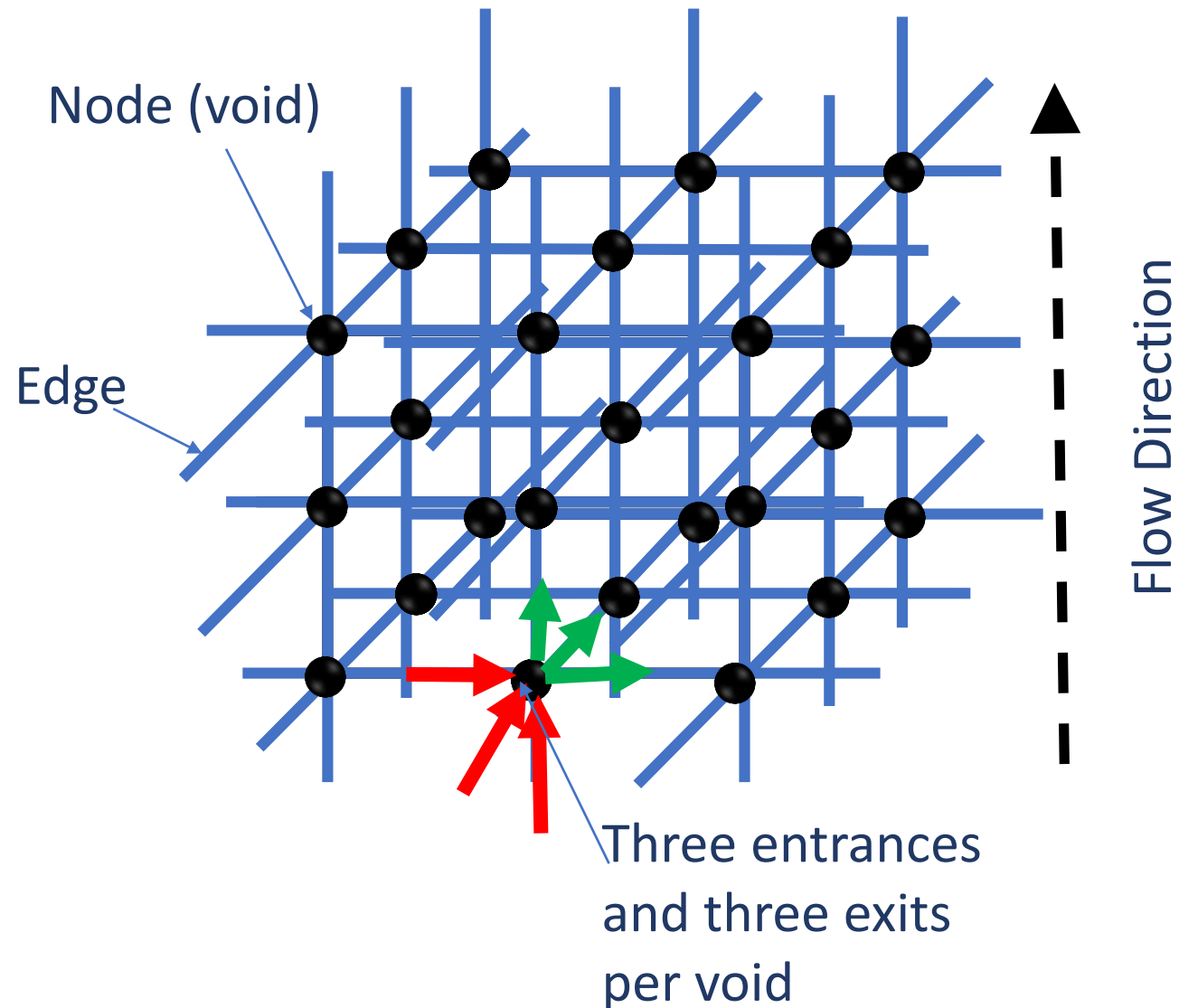
- Network model – lattice topology
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- Edges = inter void connections
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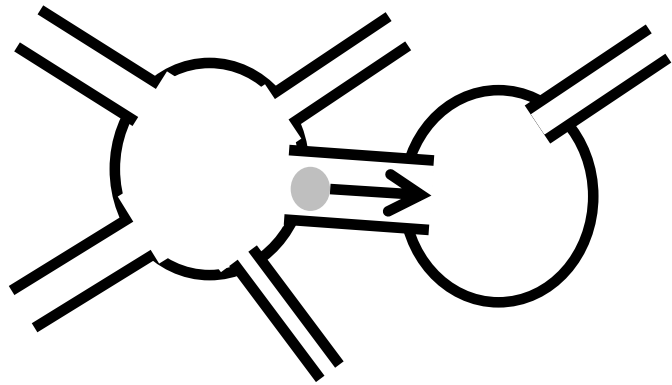
Area based random walk



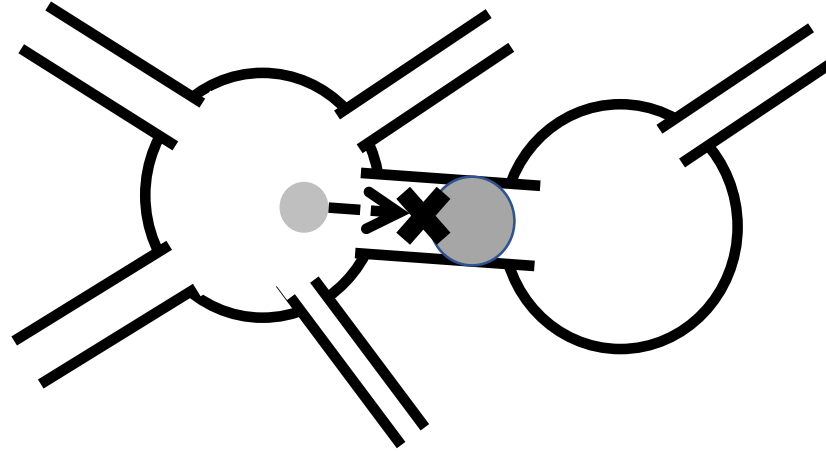
Likelihood of selecting a target edge to move through depends on constriction area



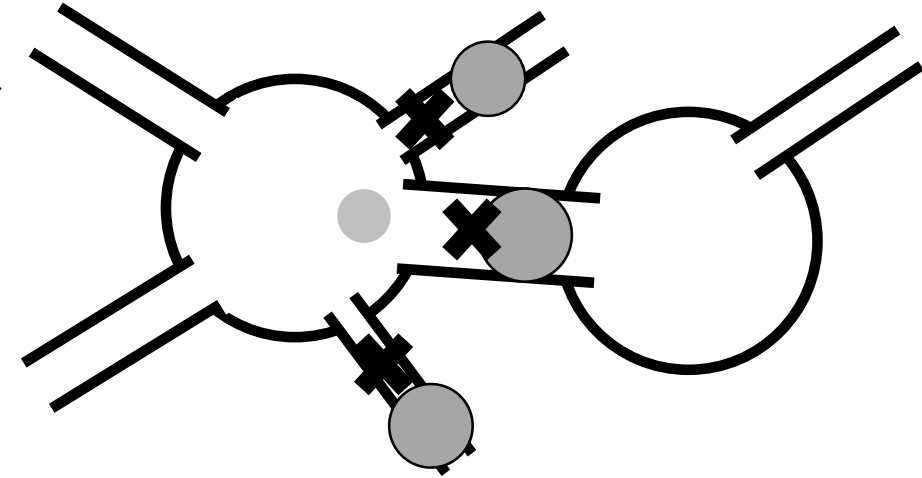
“Random walk” of base particles through network



Base particle moves through constriction

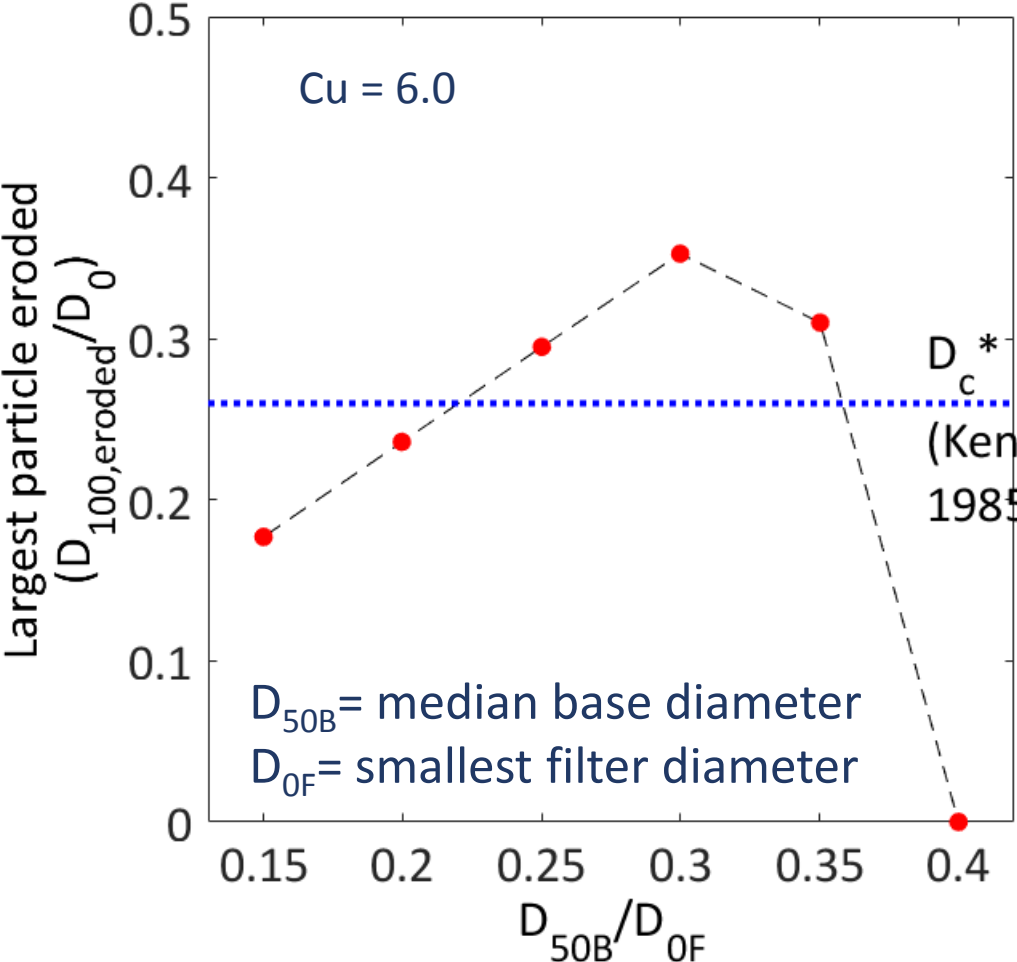


Base particle retained + constriction blocked

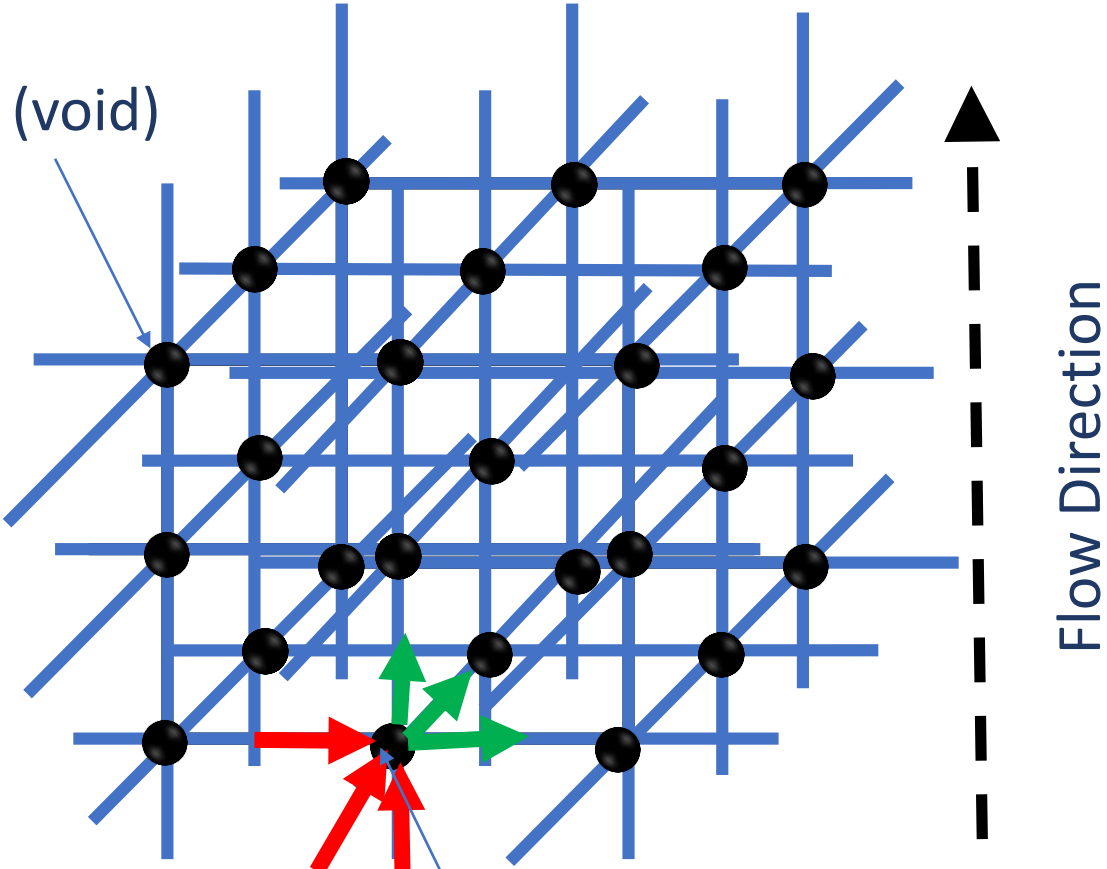


Base particle retained in void

Filtration – Network model



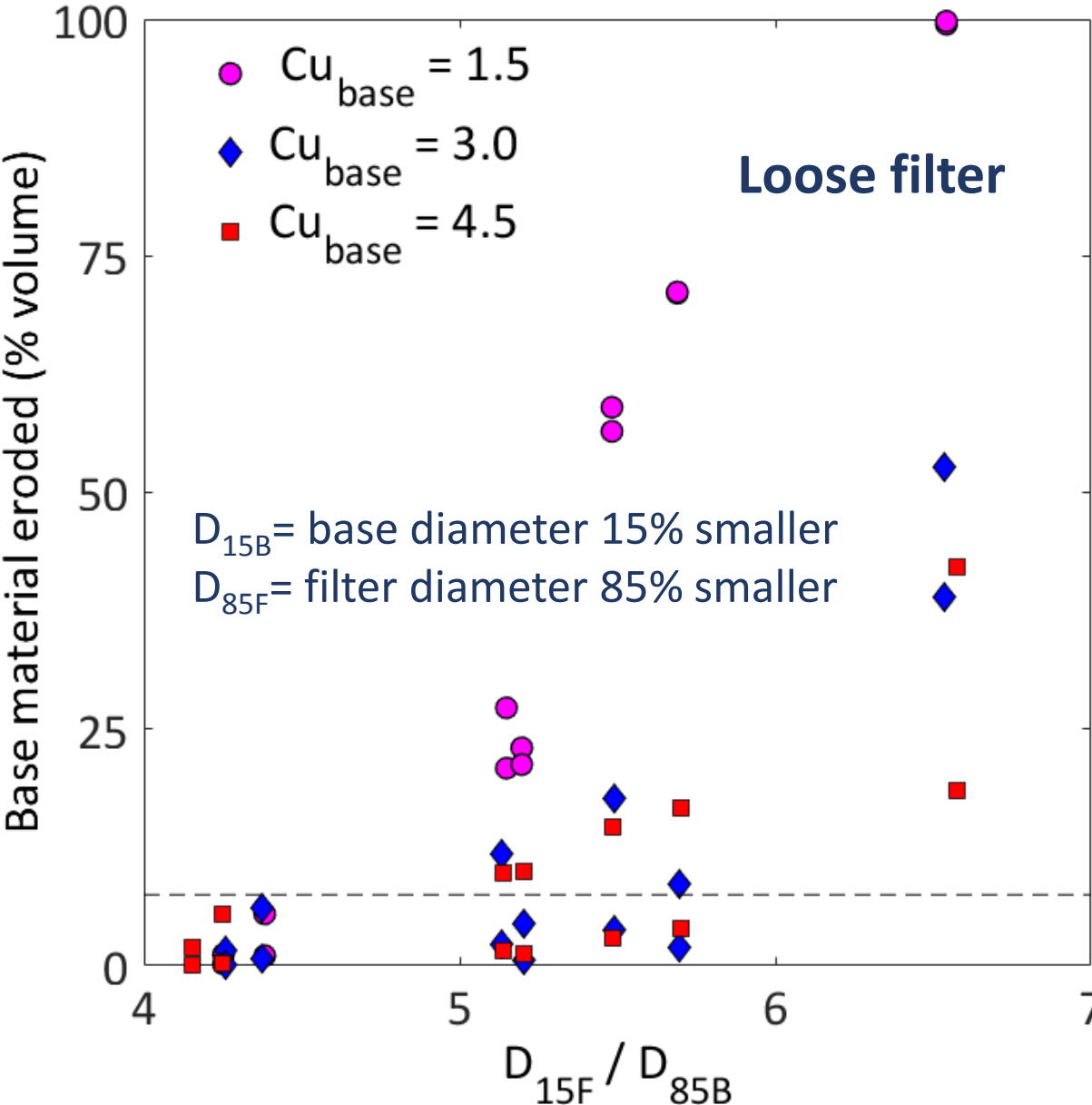
Node (void)



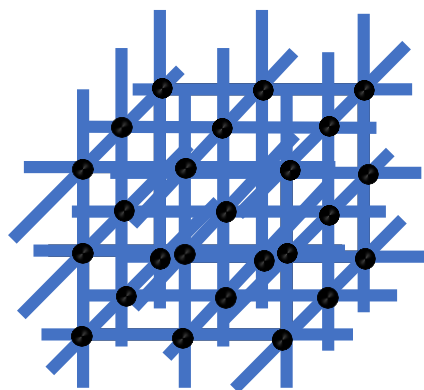
Three entrances and three exits per void

Filter $Cu = 1.2, 3, 6$, largest base particle eroded agrees with experimental data

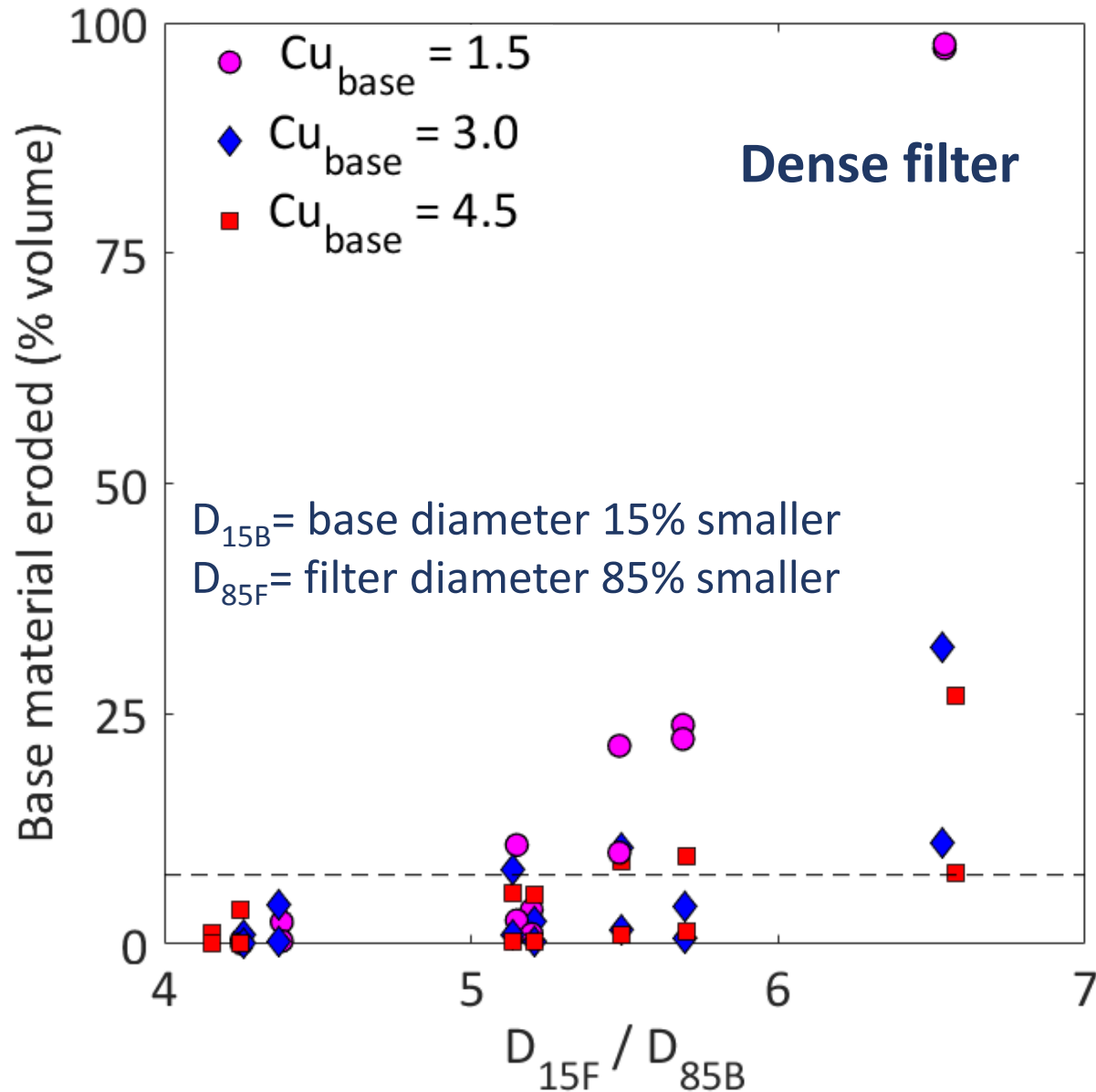
Filtration – Network model



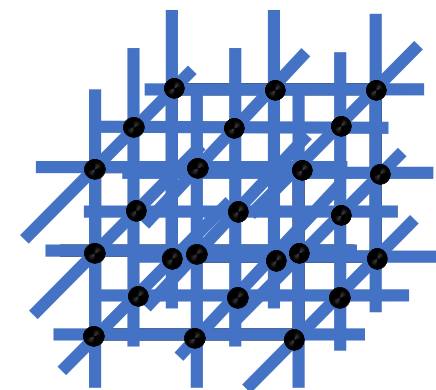
- Cu Filter = 1.5 and 3.0
- Network model that considers only constriction sizes and not full void space topology confirms experimental observation that filter characteristic diameter (D_{15F}) controls filtration



Filtration – Network model



- Cu Filter = 1.5 and 3.0
- Network model that considers only constriction sizes and not full void space topology confirms experimental observation that filter characteristic diameter (D_{15F}) controls filtration



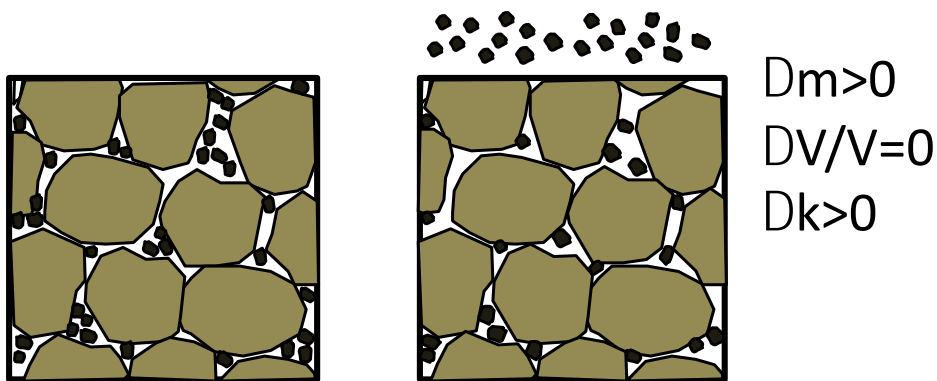
Research questions: Filter retention

- Normalization of constriction size distributions by D_{15F} gives a narrow set of curves, supporting idea that D_{15F} is indicative or representative of the constrictions sizes in a filter.
- Network analyses support use of D_{15F} / D_{85B} to judge retention capacity. Analyses also support idea that effective retention requires $D_{15F} < 4 D_{85B}$ in line with recent ICOLD documents.

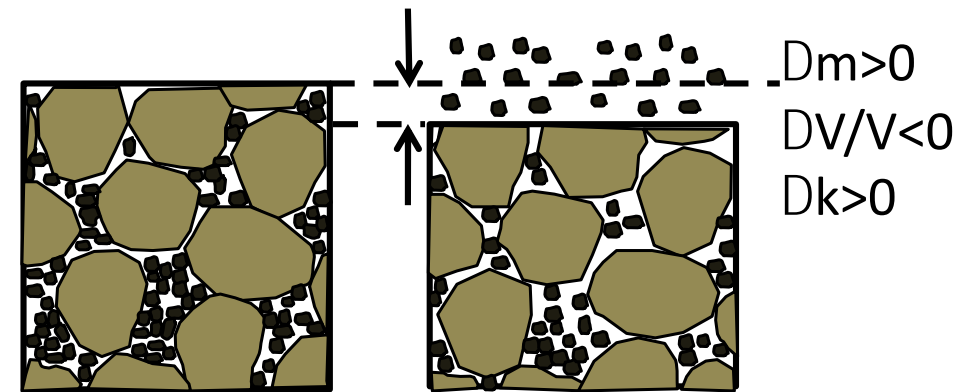
Suffusion

- **suffusion** is the selective erosion of the fine particles from the matrix of coarse particles under the action of a hydraulic gradient
- **suffusion** is sometimes associated with lack of volume change, **suffosion** associated with volume change
- **internally unstable soils** are susceptible to suffusion / suffusion – **internal instability** general term

*m - mass
V - volume
k - hydraulic
conductivity*



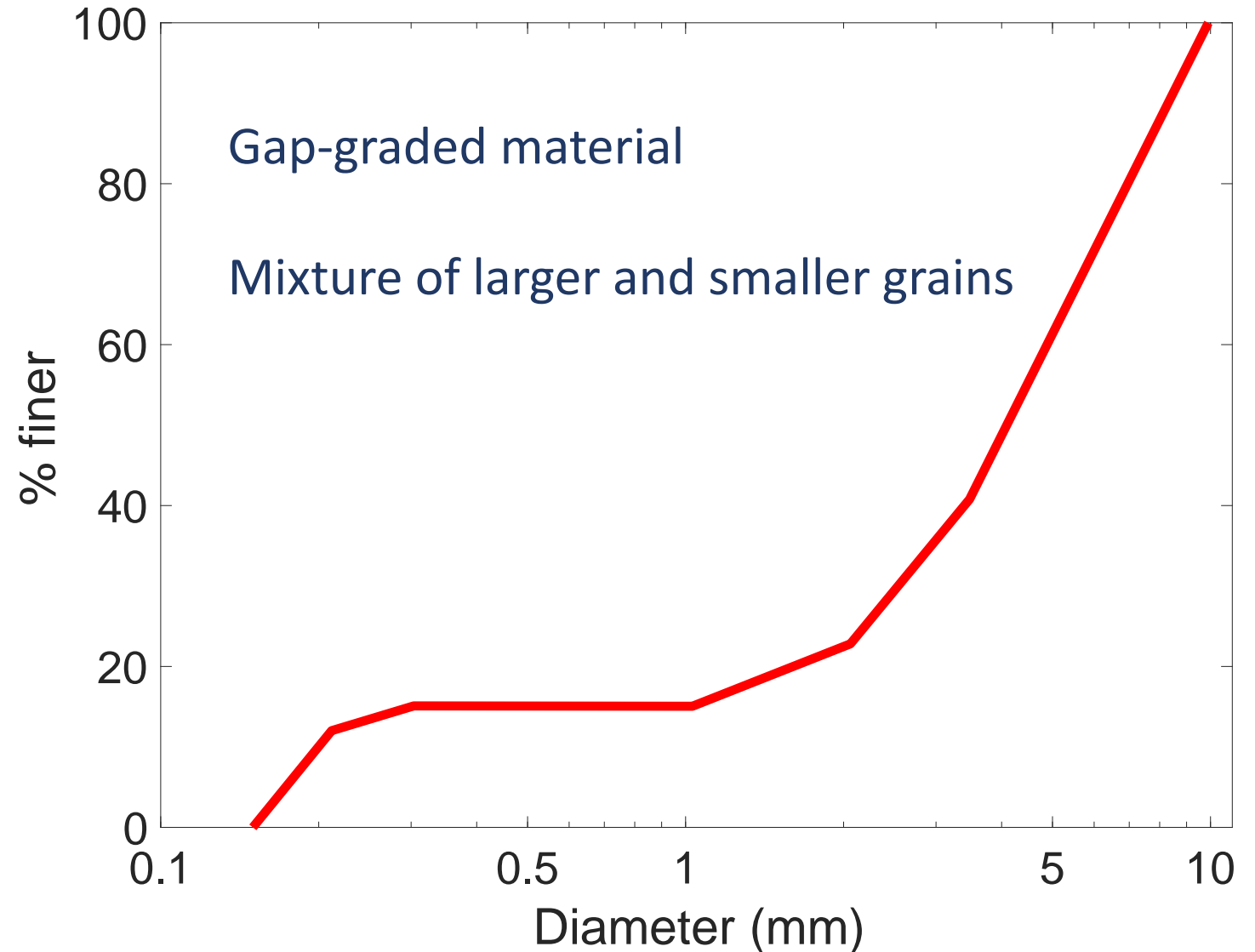
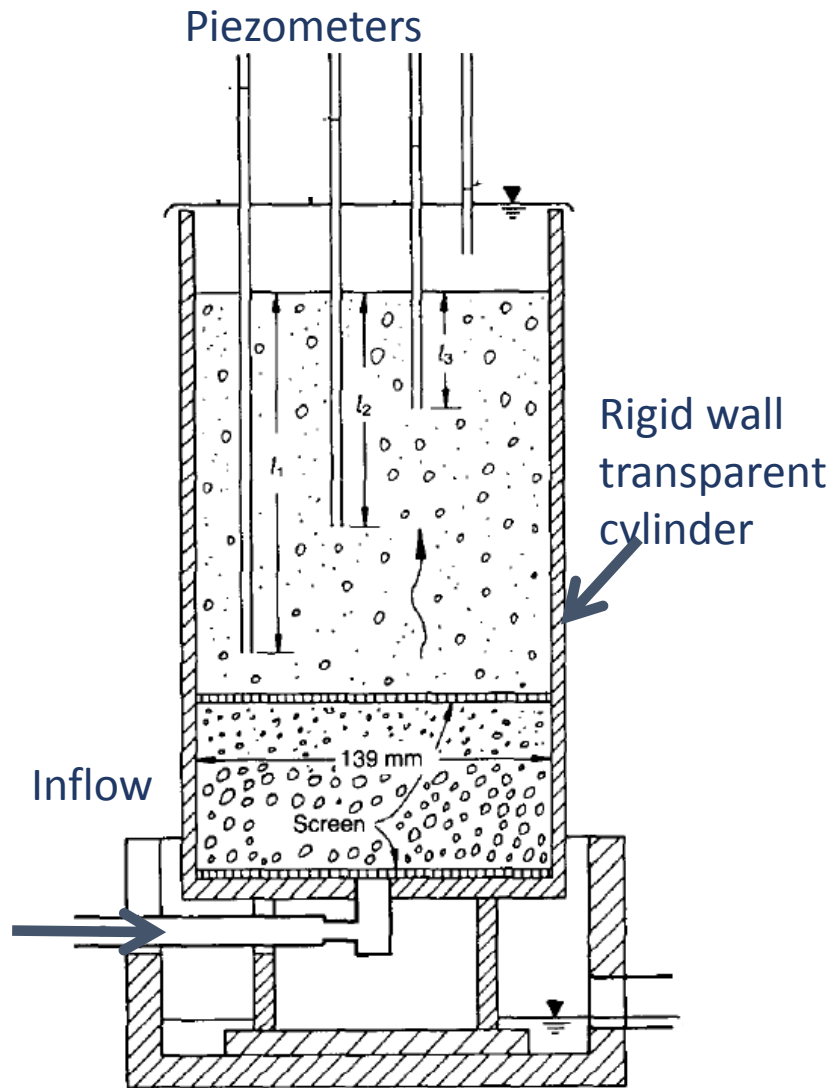
Instability without volume change



Instability with volume change

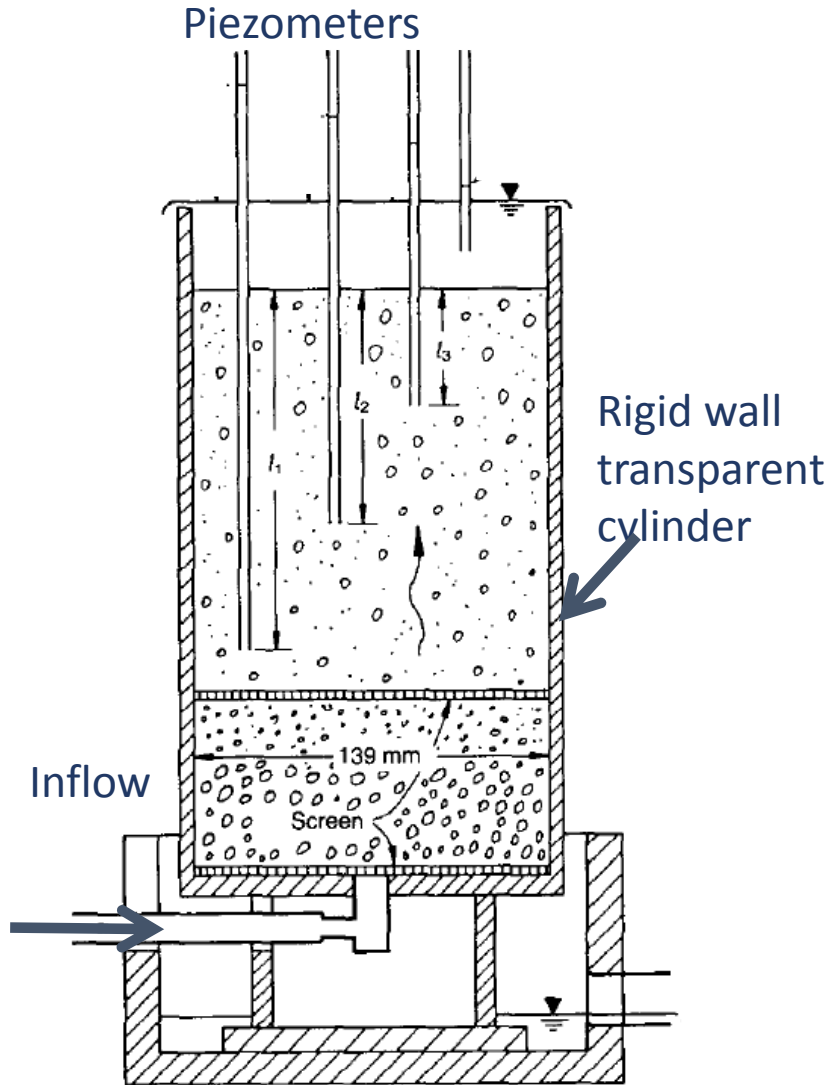
After Slangen and Fannin

Skempton and Brogan Permeameter Experiments

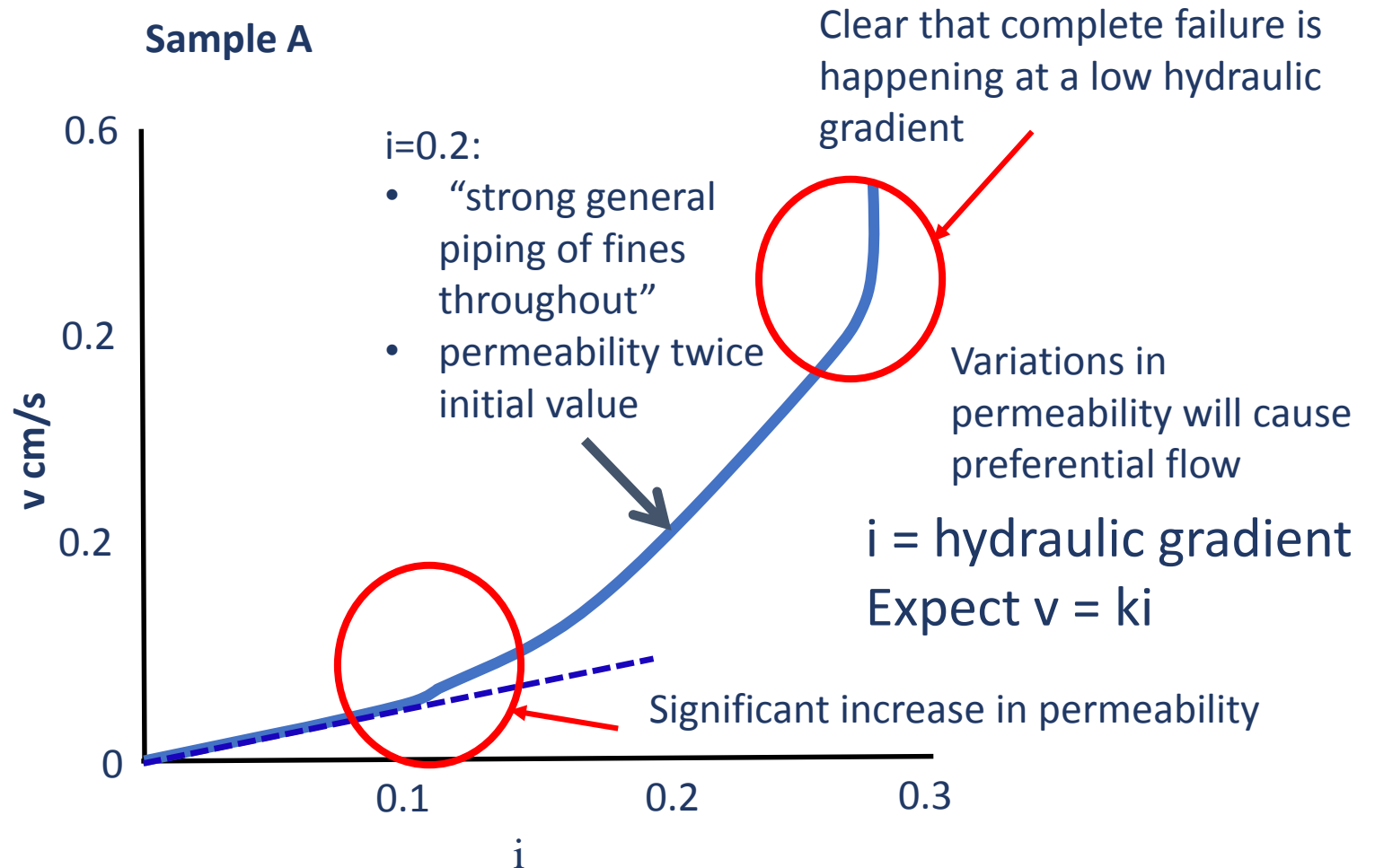


Skempton and Brogan Permeameter Experiments

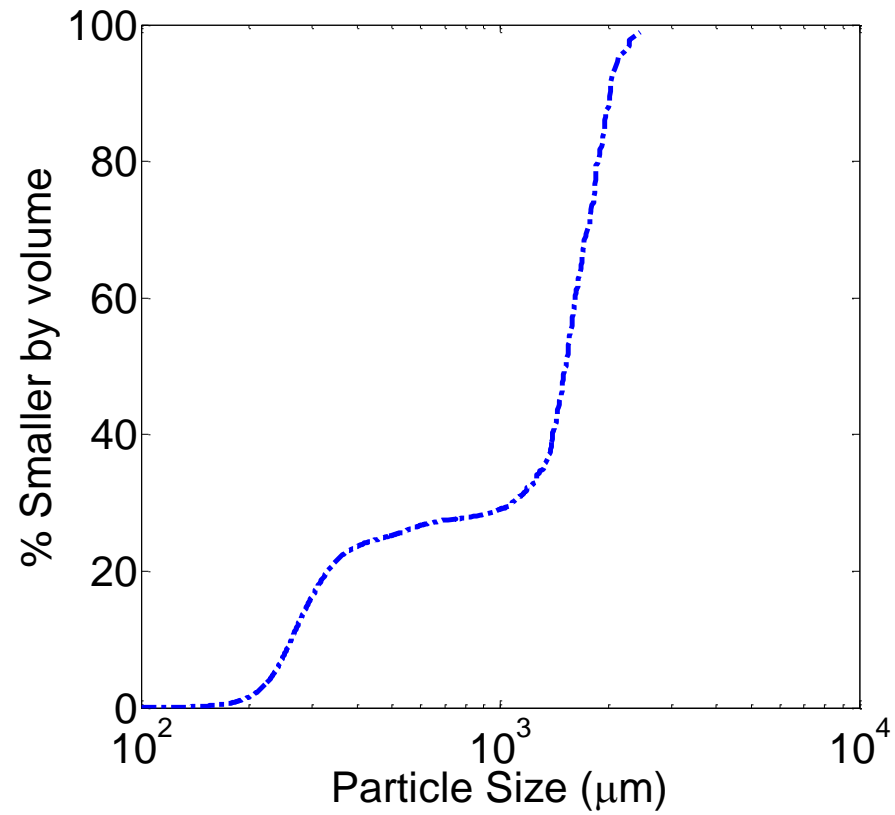
Skempton and Brogan (1994)
Géotechnique



Sample A



Internal Instability



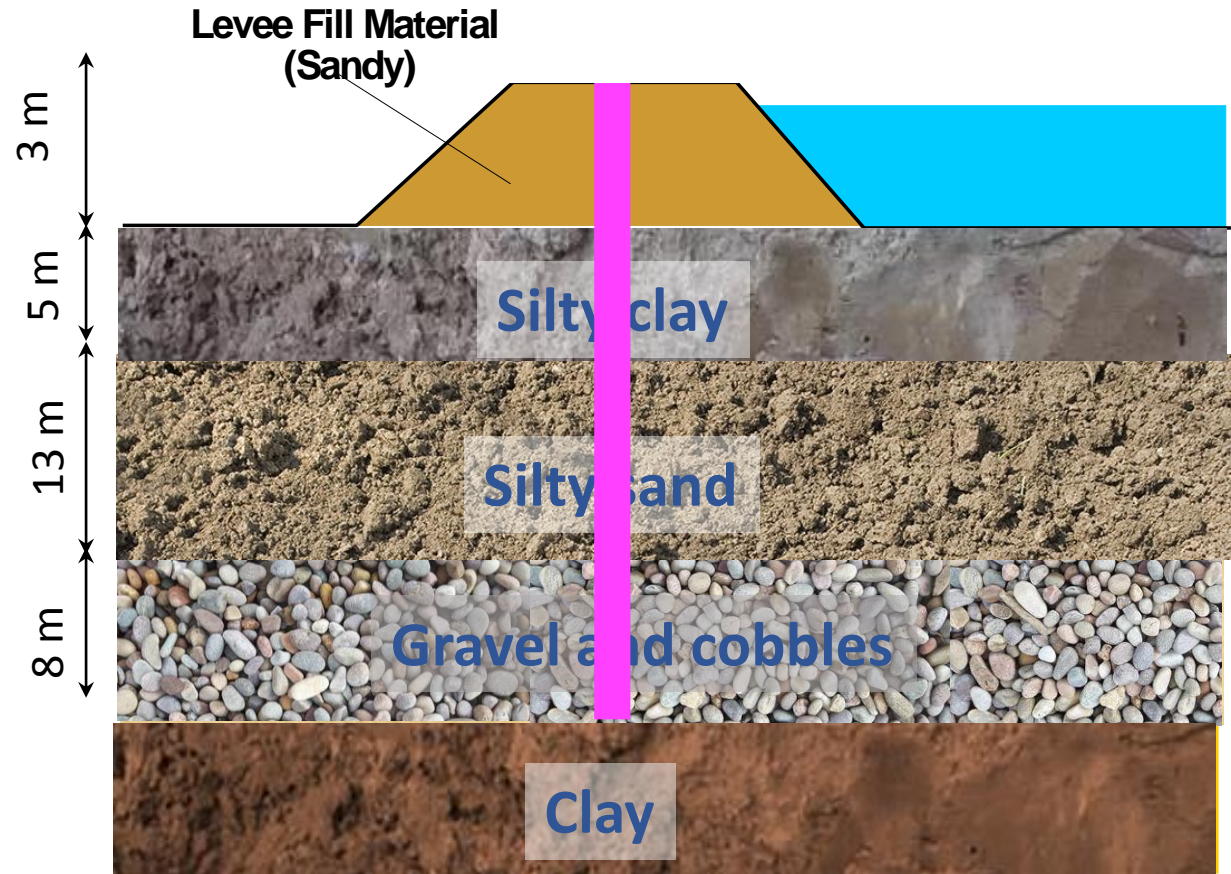
Robert Negri MSc

MSc student photo of internal instability

Flood embankments

Design aim :

- Reduce downstream hydraulic gradient, i
- Critical case $i=1$

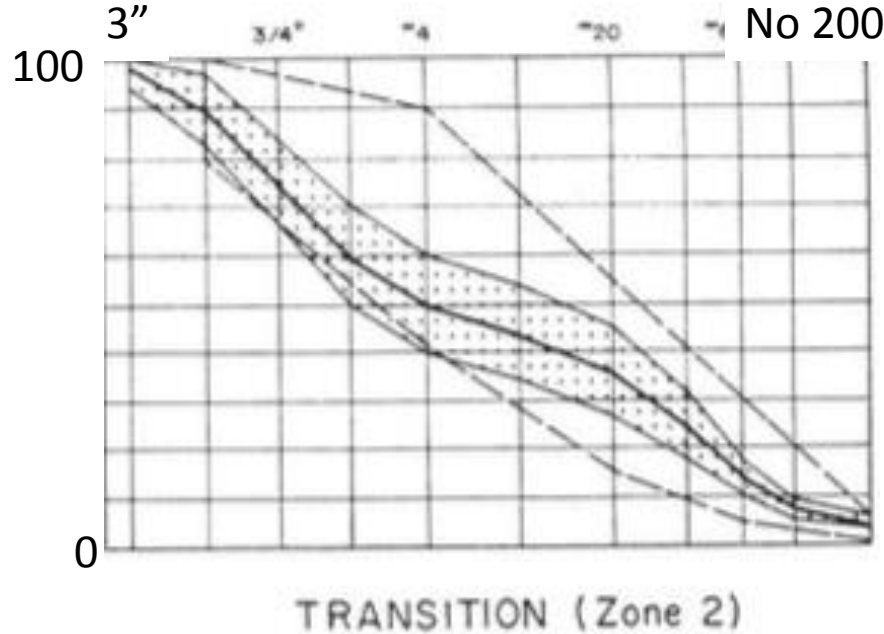
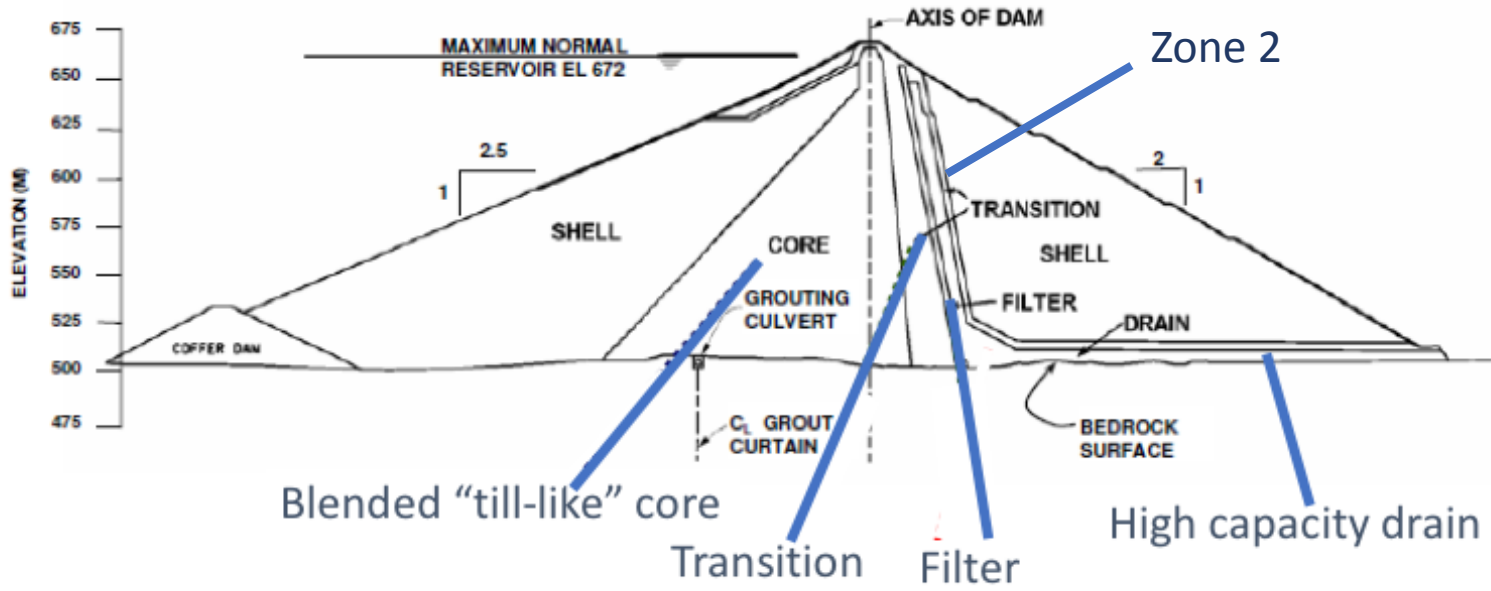


WAC Bennett Dam



- Located in British Columbia, Canada
- Owned by BC Hydro
- High as a 60-storey building and two kilometres wide
- Holds back 360 kilometres of Williston Lake, the largest reservoir in North America

Bennett dam transition



BC Hydro as cited by
Muir Wood (2007)

https://www.imperial.ac.uk/media/imperial-college/faculty-of-engineering/civil/public/geotechnics/Fannin_1Sept17_London.pdf



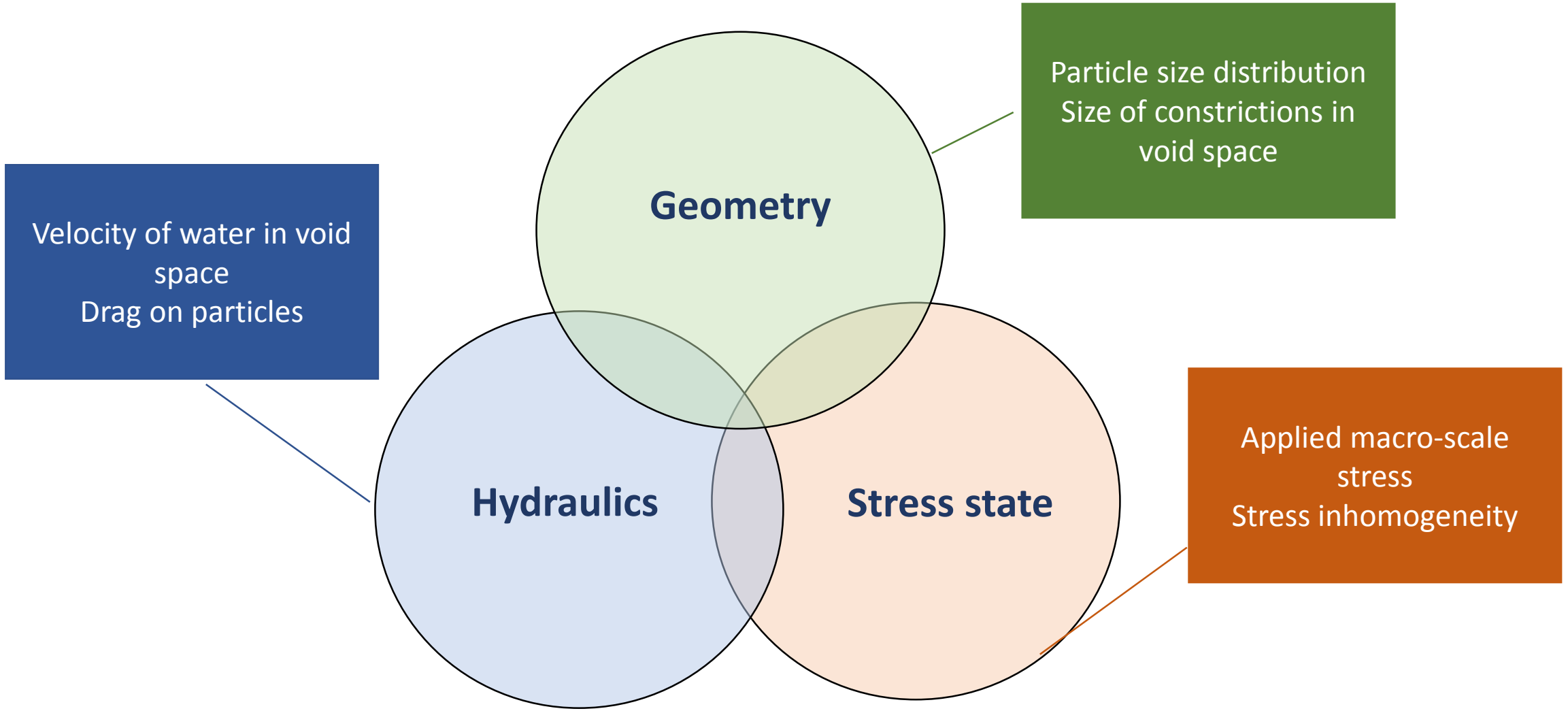
UBC Permeameter

WAC Bennett Dam

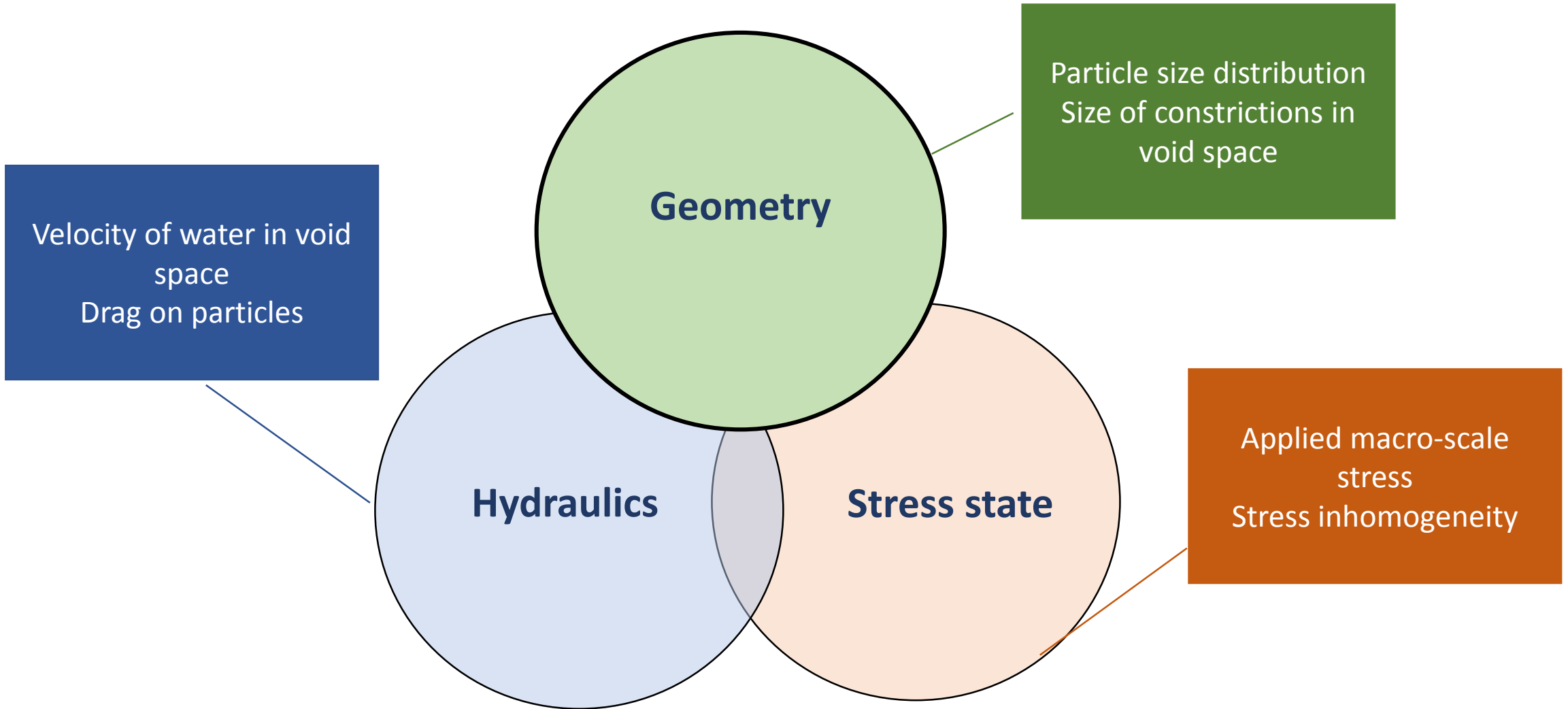


1996 Sinkhole at WAC Bennett Dam
(BC Hydro as cited by Muir Wood, 2007)

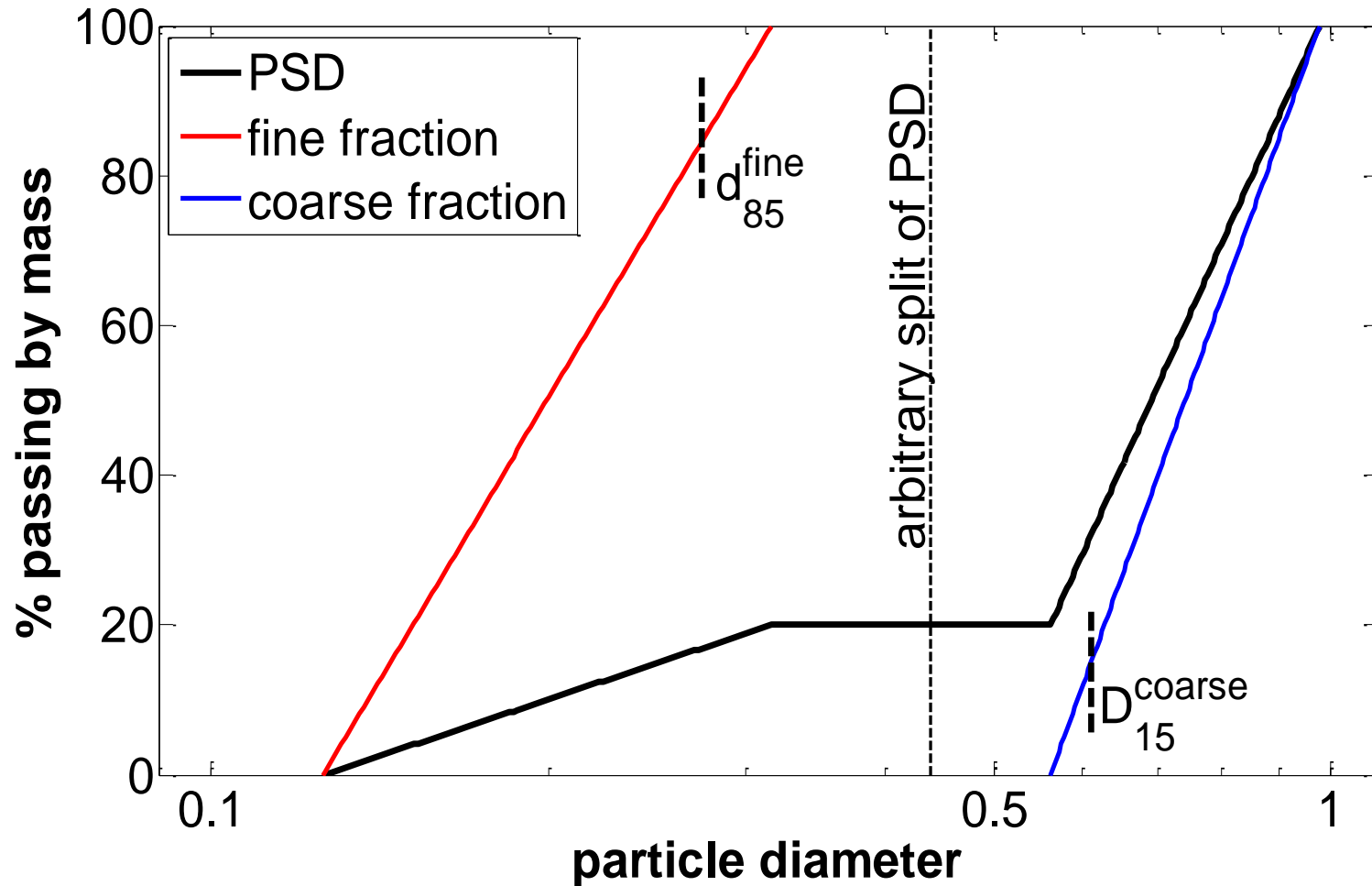
Factors influencing internal instability risk



Factors influencing internal instability risk



Empirical Filter Criteria: Kézdi (1979)



Split PSD into coarse and fine “PSDs”

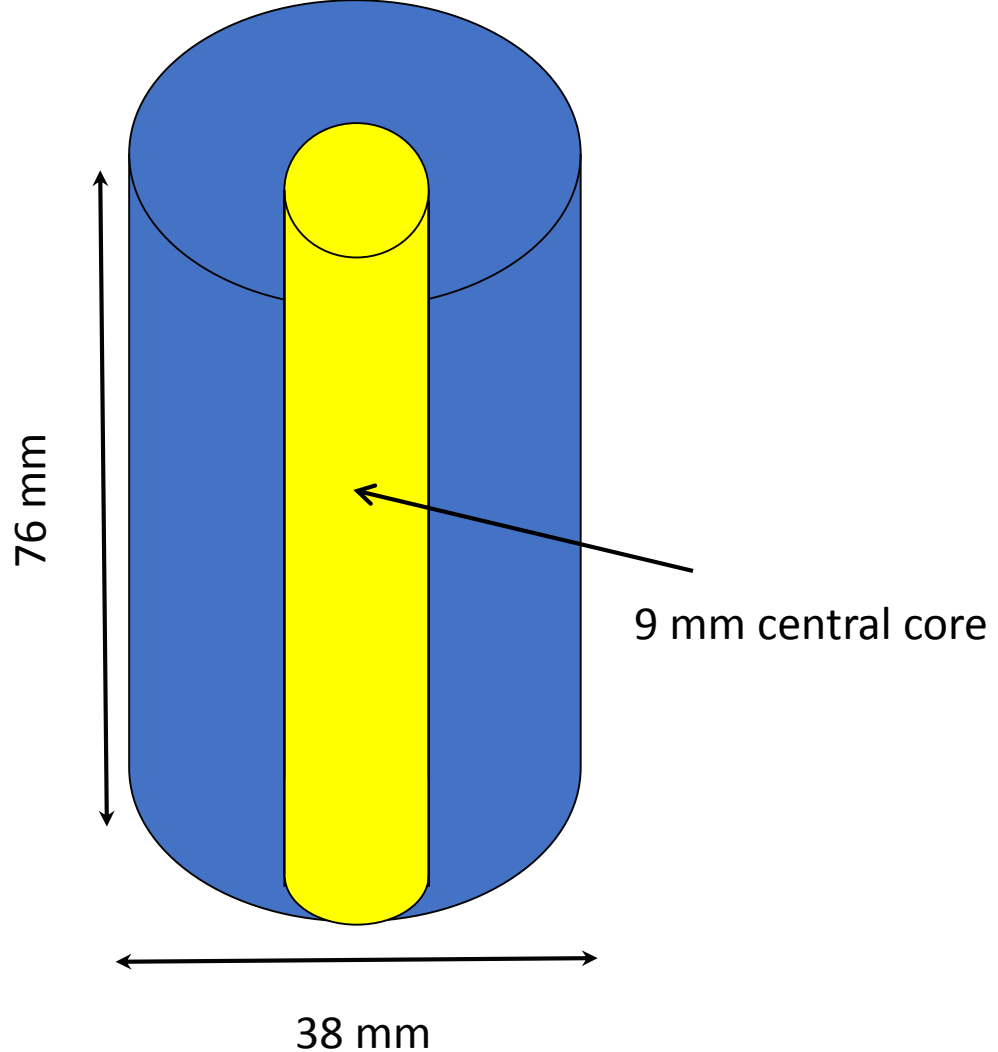
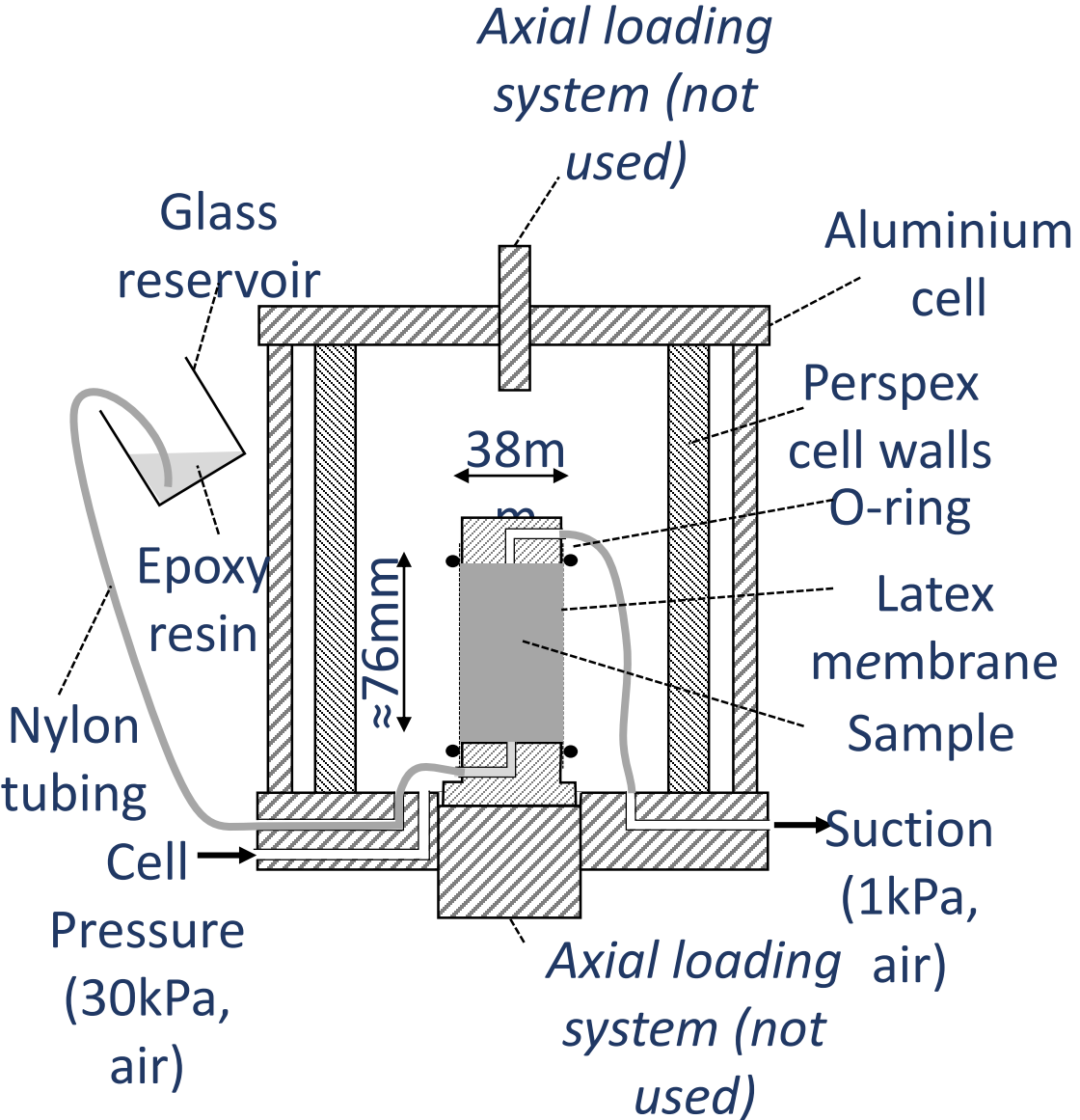
Stable if: $d_{85}^{fine} > (D_{15}^{coarse} / 4)$

i.e. if

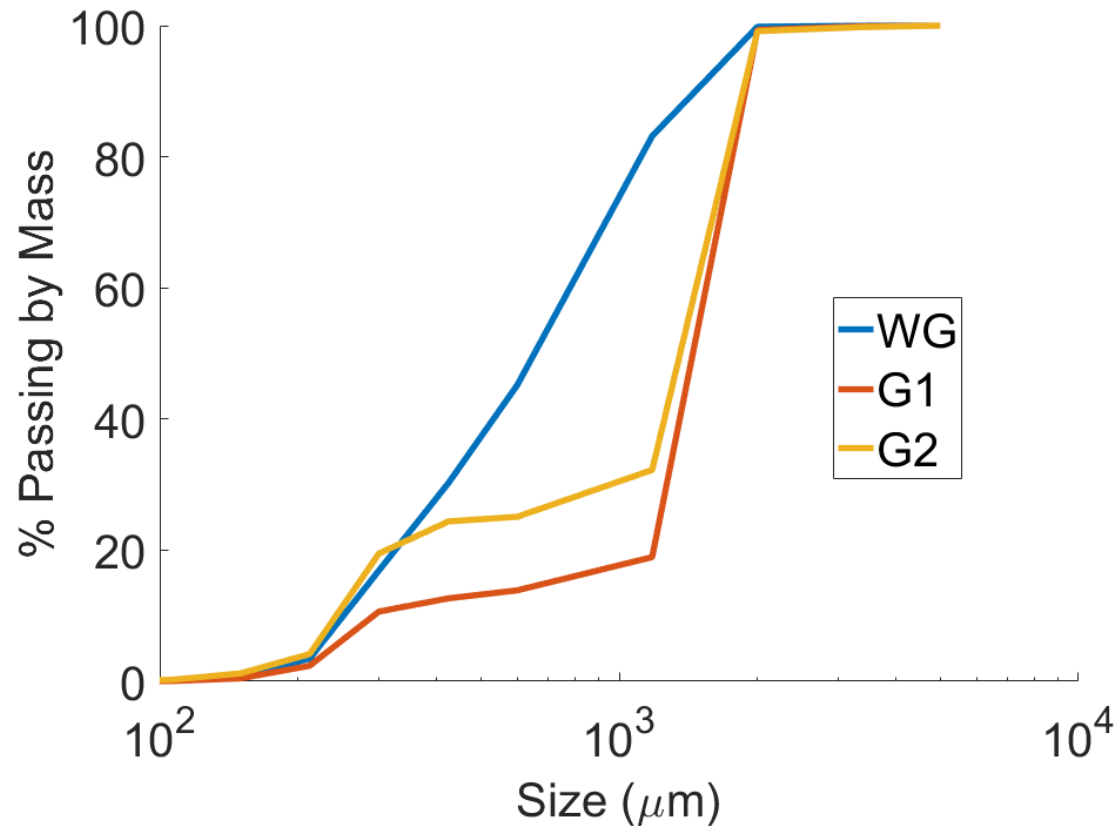
$D_{15}^{coarse} / d_{85}^{fine} < 4$

Relates to Terzaghi filter rule

Microcomputed Tomography (μ CT)



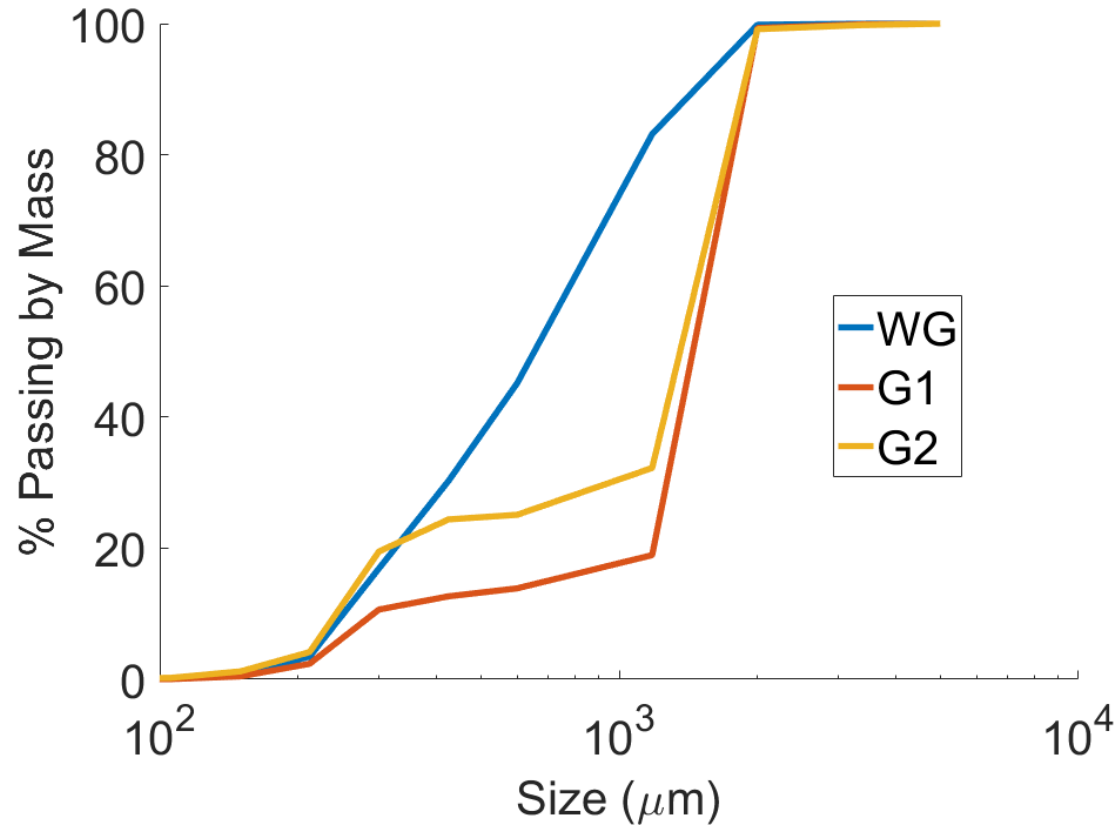
Internal Instability: μ CT study materials



3 scan samples for each grading

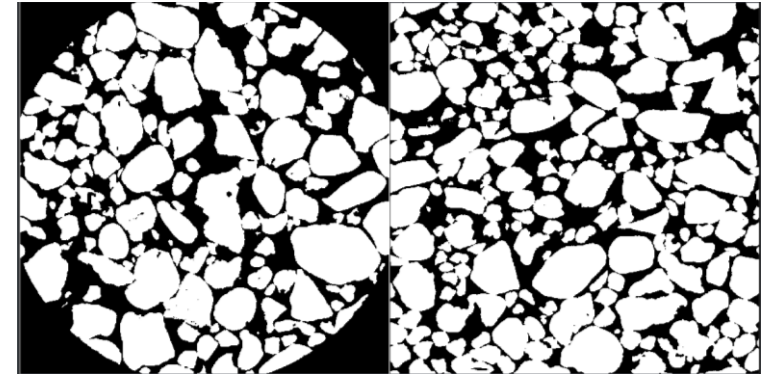
- WG – Kézdi ratios 1.54-1.62
- G1 - Kézdi ratios 3.3 – 4.66
- G2 - Kézdi ratios 4.01 – 4.29

Internal Instability: μ CT study materials



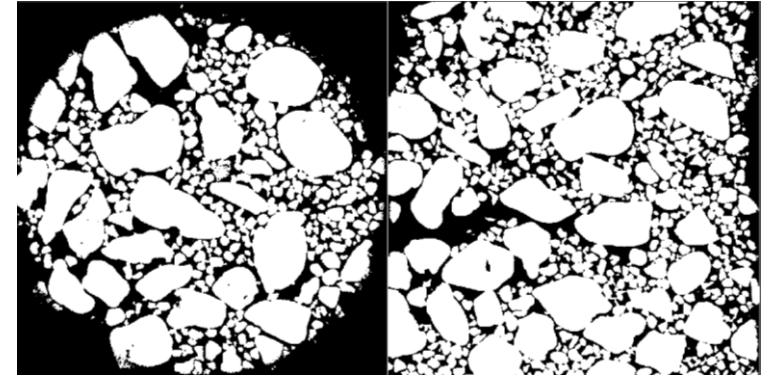
WG Middle

$$D_{15}^{\text{coarse}}/d_{85}^{\text{fine}}=1.62$$



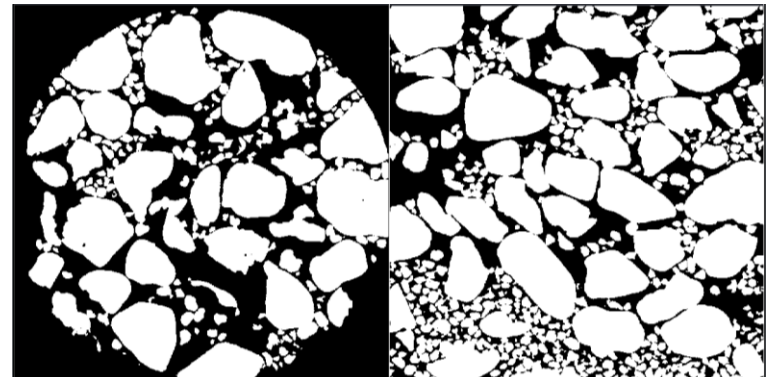
G1 Bottom

$$D_{15}^{\text{coarse}}/d_{85}^{\text{fine}}=3.30$$



G2 Middle

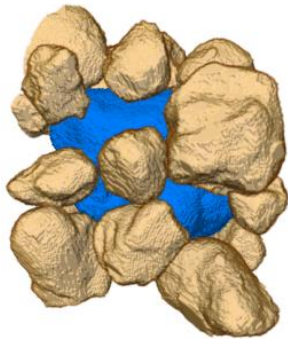
$$D_{15}^{\text{coarse}}/d_{85}^{\text{fine}}=4.29$$



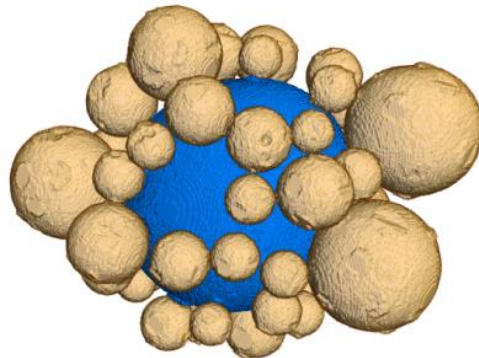
Coordination number

N_c = Coordination number

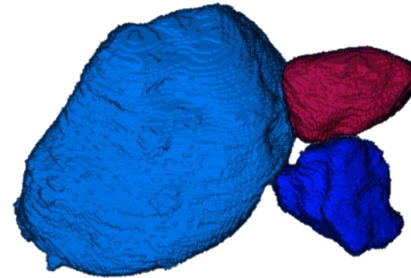
No of contacts per particle



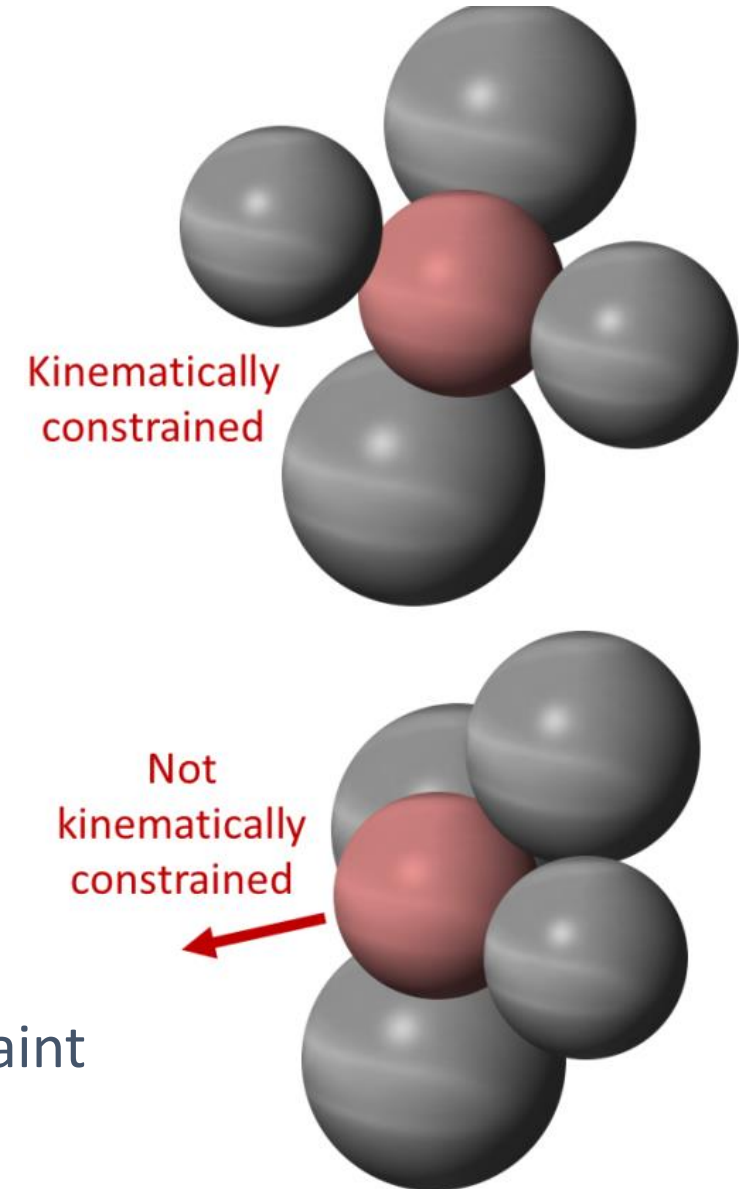
Leighton Buzzard
Sand
Blue particle
20 contacts



Glass beads
Blue particle
50 contacts

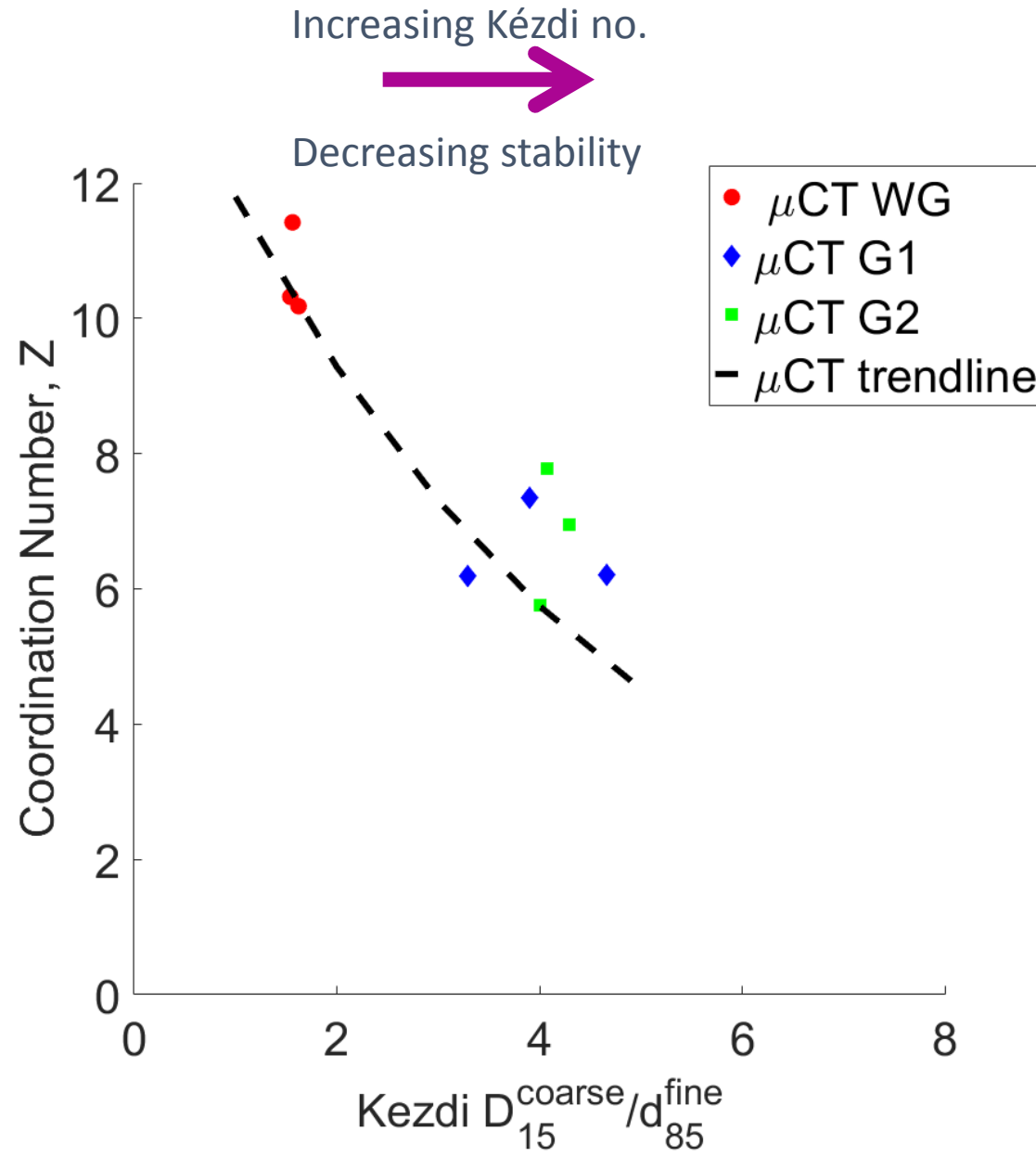


Leighton Buzzard
Sand
Blue particle
2 contacts



No of contacts gives indication of kinematic constraint

Variation in Coordination No. with Kézdi Ratio



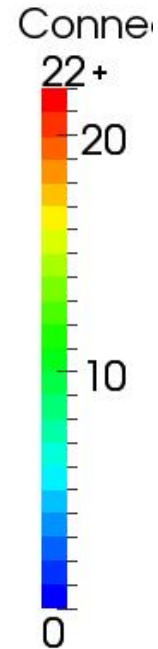
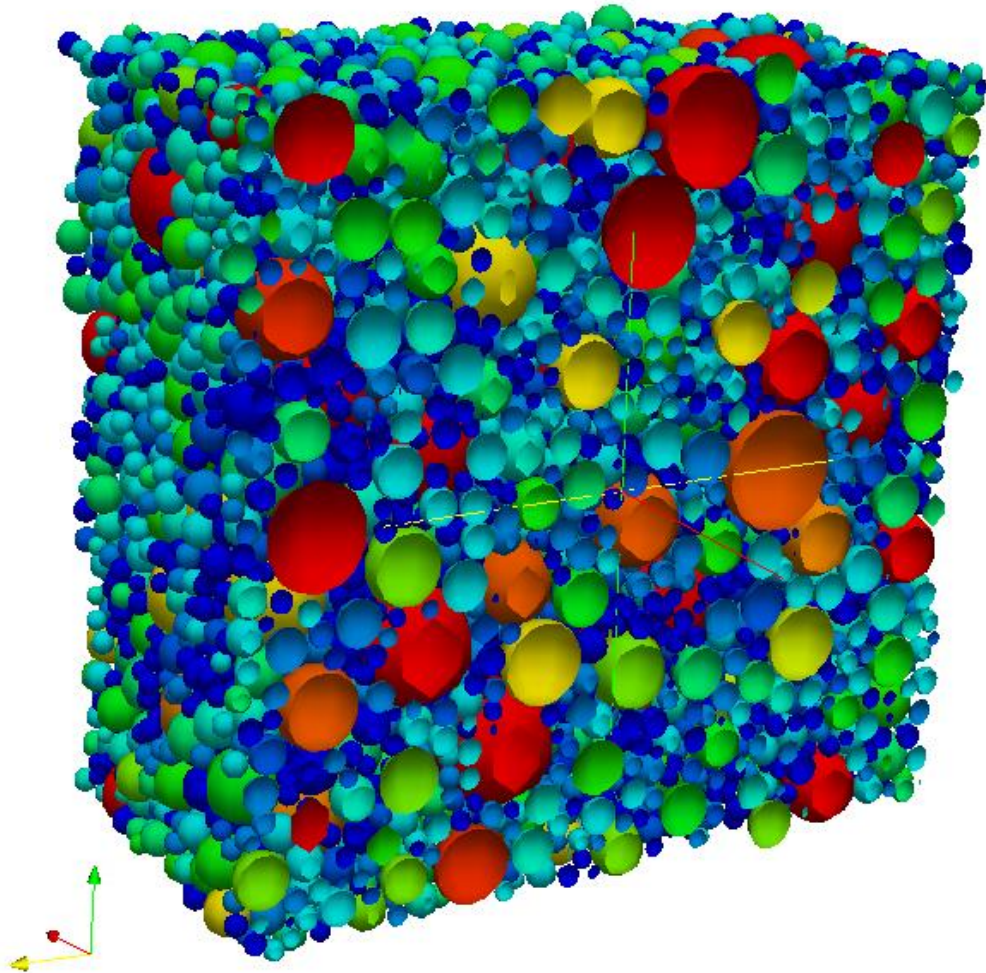
Fonseca et al. (2014)

Géotechnique

Shire and O'Sullivan (2013)

Acta Geotechnica

Discrete element method simulations



Spherical particles

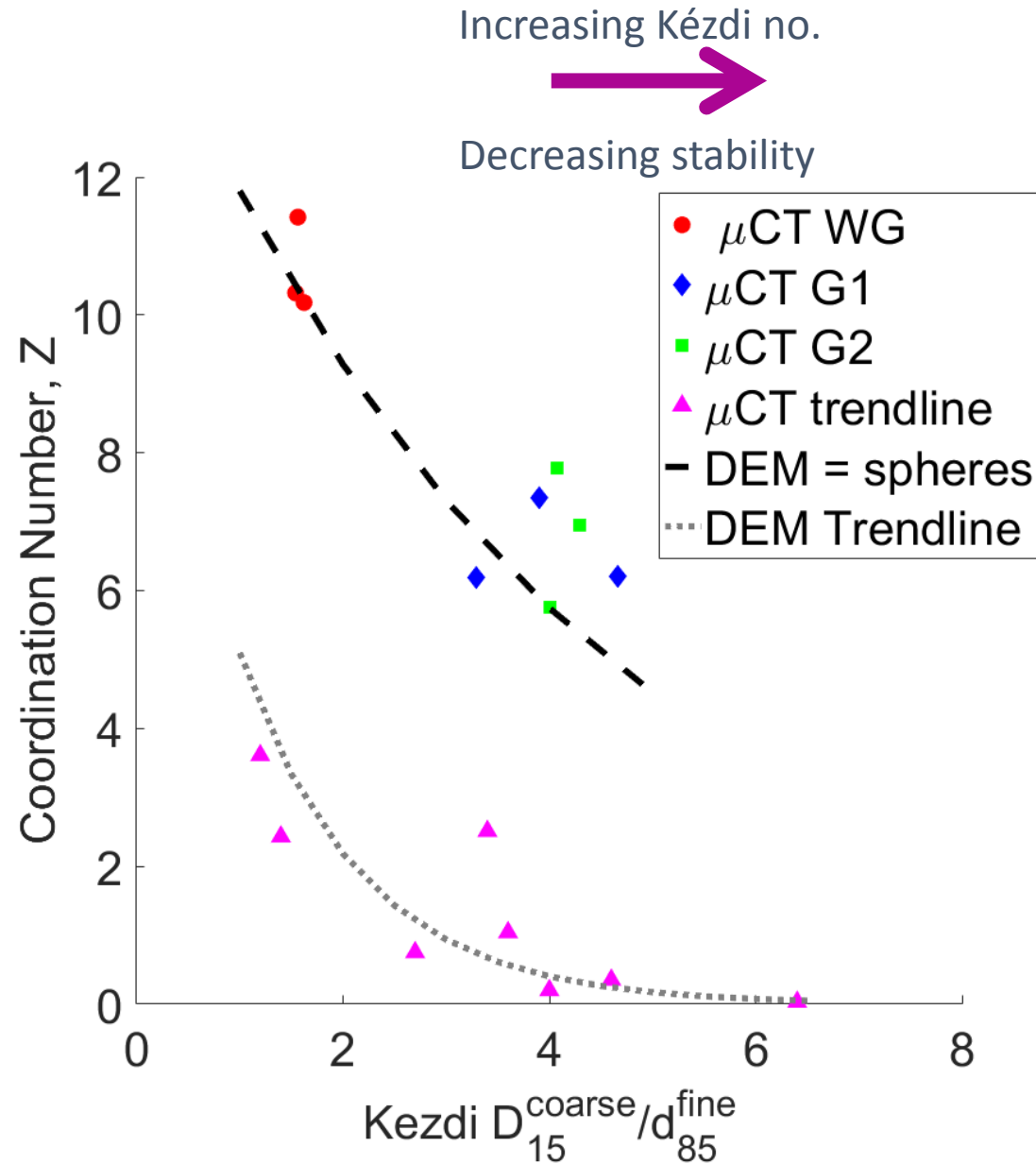
Simple contact models

Isotropic samples

Gravity neglected

Shire and O'Sullivan (2013)
Acta Geotechnica

Variation in Coordination No. with Kézdi Ratio



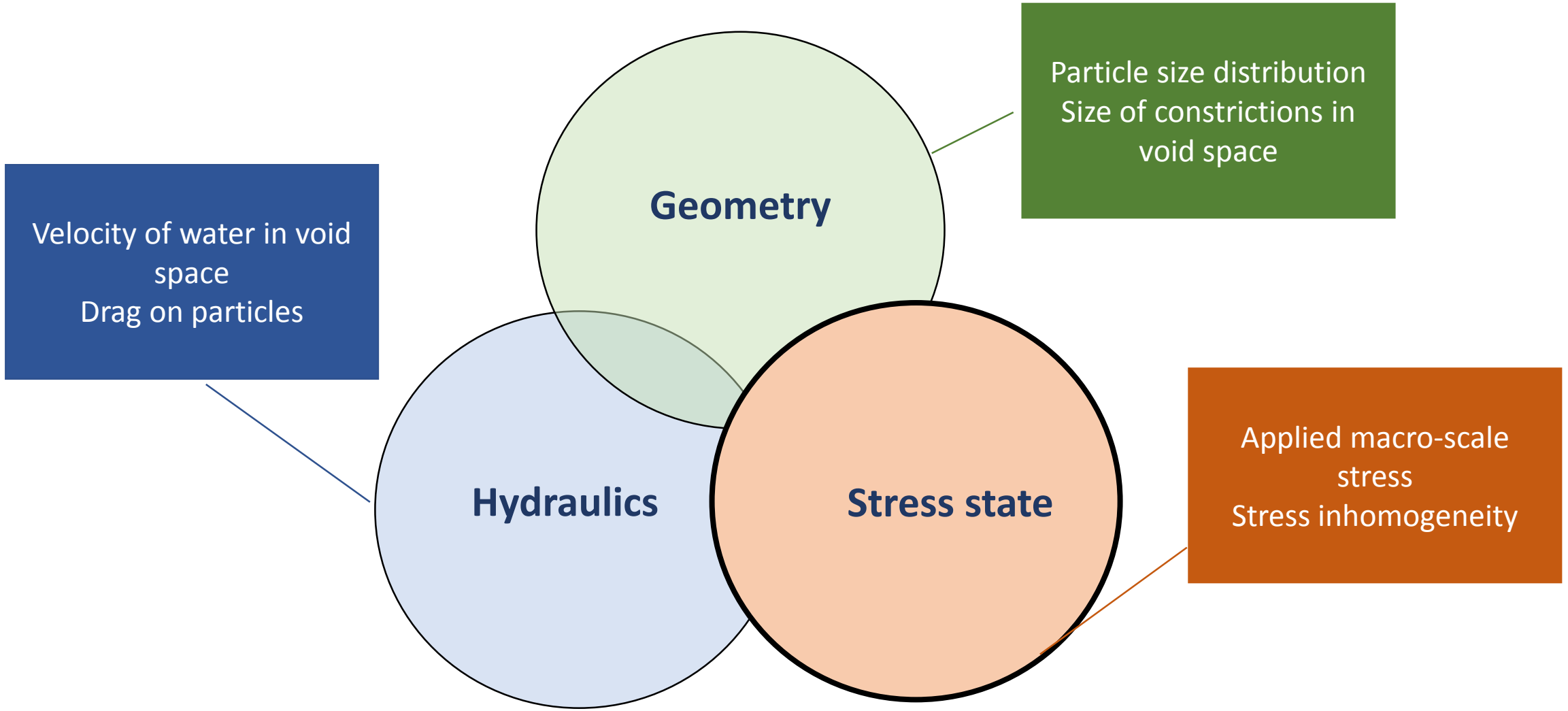
Fonseca et al. (2014)

Géotechnique

Shire and O'Sullivan (2013)

Acta Geotechnica

Factors influencing internal instability risk



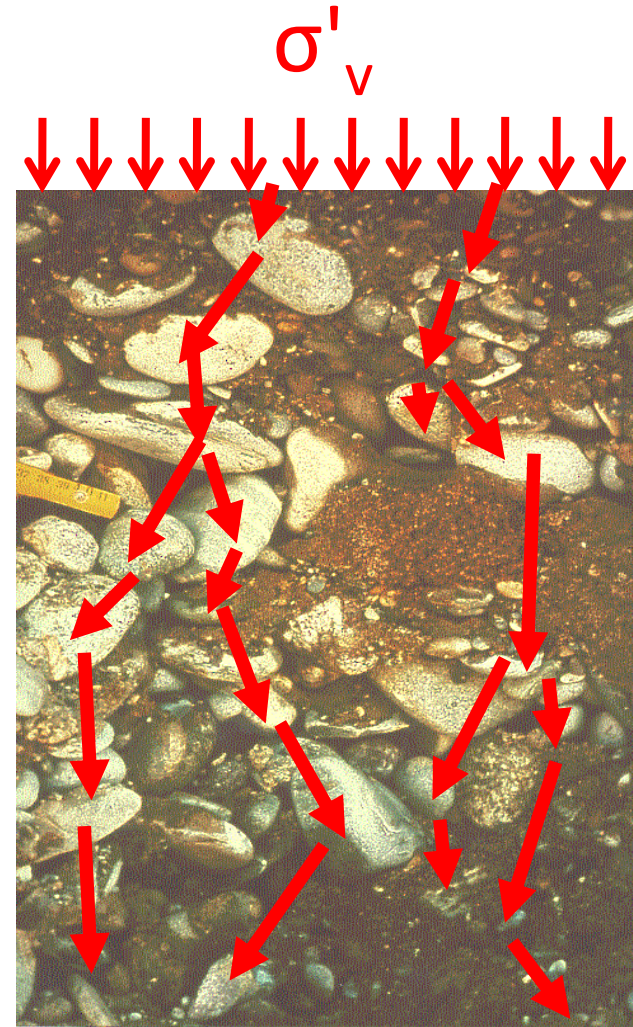
Stress Partition - α

- Hypothesis to explain erosion at low hydraulic gradients
- Based on observations of permeameter tests
- Coarse matrix transfers most of stress
- Finer grains carry reduced effective stress:

$$\sigma'_{\text{fines}} = \alpha \times \sigma'$$

$$\alpha = i_{\text{crit}} / i_{\text{crit(heave)}}$$

Skempton and Brogan (1994)
Géotechnique



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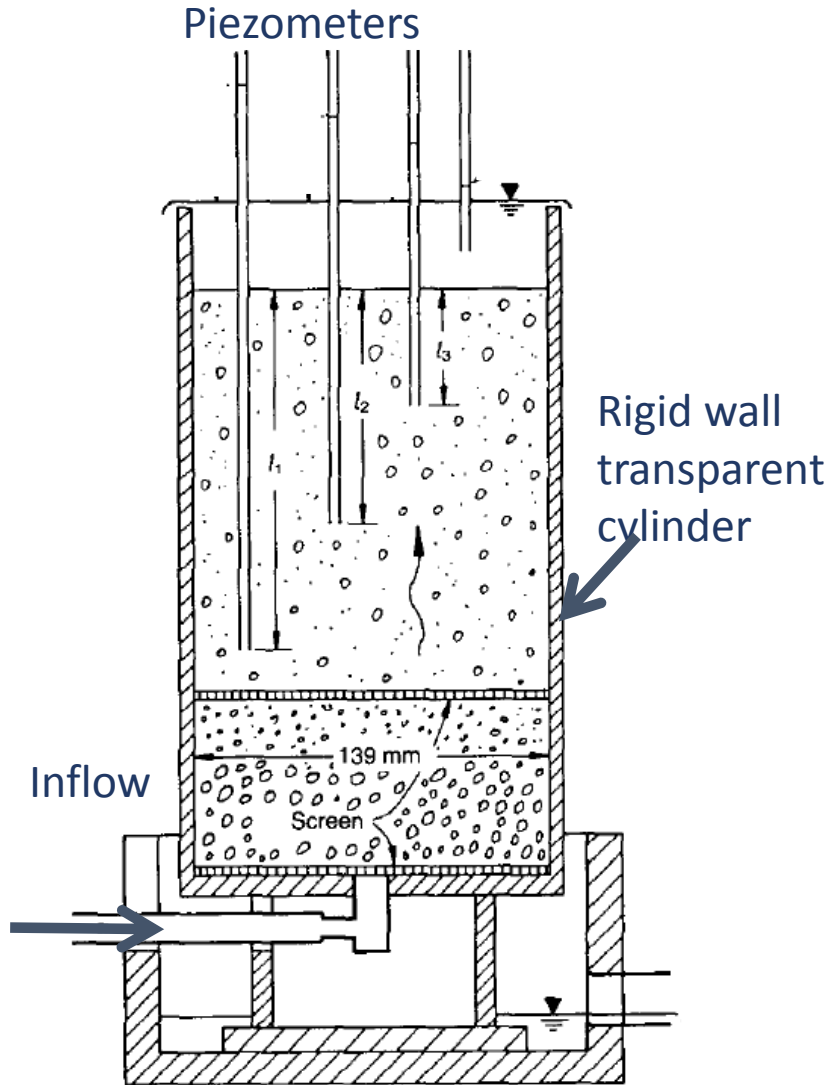
$$\alpha = i_{\text{crit}} / i_{\text{crit(heave)}}$$



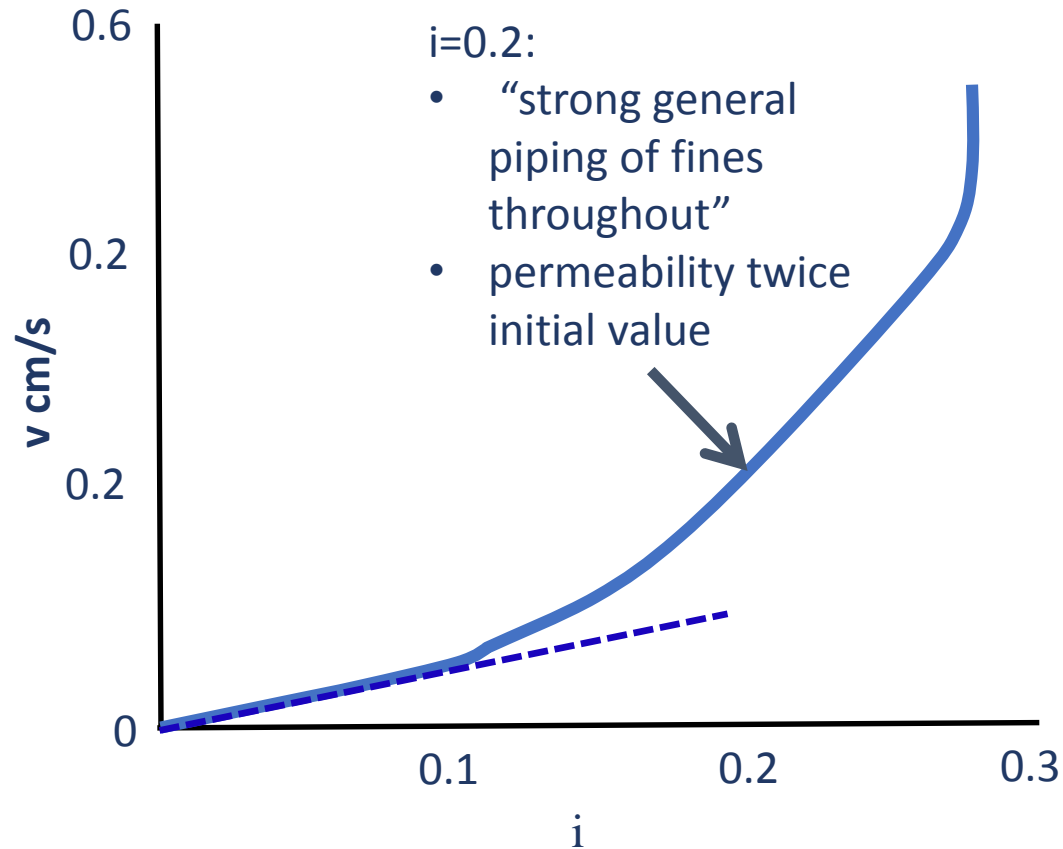
Skempton and Brogan (1994)
Géotechnique

Skempton and Brogan Permeameter Experiments

Skempton and Brogan (1994)
Géotechnique



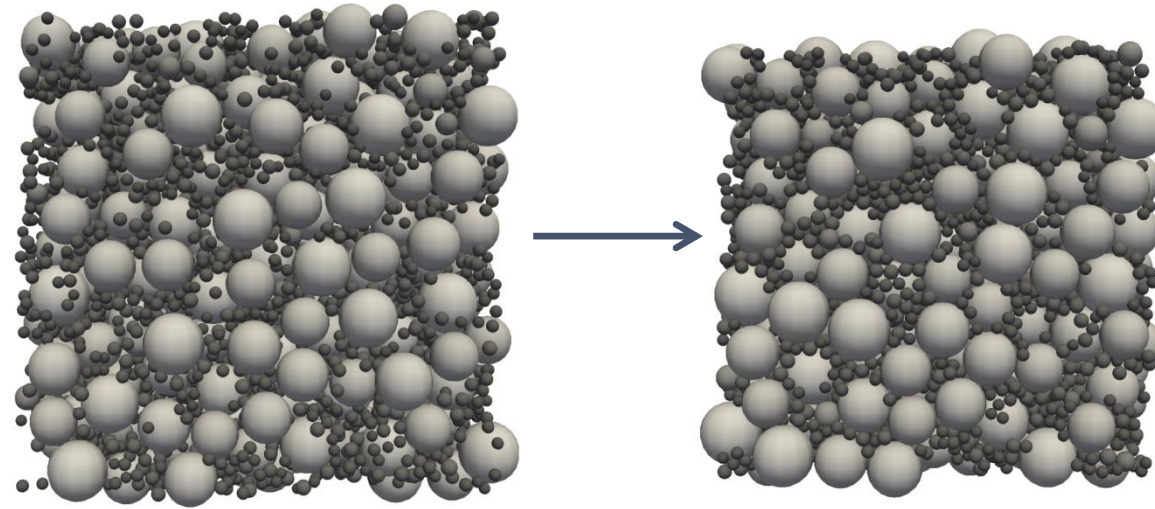
Sample A



$$\alpha = i_{crit} / i_{crit(heave)}$$

$$\sigma'_{fines} = \alpha \times \sigma'$$

DEM Simulations to Investigate Instability



- DEM code granular LAMMPS with periodic boundaries
- Isotropic compression at to $p' = 50\text{kPa}$
- Sample density controlled using inter particle friction

(μ):

$\mu = 0.0$ (Dense)

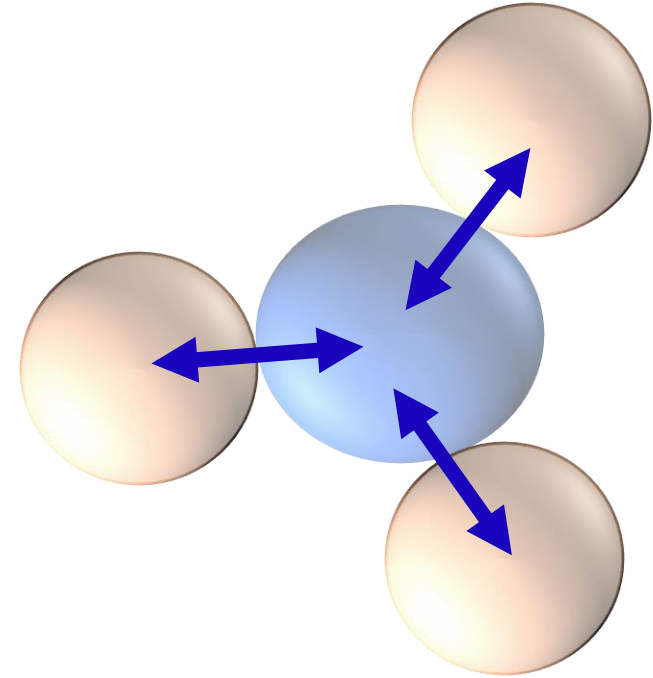
$\mu = 0.1$ (Medium dense)

$\mu = 0.3$ (Loose)

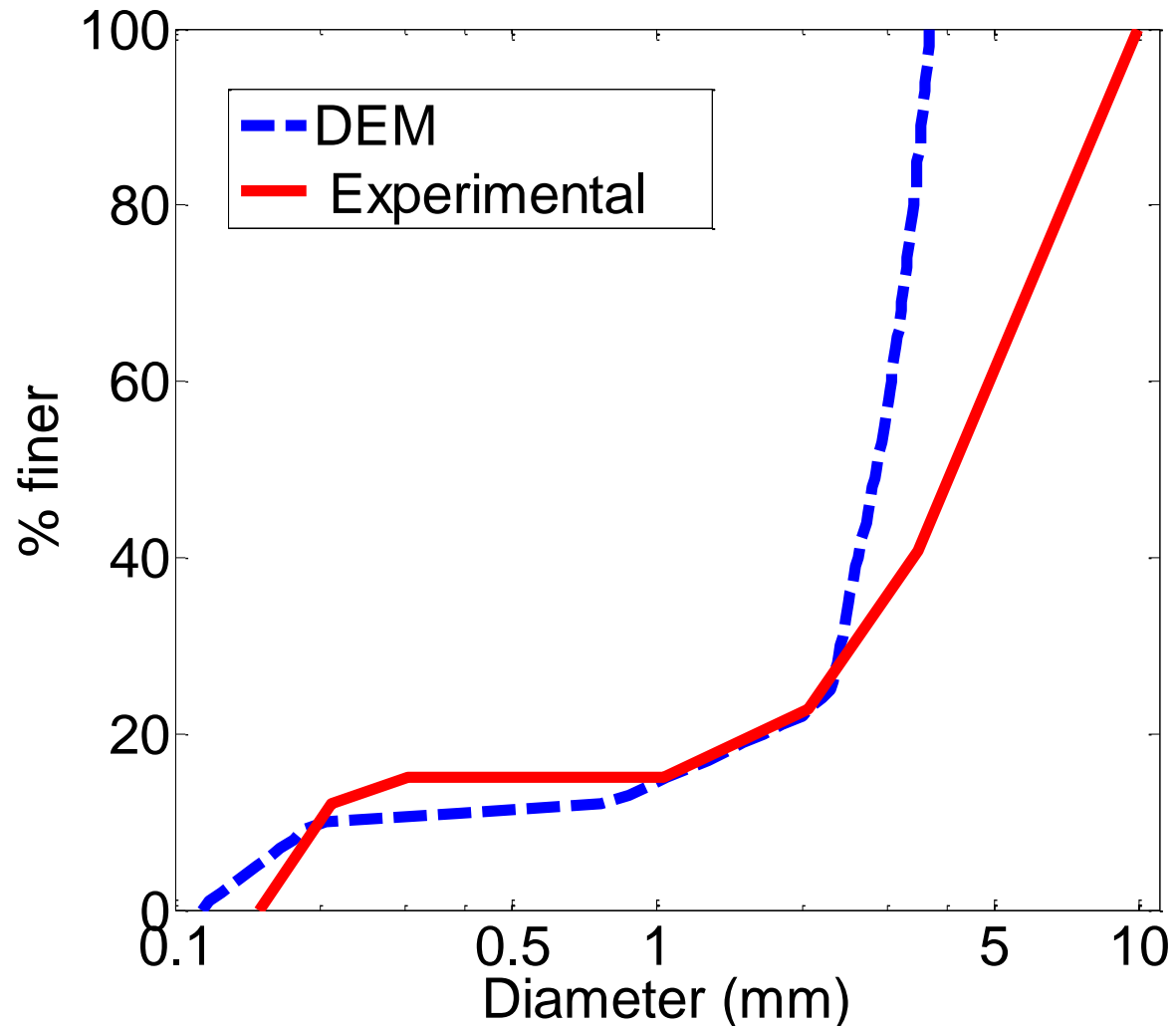
α – DEM Calculations

$$\alpha = \frac{p'_{fine}}{p'}$$

- p' =overall mean effective stress
- p'_{fine} =mean effective stress in finer fraction
- p' and p'_{fine} can be directly obtained from a summation of contact forces in DEM



Skempton and Brogan Sample A: comparison of α values

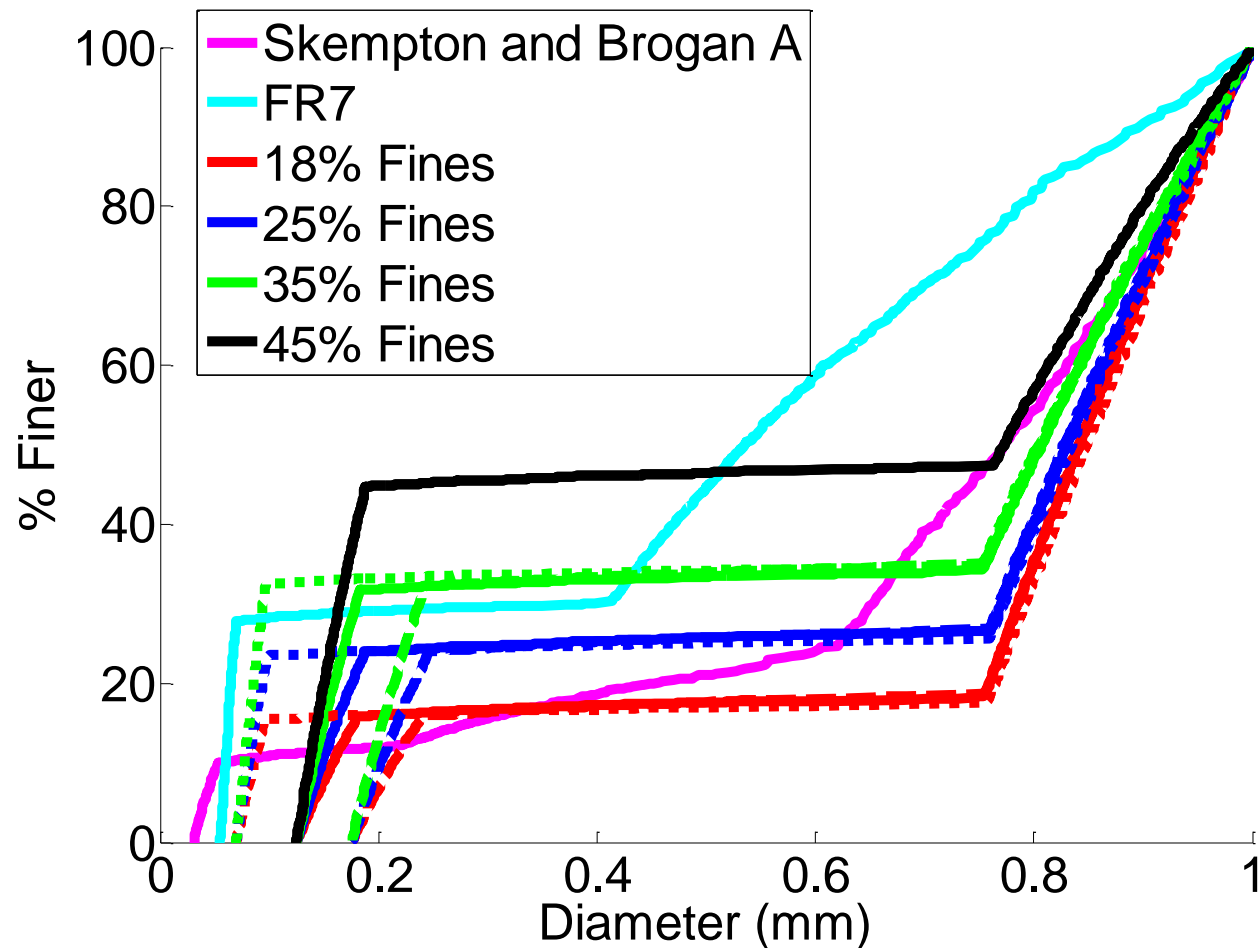


Density	α_{DEM}
Loose	0.15
Medium	0.06
Dense	0.04

$$\alpha_{\text{experiment}} = 0.18$$

Experimental sample placed moist with no densification

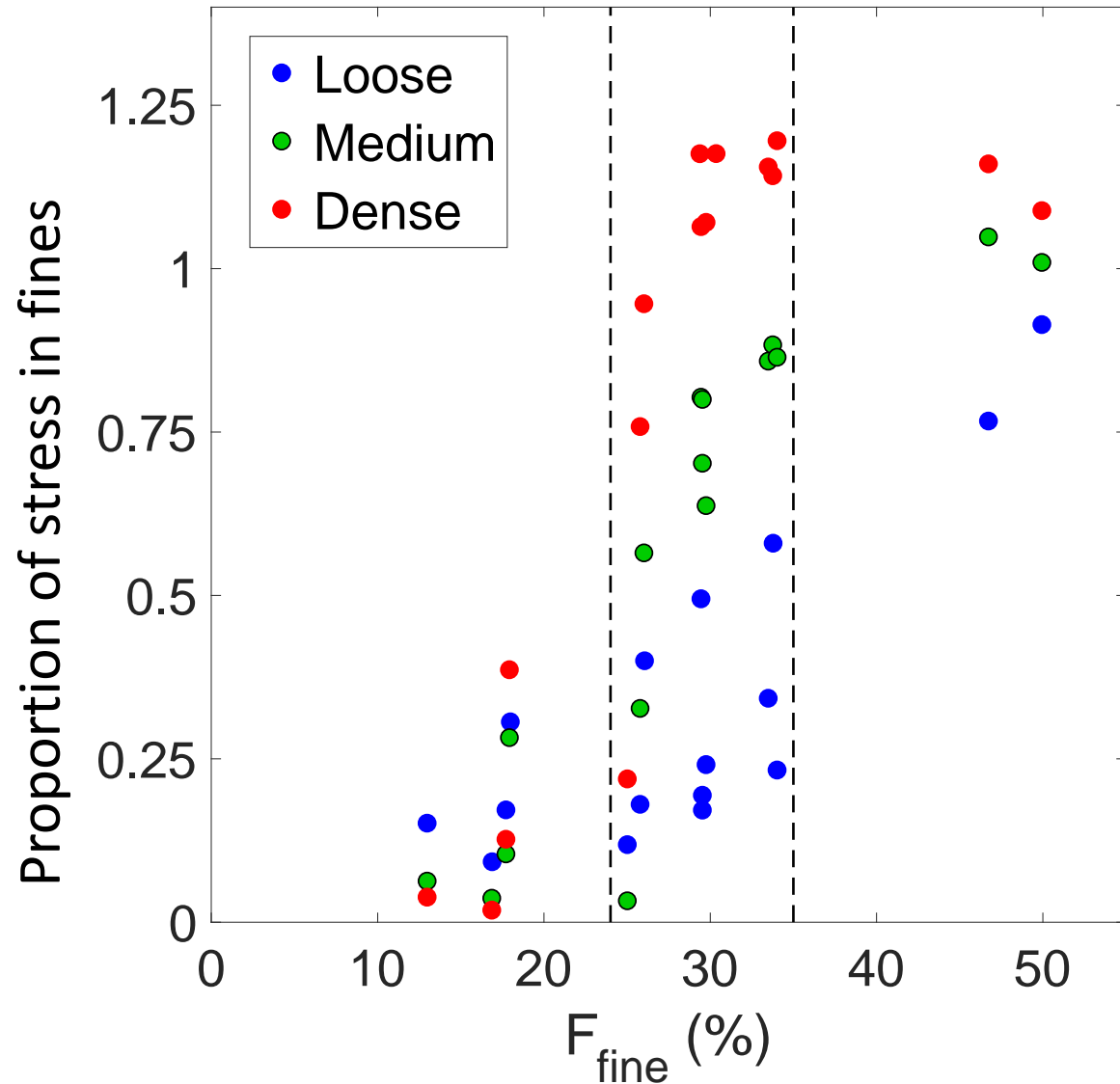
Link between α and particle size distribution



Looked at a range of gap graded materials

Density varied for all samples

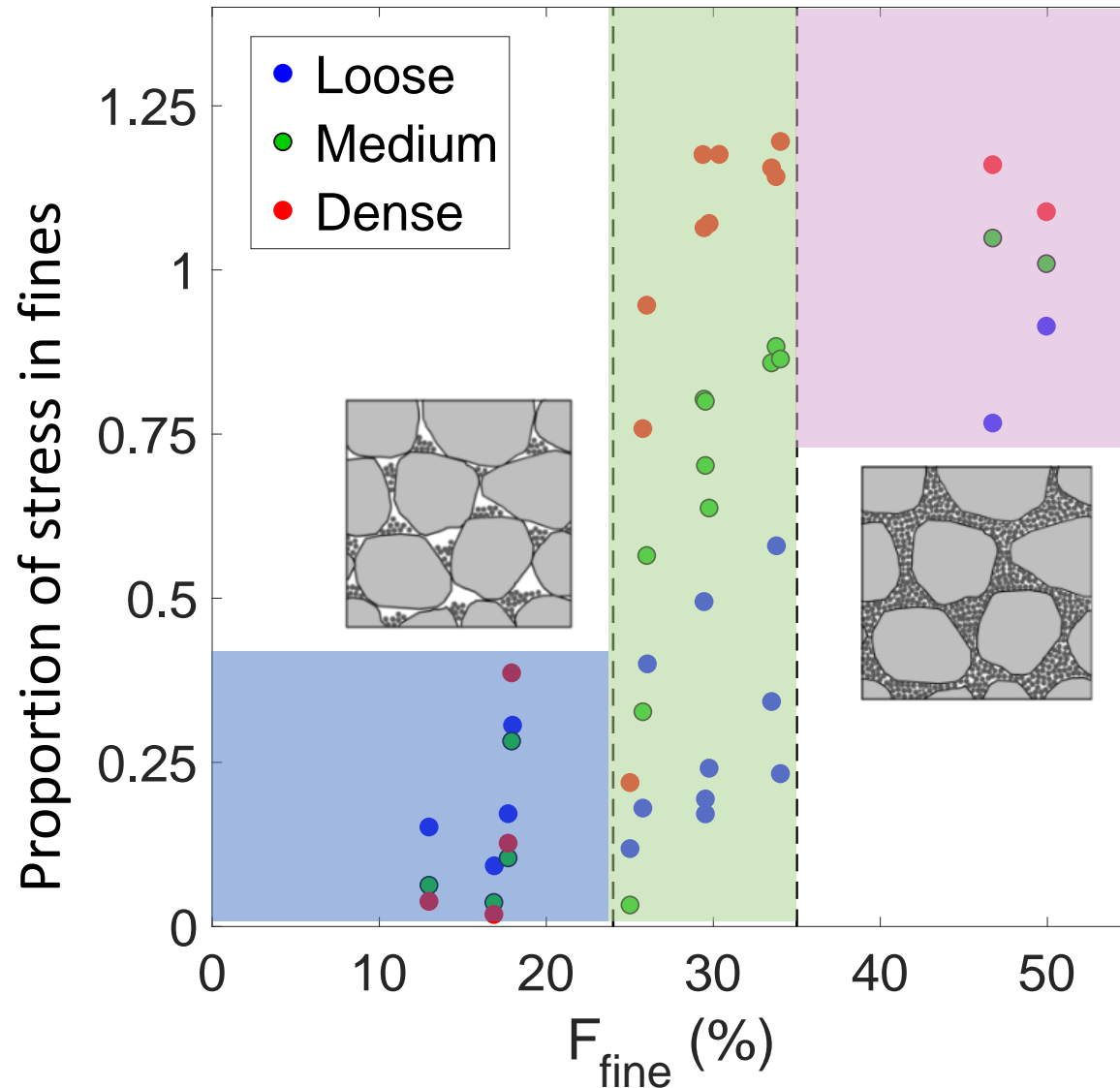
Variation in α with Fines Content (F_{fine})



$$\sigma'_{\text{fines}} = \alpha \times \sigma'$$

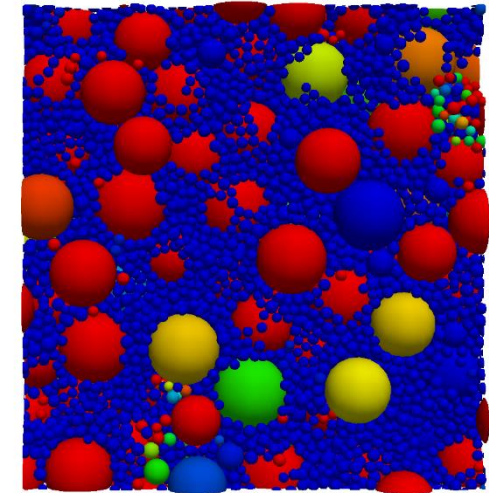
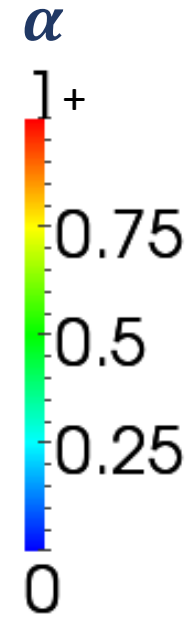
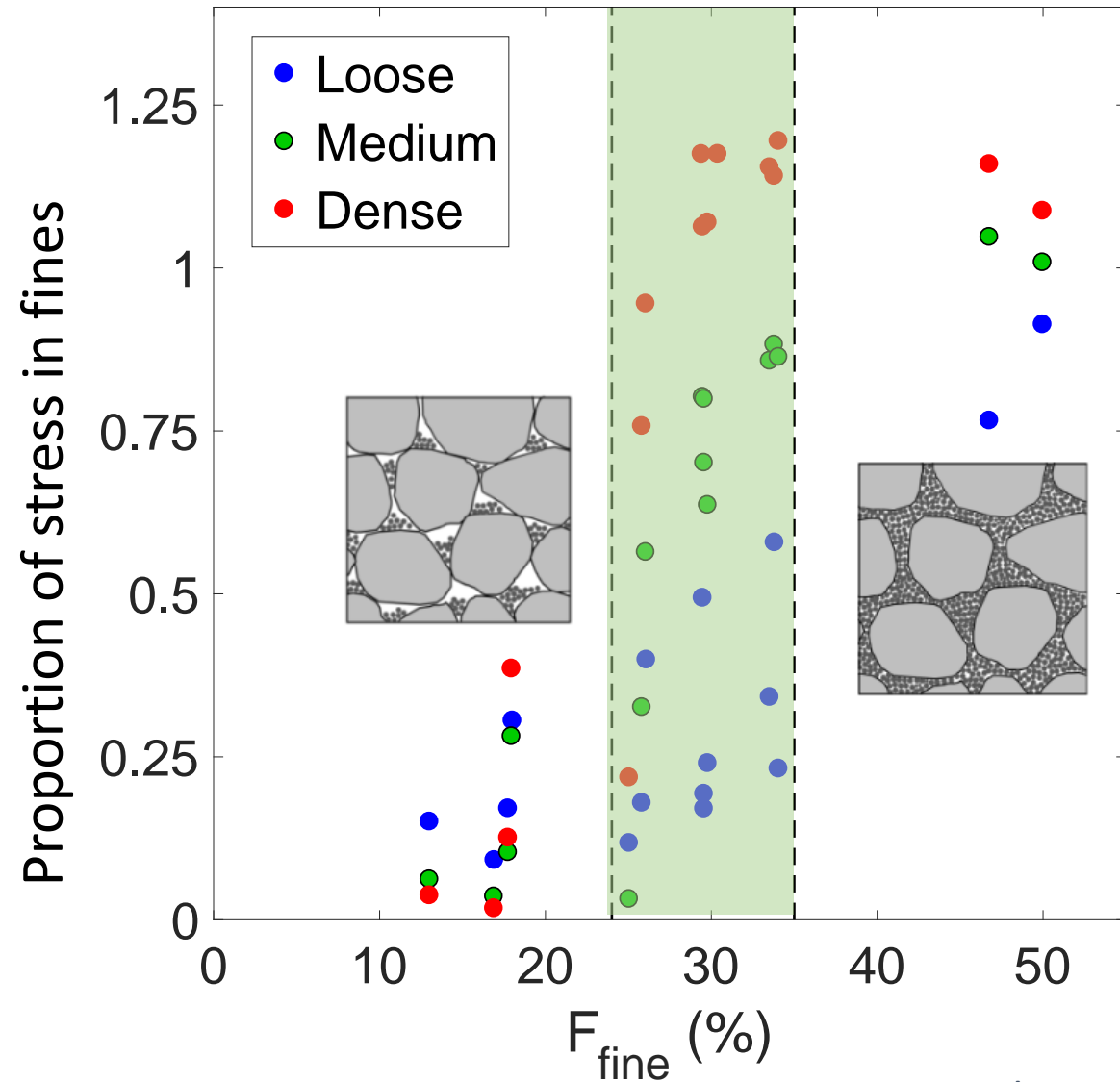
α is proportion of stress carried by finer fraction

Variation in α with Fines Content (F_{fine})

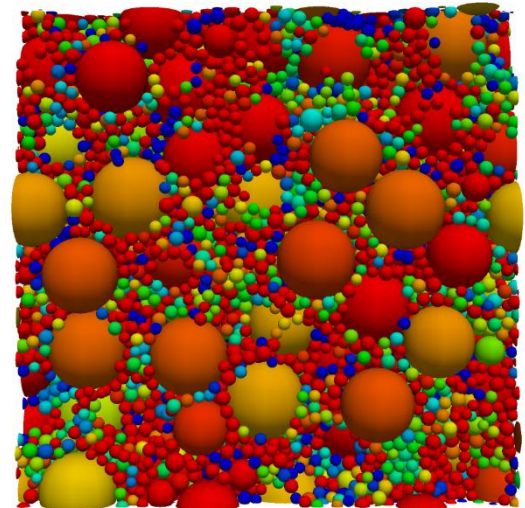


- Critical fines content where fines just fill voids: $F_{\text{fine}} = 24\text{-}29\%$
- Finer fraction separates coarse fraction particles: $F_{\text{fine}} = 35\%$
- Confirms hypotheses of Skempton and Brogan (1994)

Variation in α with Fines Content (F_{fine})

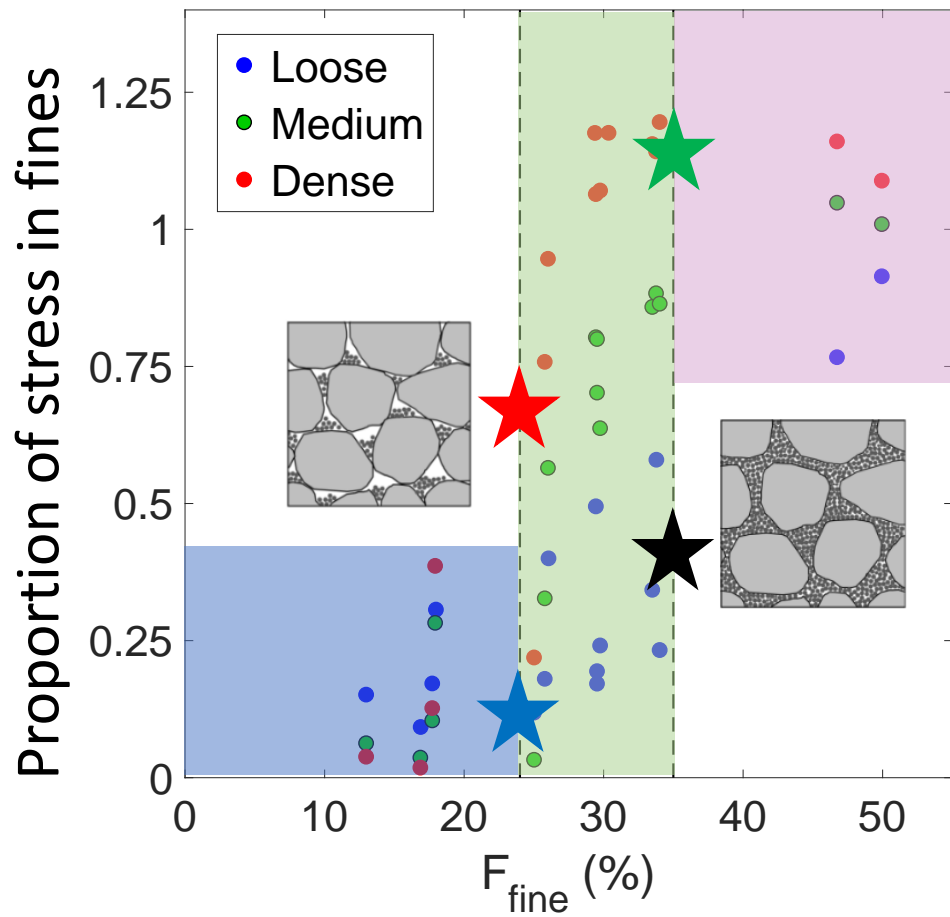


30% Fines - Loose

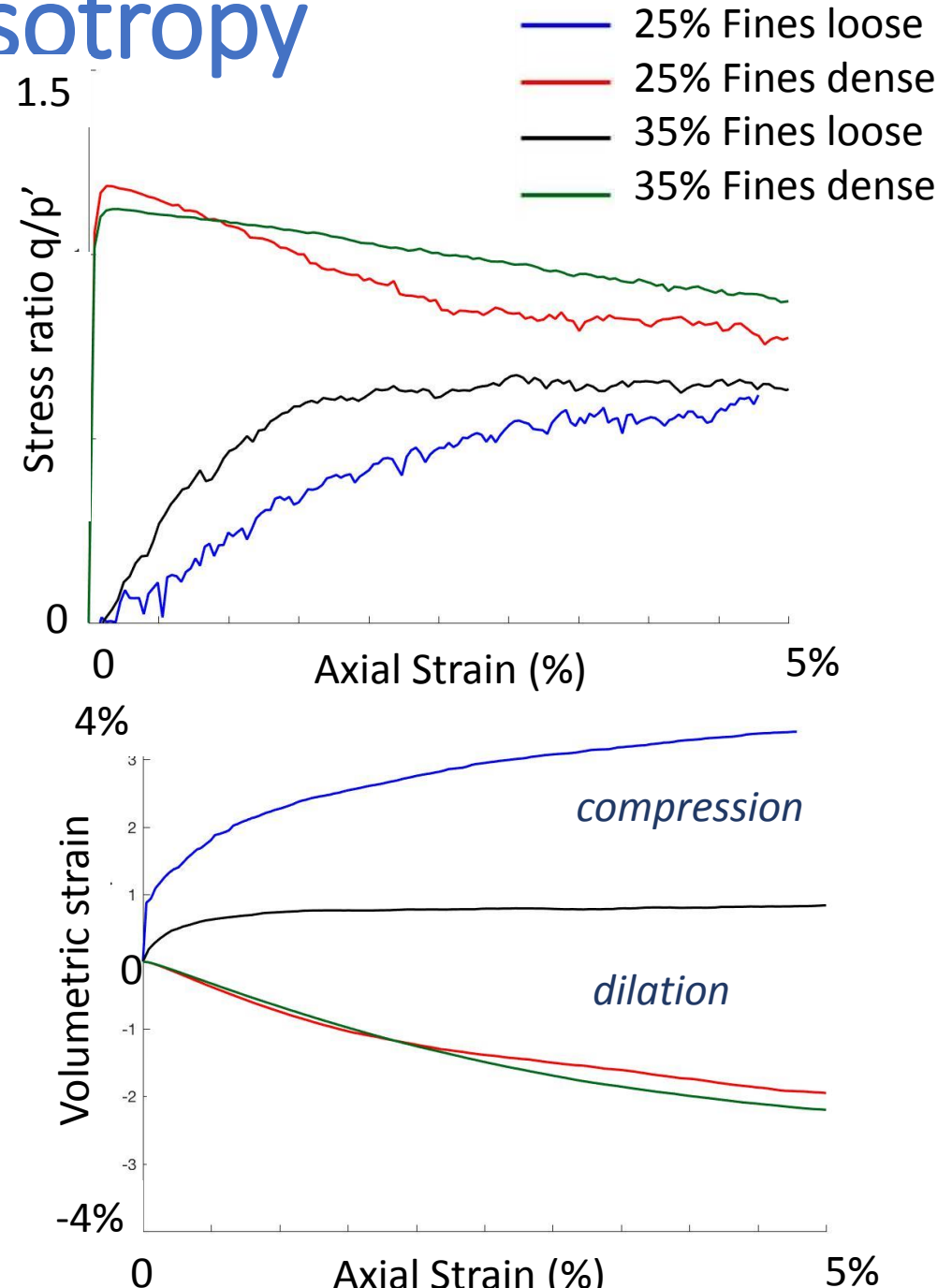


30% Fines - Dense

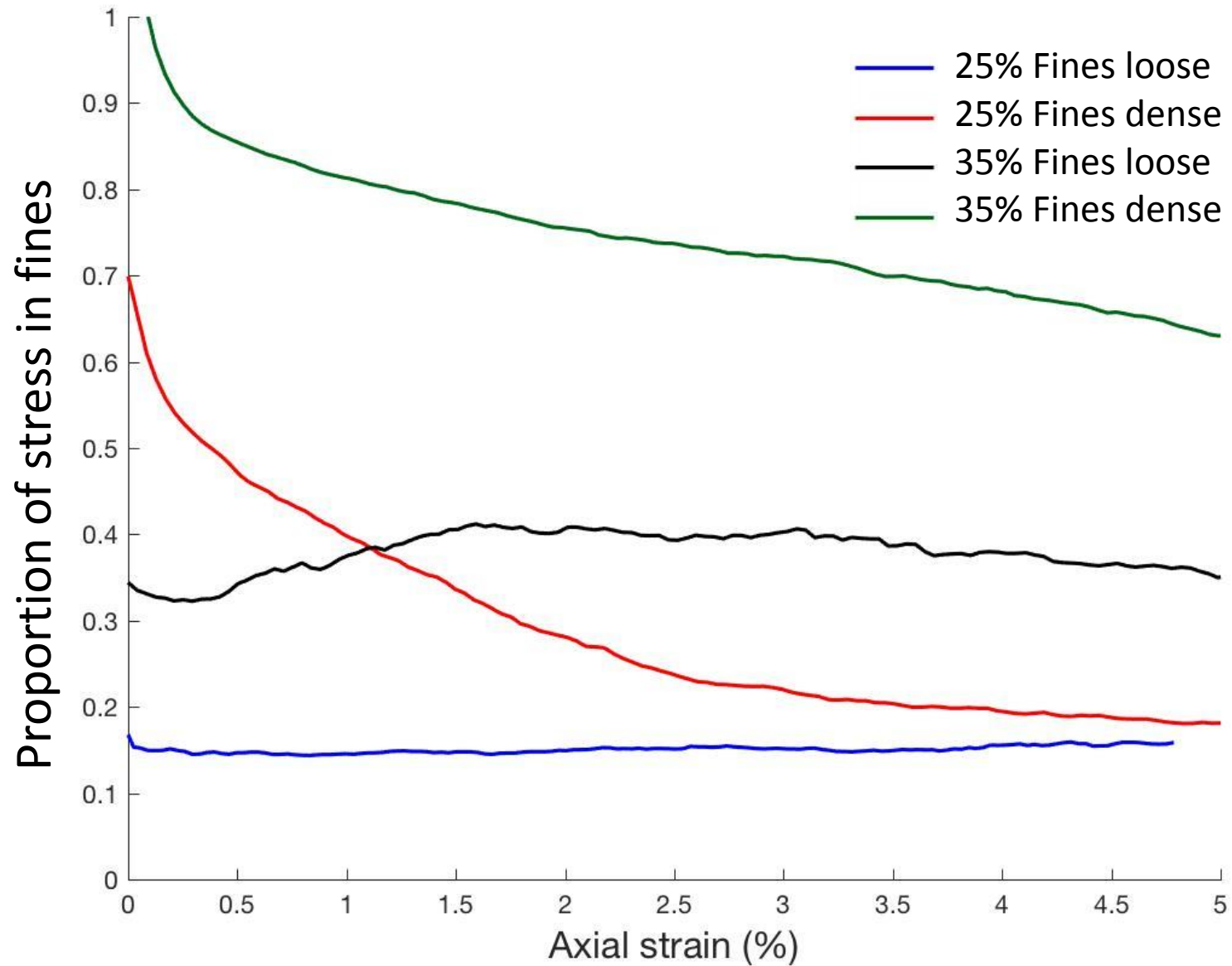
Variation in α with stress anisotropy



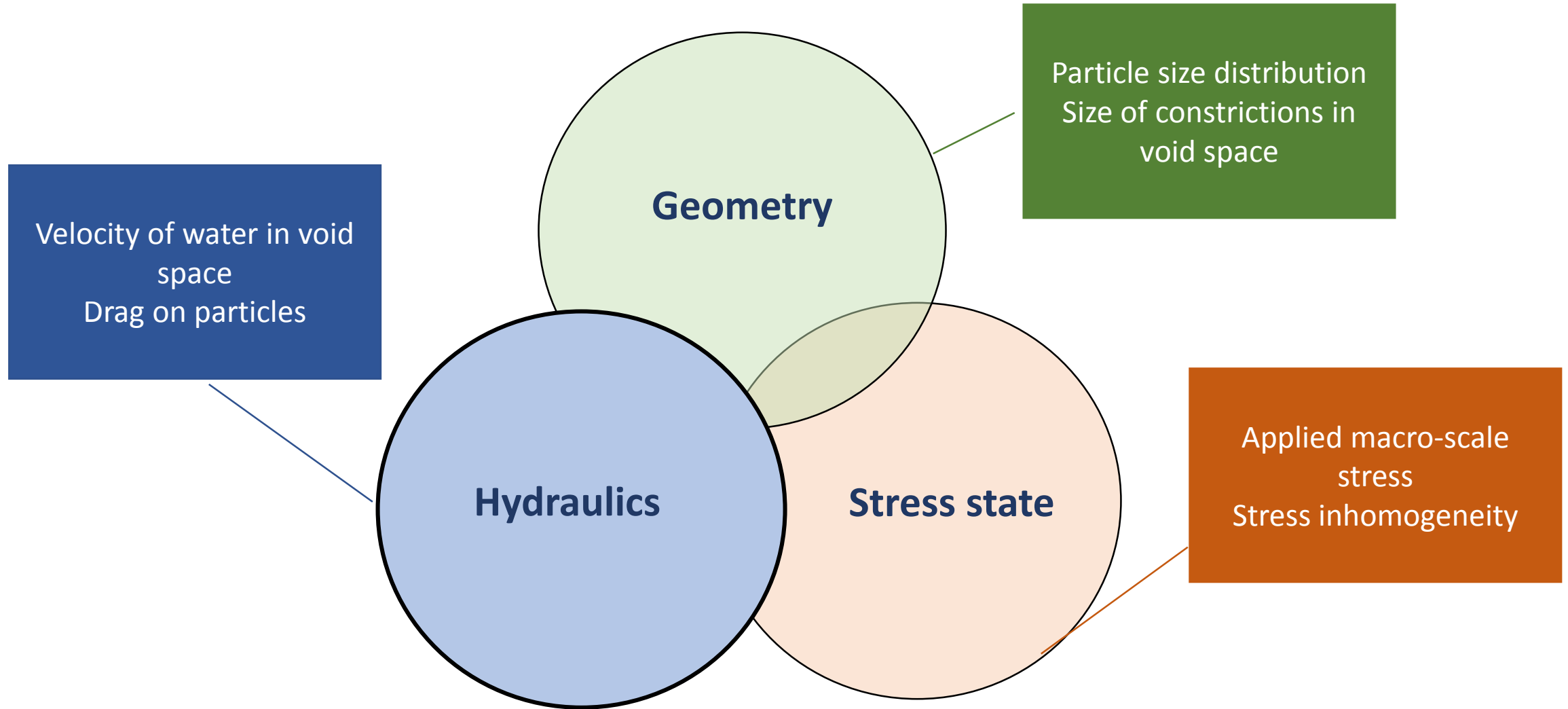
Triaxial
compression
– constant p'



Variation in α with stress anisotropy

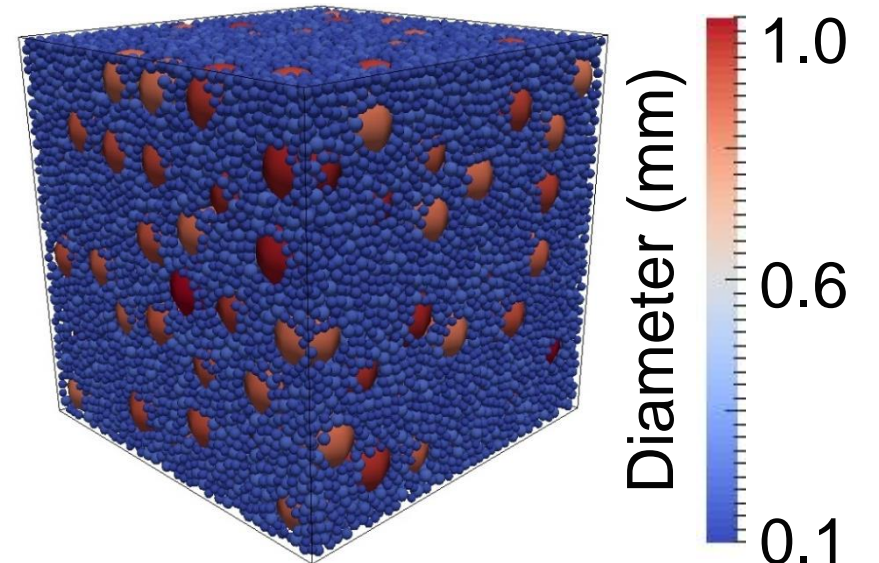
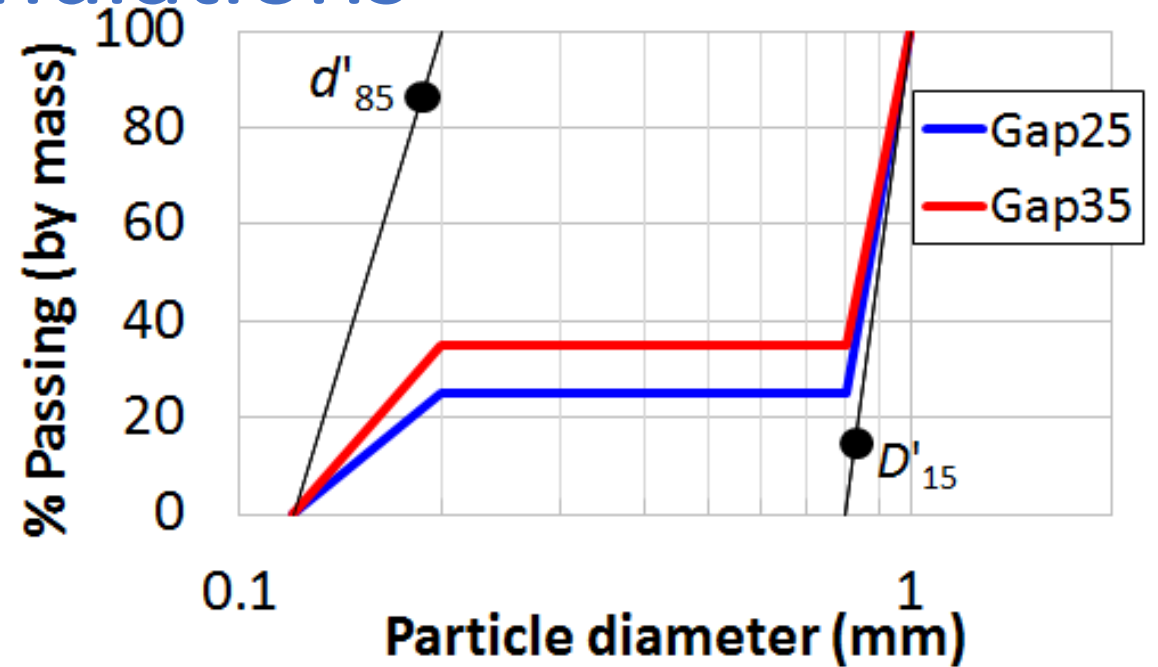


Factors influencing internal instability risk

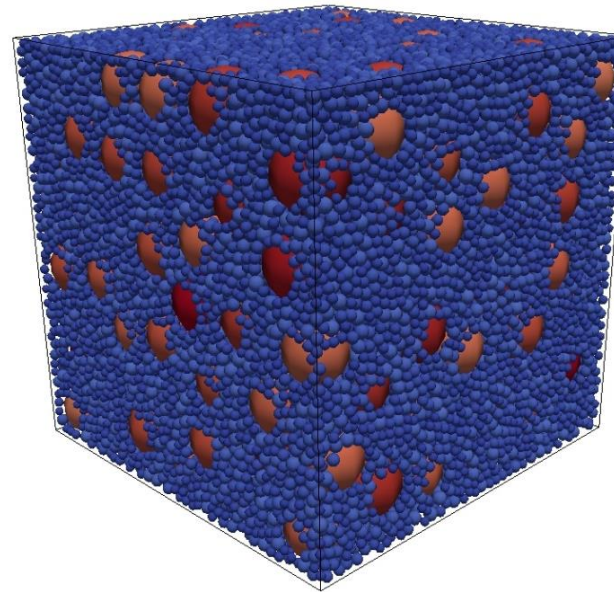
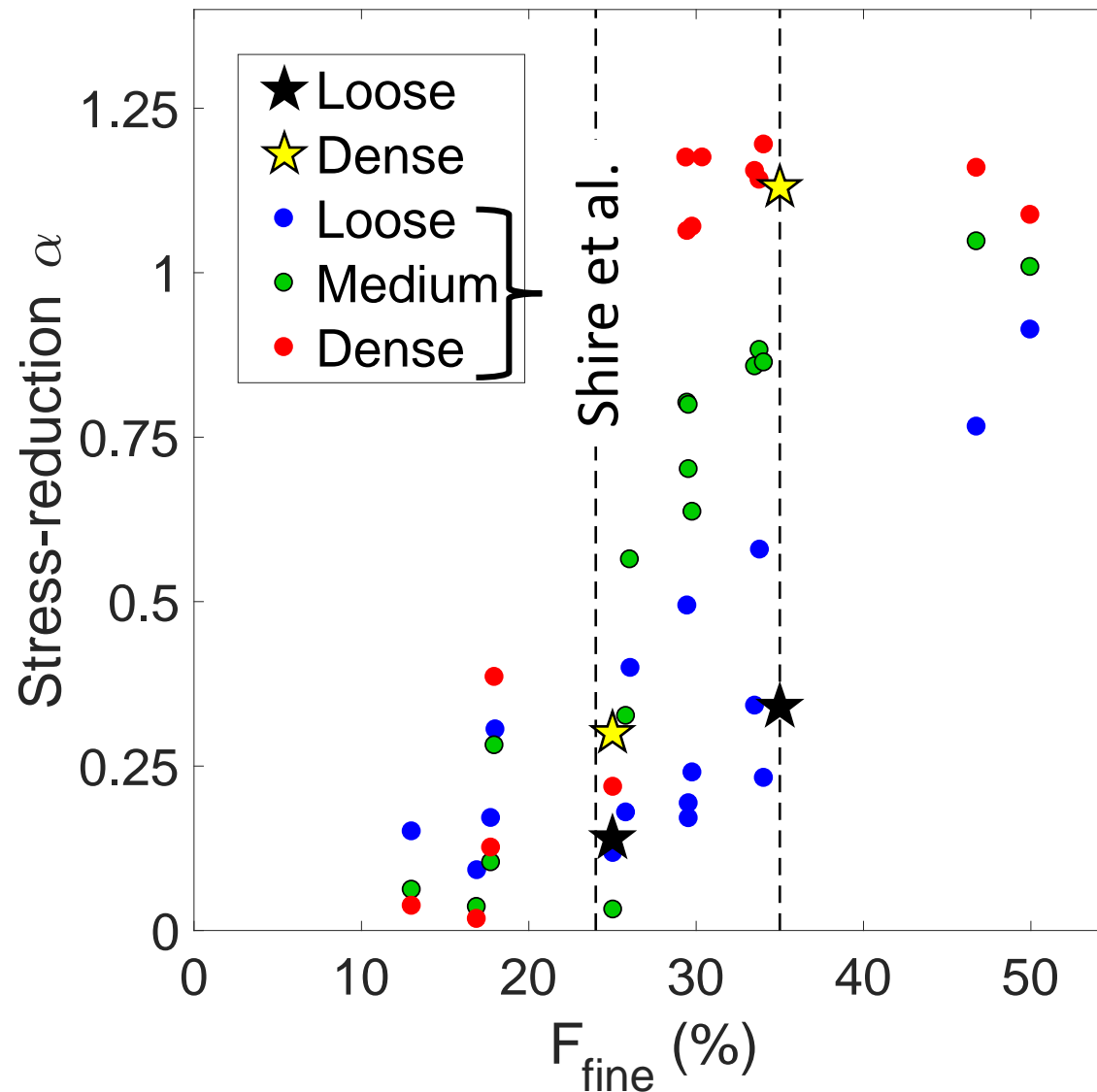


Permeameter test simulations

- PFC 3D Coupled with CCFD
- Circa 30,000 particles
- Di Felice drag expression
- Particle assembly: 6.1 mm cube
- Fluid cell size: 1.2 mm



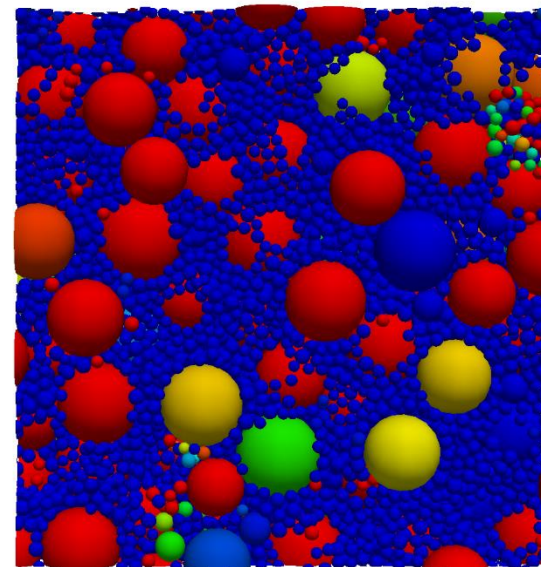
Virtual permeameter test samples



Current study:

PFC+CCFD

Rigid walls



Shire et al.

LAMMPS

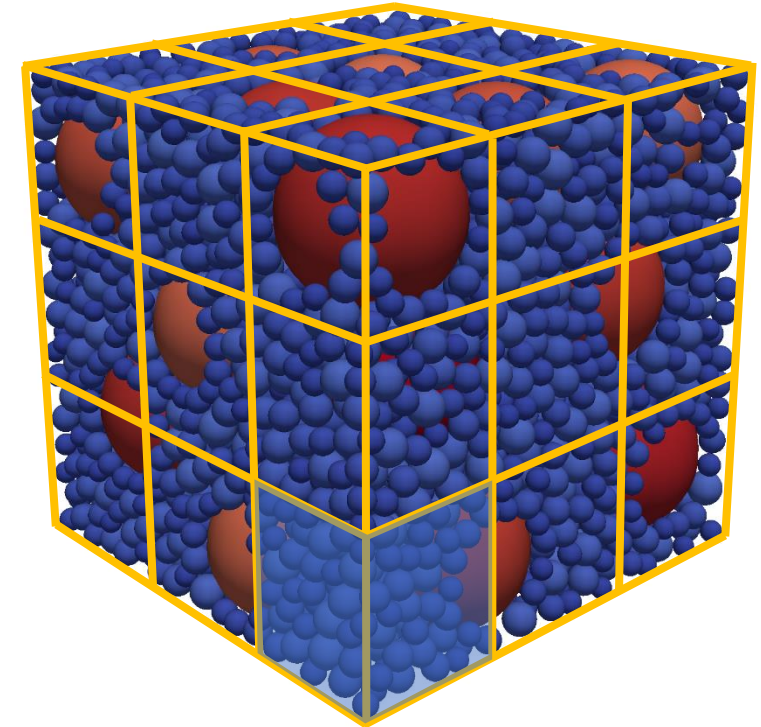
Periodic boundaries

Permeameter test simulations

Combination of DEM (PFC3D) and CFD (CCFD)

- DEM for soil particles
- CFD for water seepage

Coarse grid method proposed by Tsuji



Data exchange

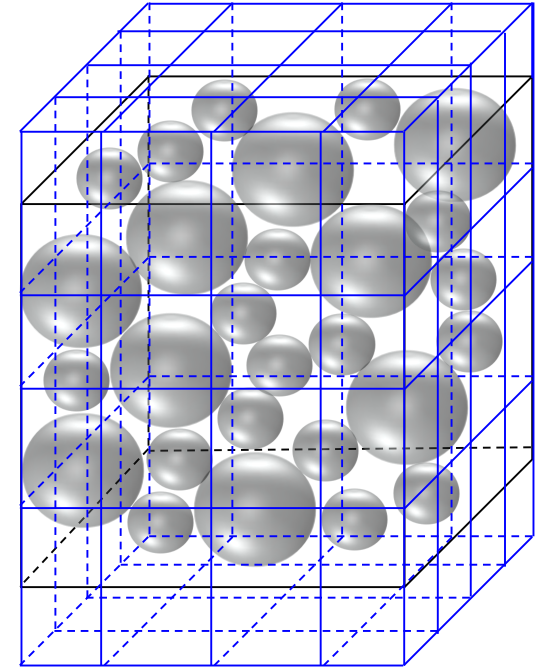
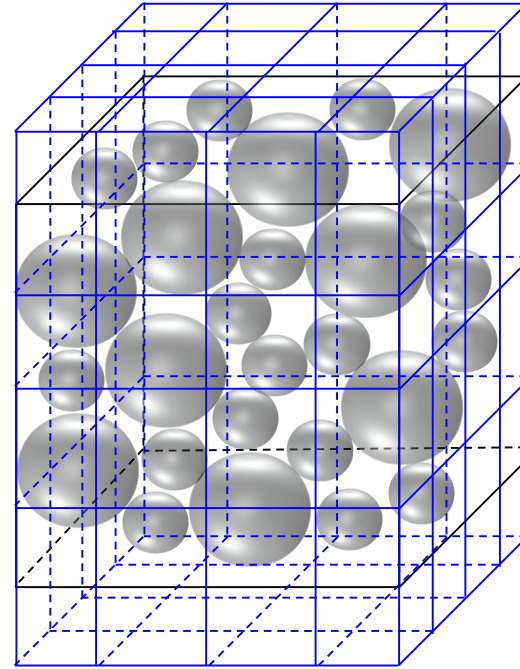
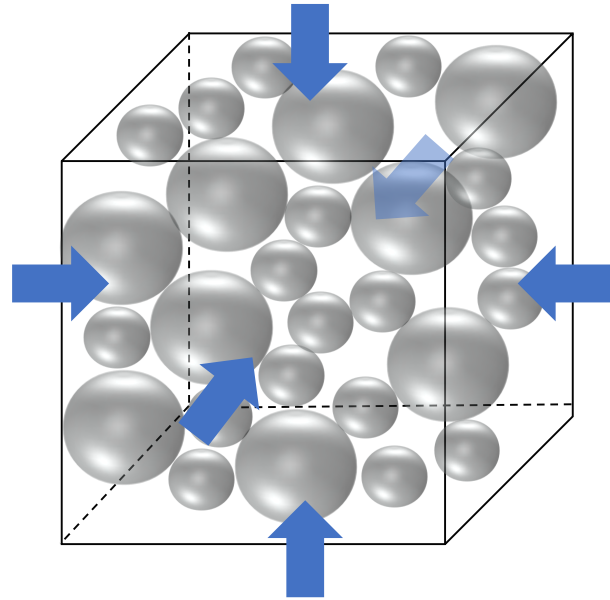
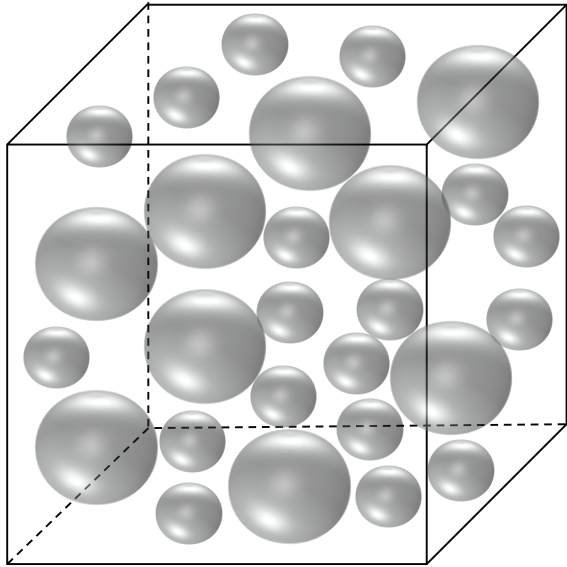


-Porosity
-Drag force

-Fluid velocity
-Fluid pressure gradient

(Tsuji et al., 1993, Xu and Yu, 1997)

Permeameter test simulations



Create non-contacting
cloud of spheres

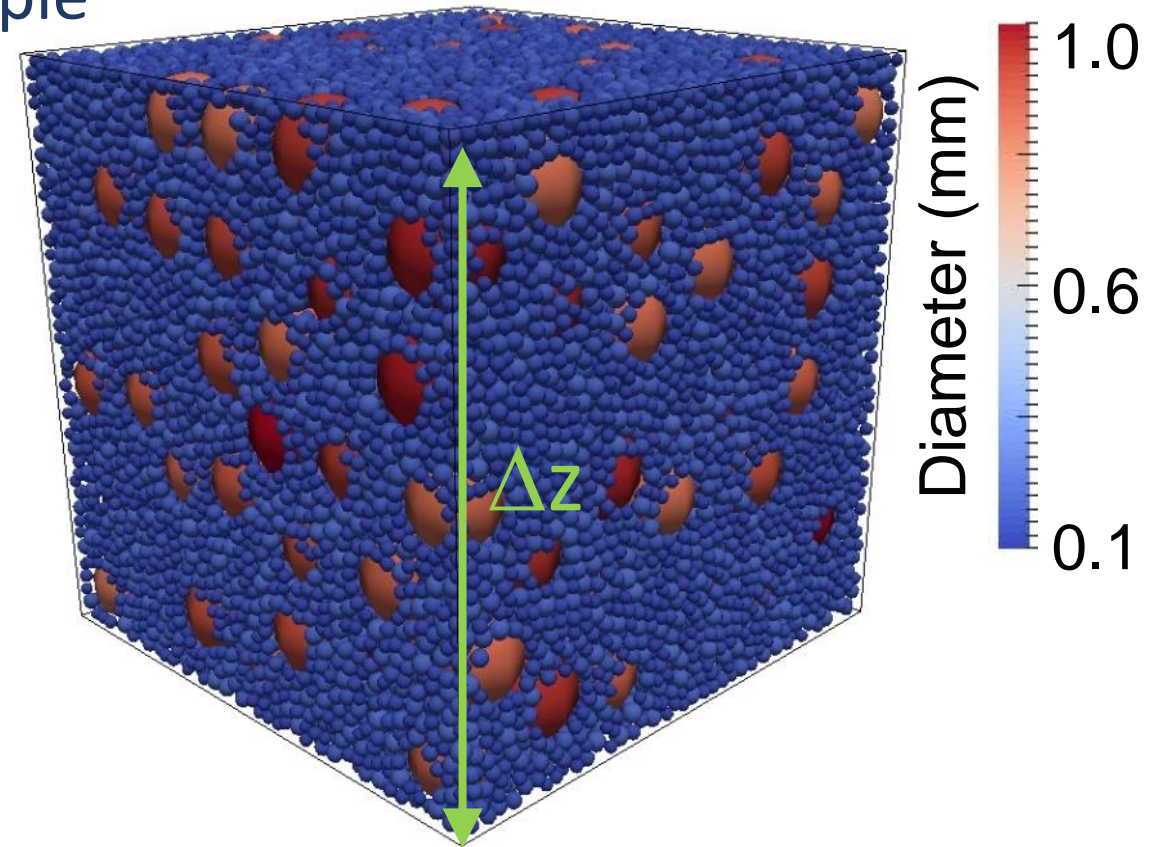
Compress to 50kPa,
Apply gravity

Create fluid mesh,
Fix boundaries,
Fix particle positions,
Apply pressure gradient

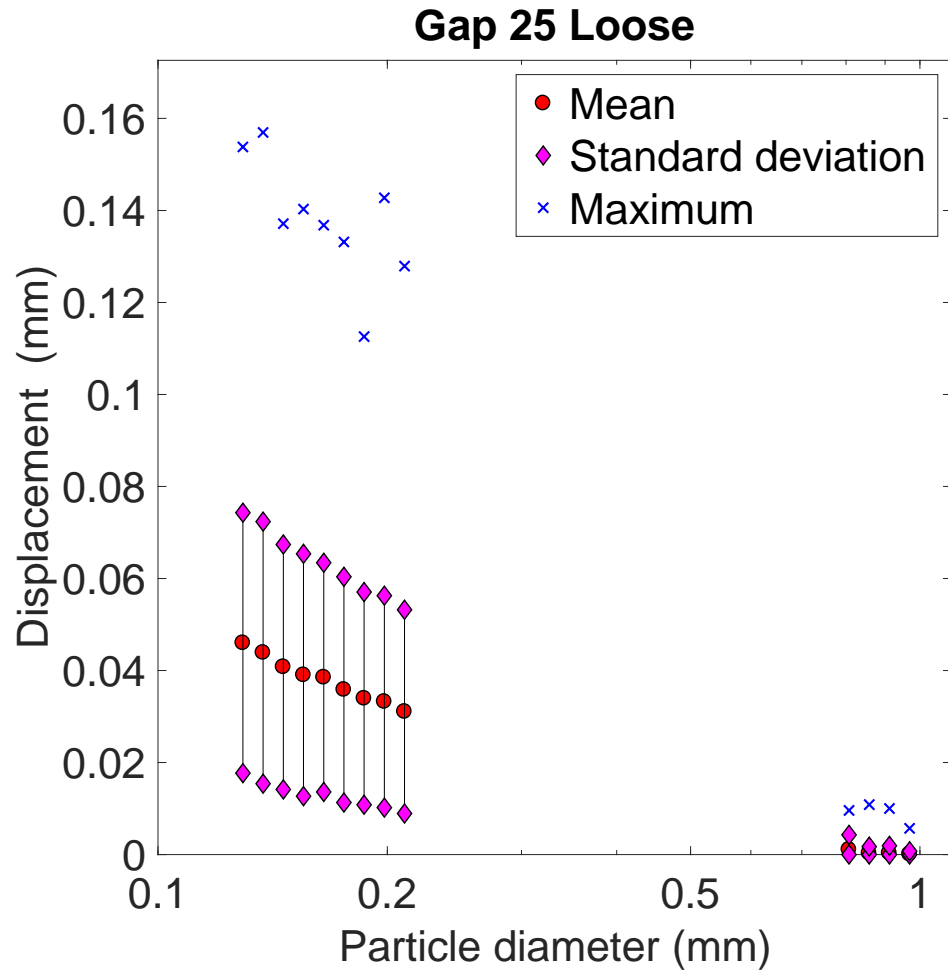
Steady state fluid,
Release particles,
Monitor response

Permeameter test simulations

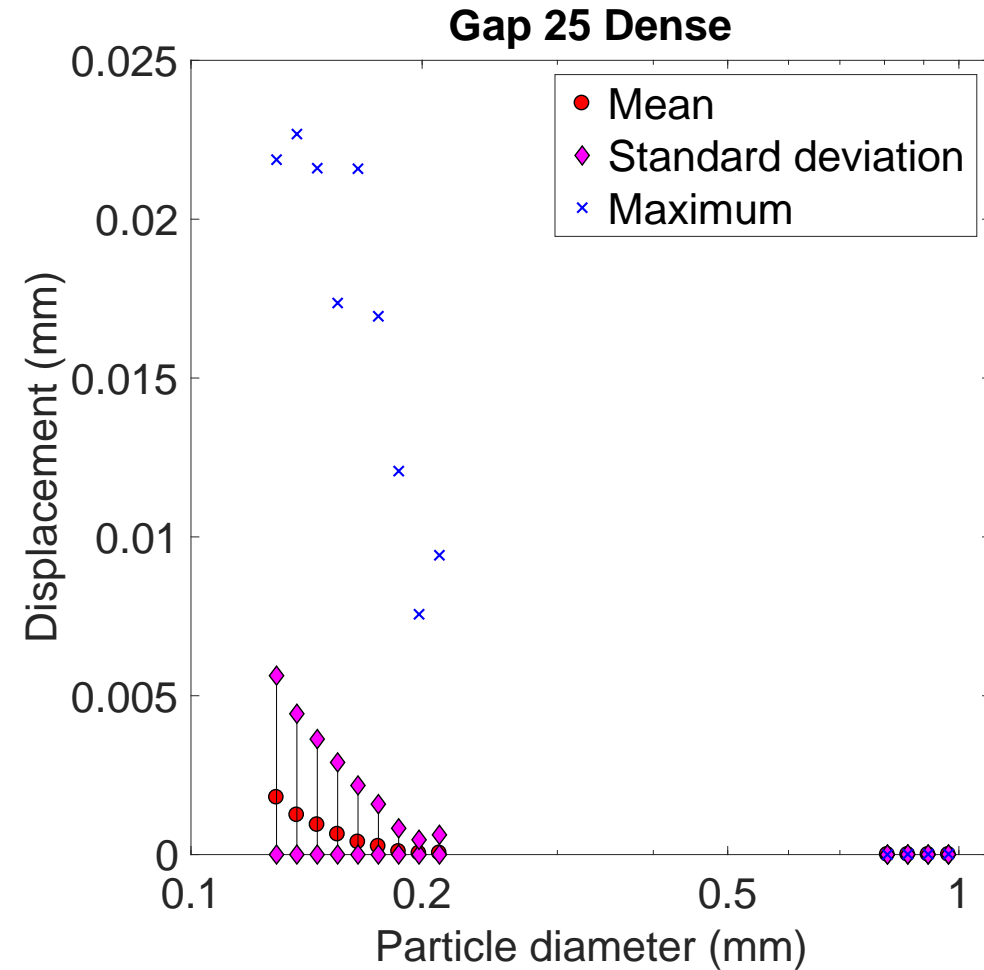
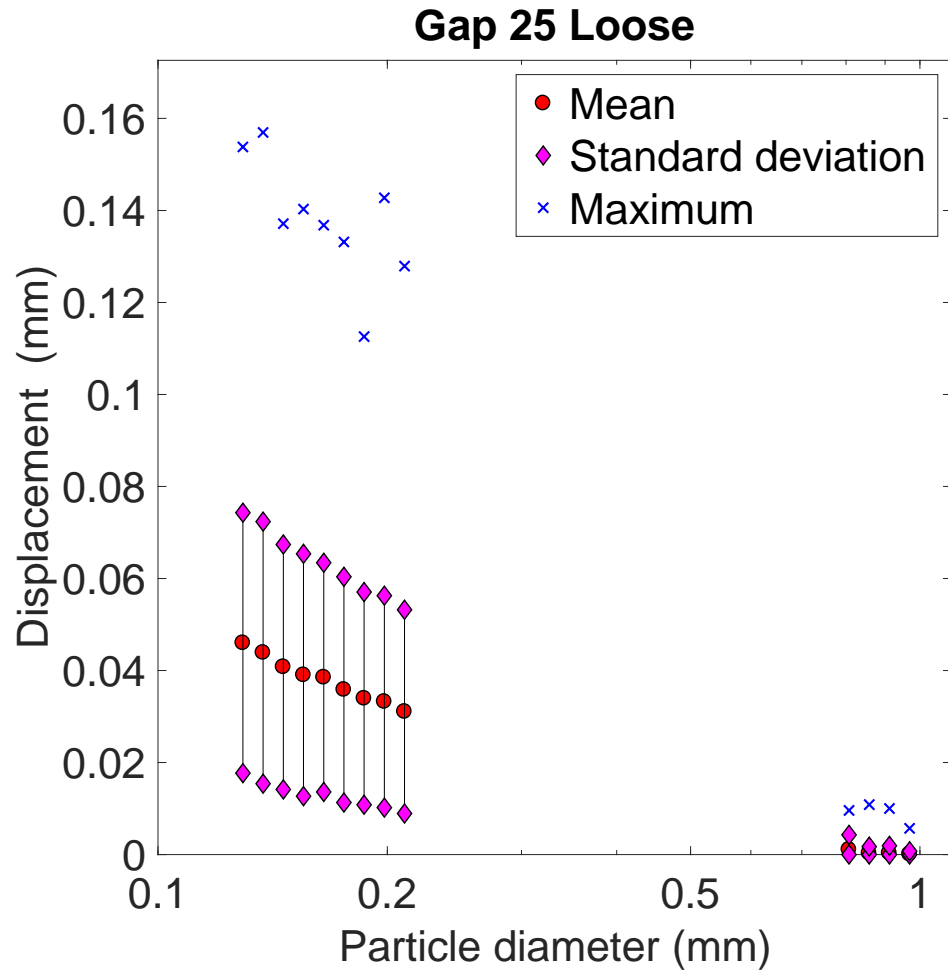
- Applied pressure differential across sample (Δp)
- Increased hydraulic gradient (i) in steps
- As samples small
- $$i = \frac{\Delta h}{\Delta z} \approx \frac{\Delta p}{\gamma_w \Delta z}$$
- Δh =head drop across sample
- γ_w = unit weight of water
- Simulation gives permeability $k \approx 5 \times 10^{-3}$ m/s



Particle displacements – for $i = 1$



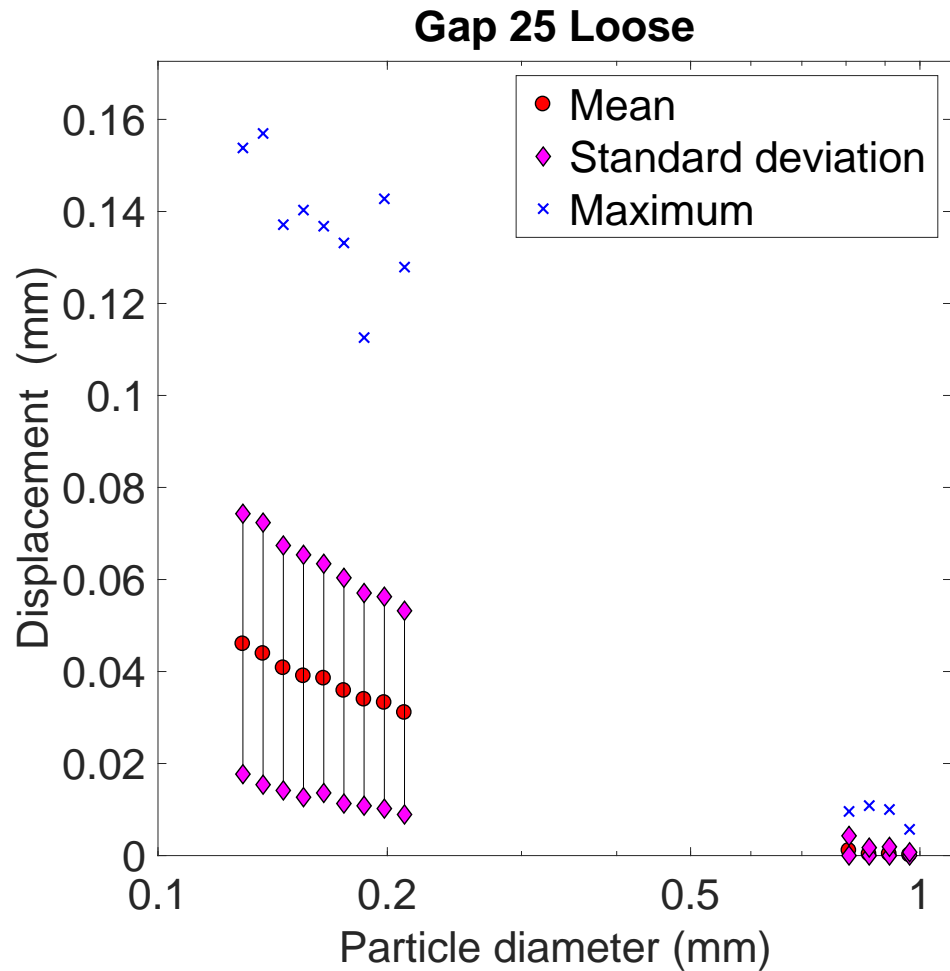
Particle displacements – for $i = 1$



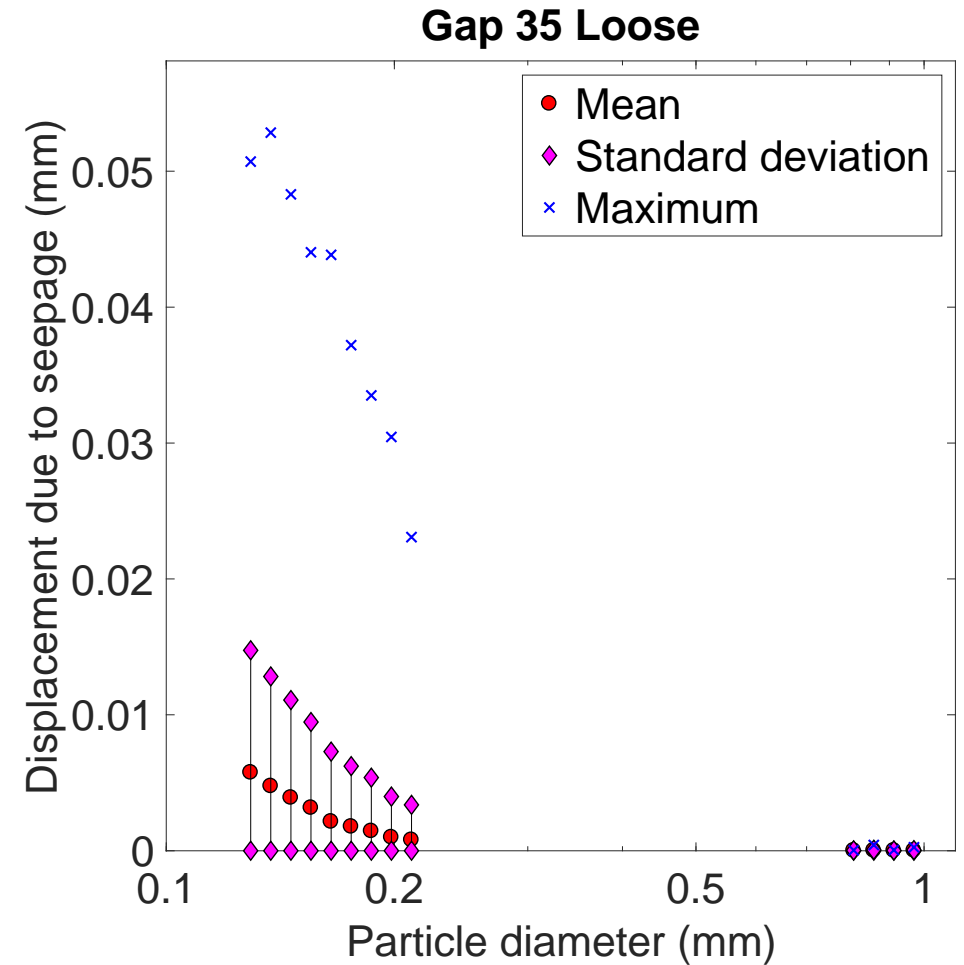
Increase in density



Particle displacements – for $i = 1$



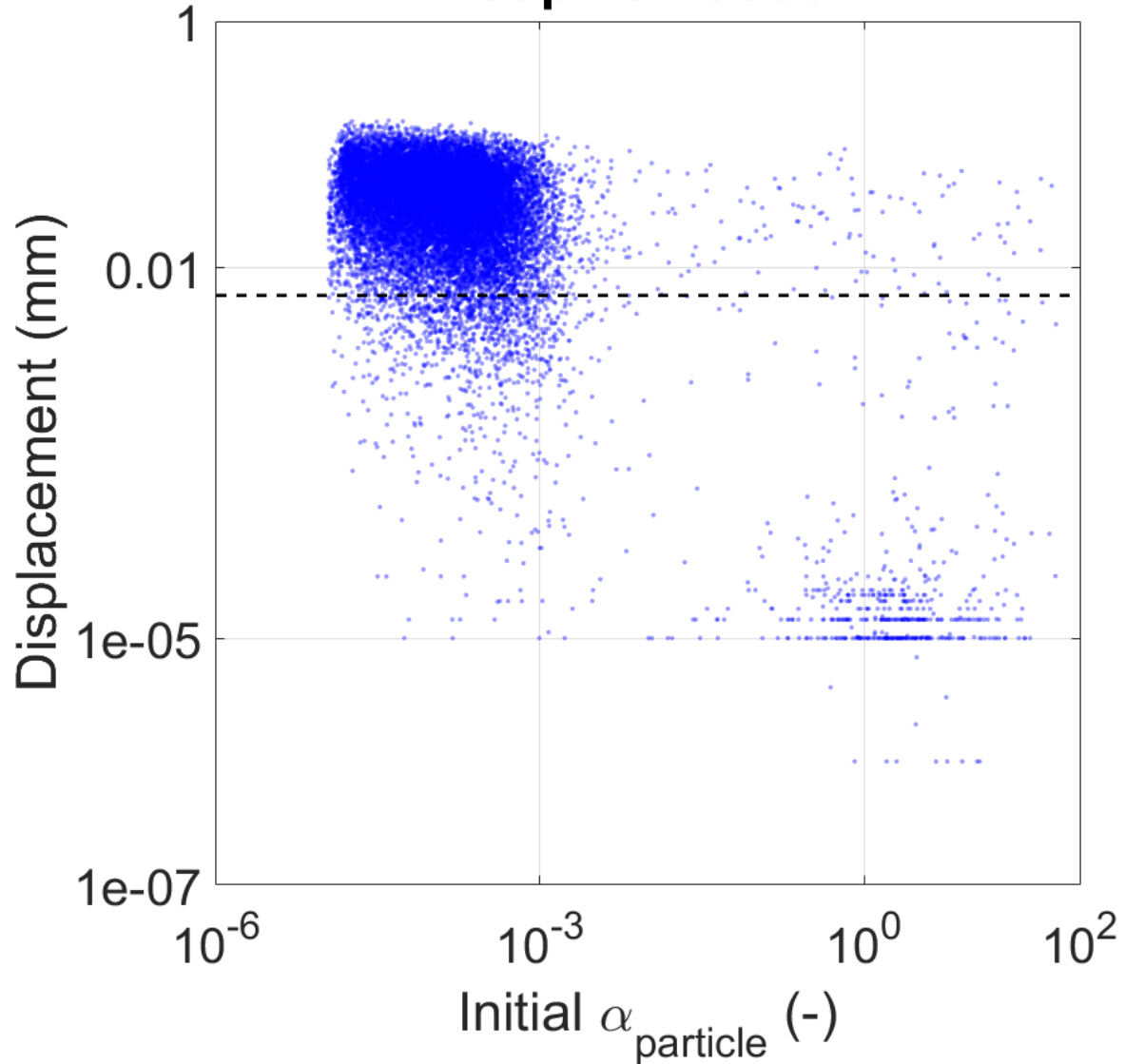
Just underfilled



Just overfilled

Particle displacements – for $i = 1$

Gap 25 Loose



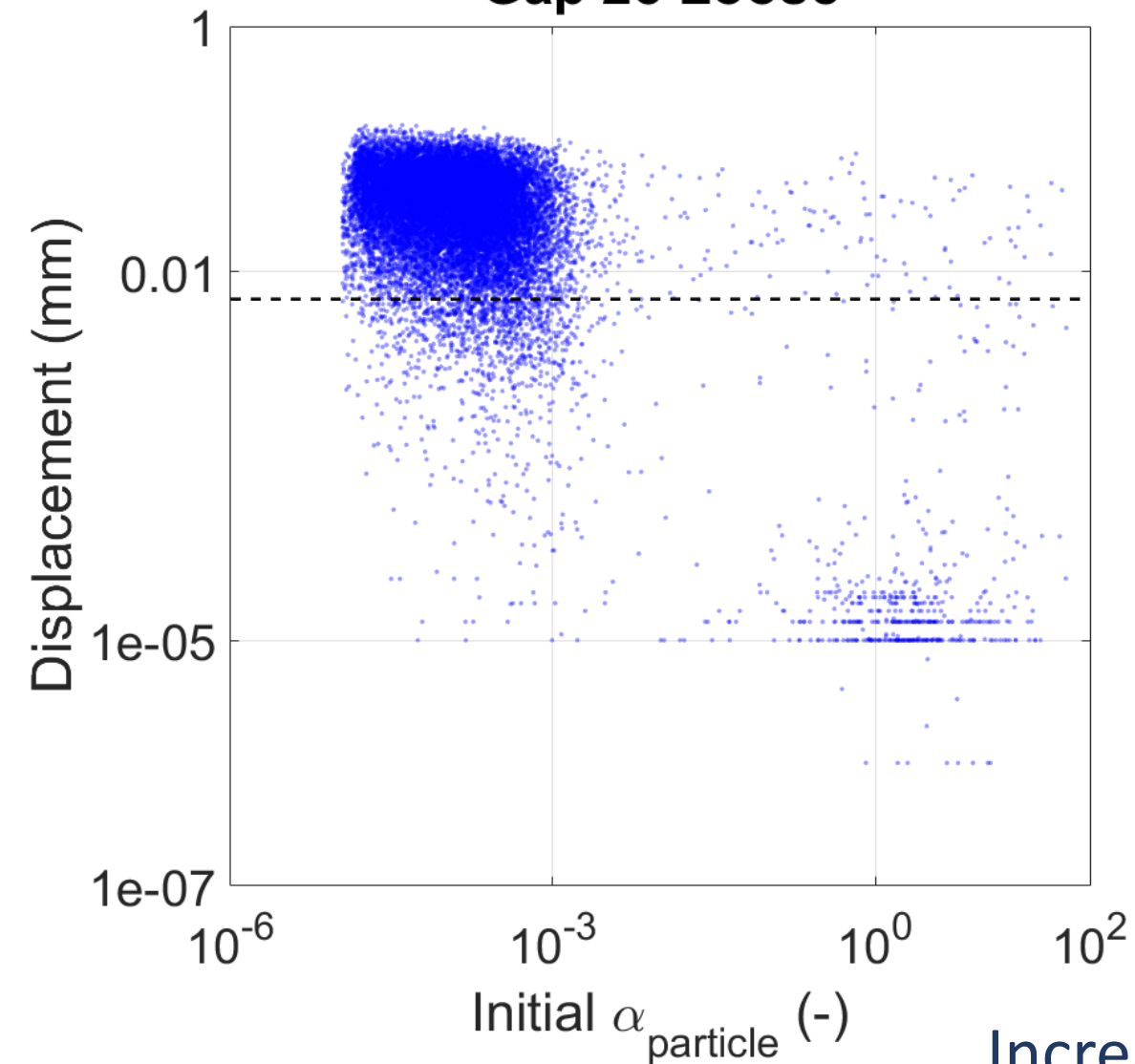
$$\alpha_{particle} = \frac{\sigma_{particle}}{\sigma_{overall}}$$

$\sigma_{particle}$ = average stress in a particle

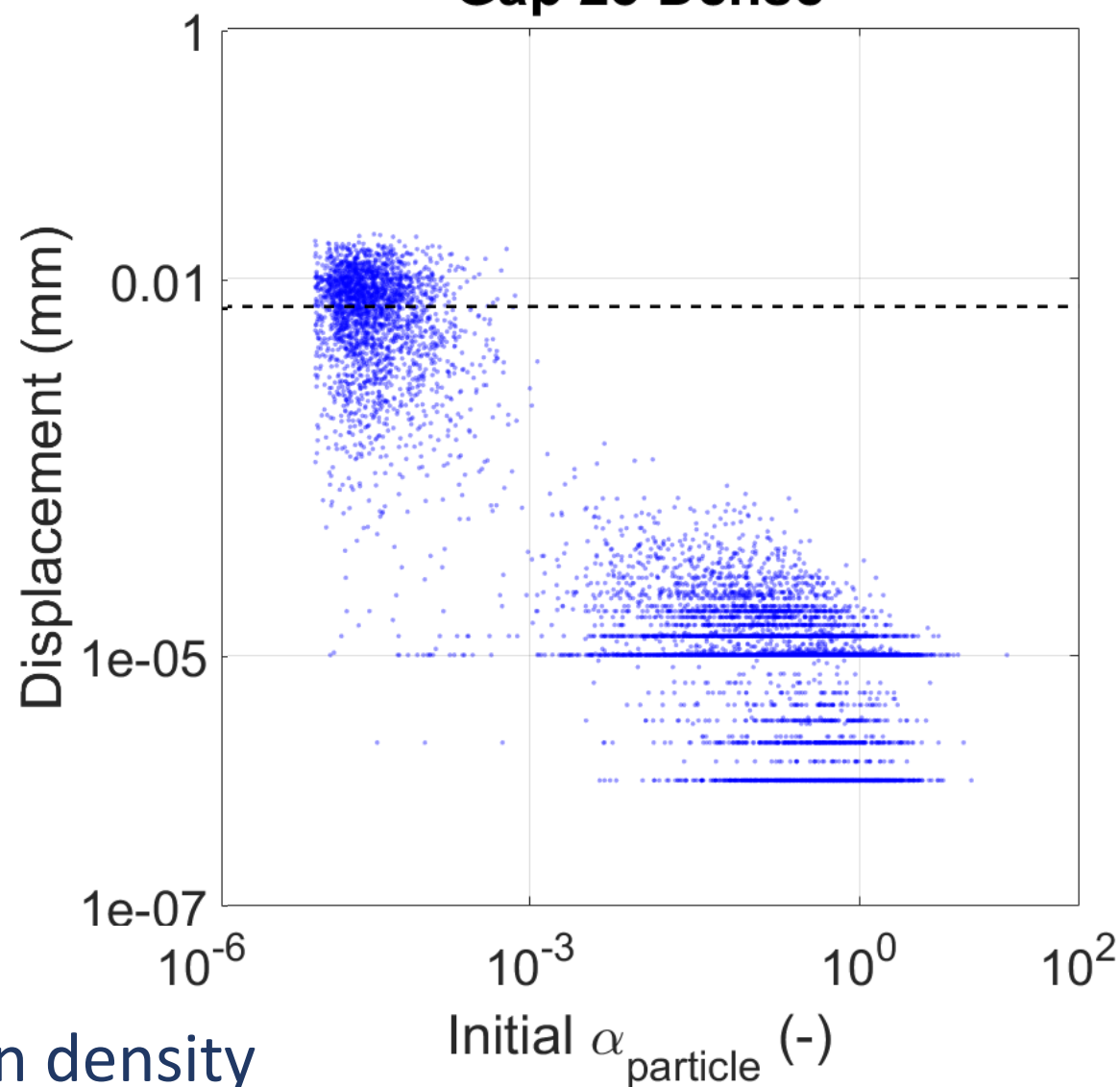
$\sigma_{overall}$ = overall sample stress

Particle displacements – for $i = 1$

Gap 25 Loose



Gap 25 Dense

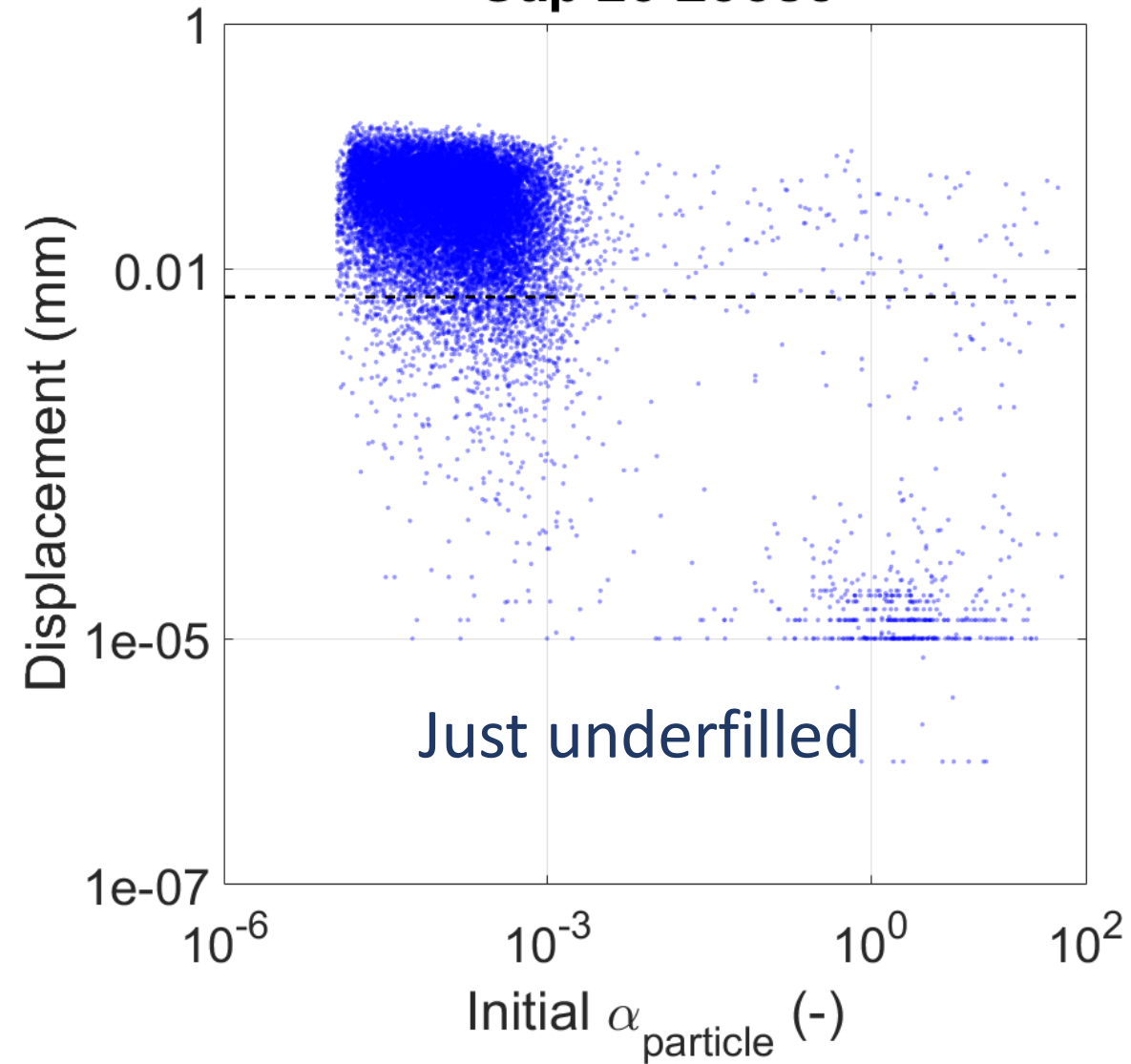


Increase in density

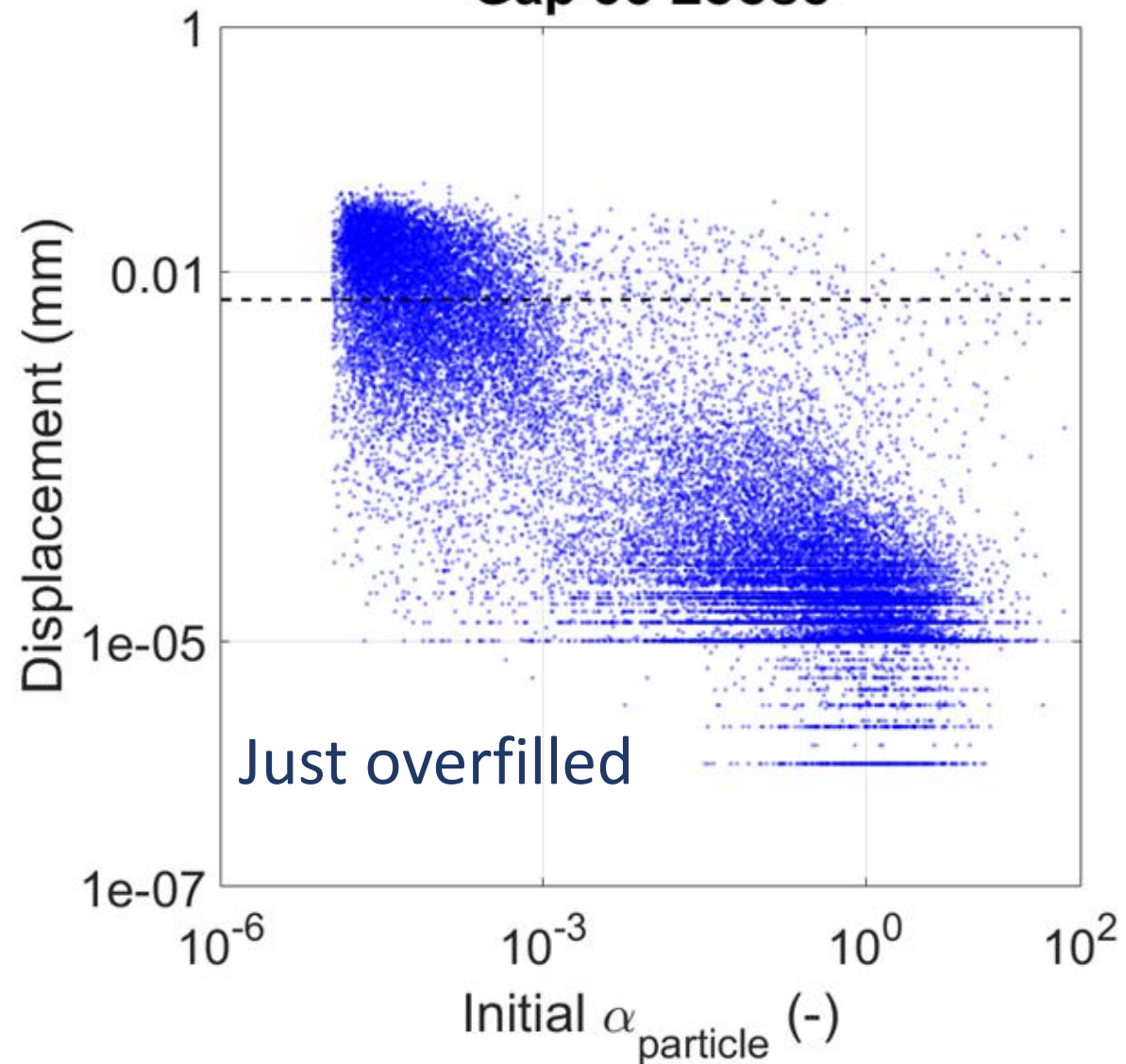


Particle displacements – for $i = 1$

Gap 25 Loose

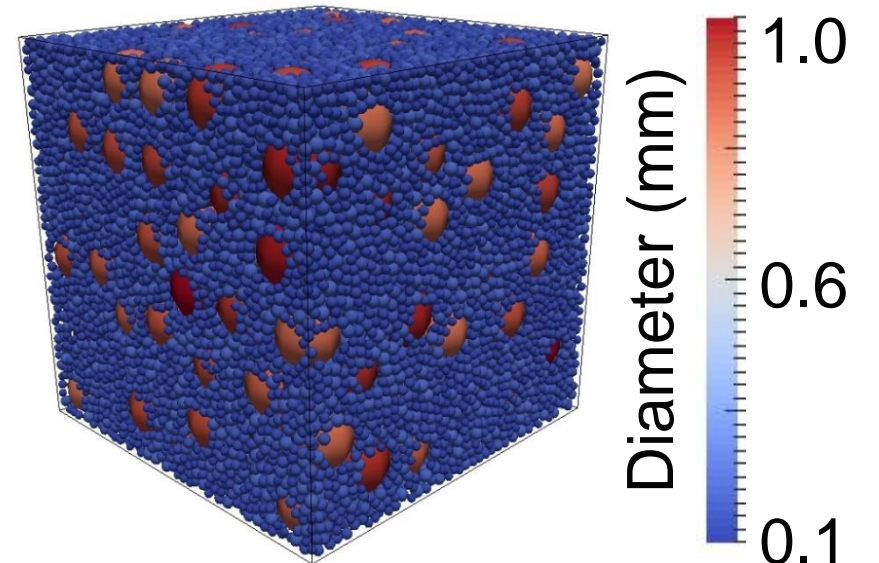
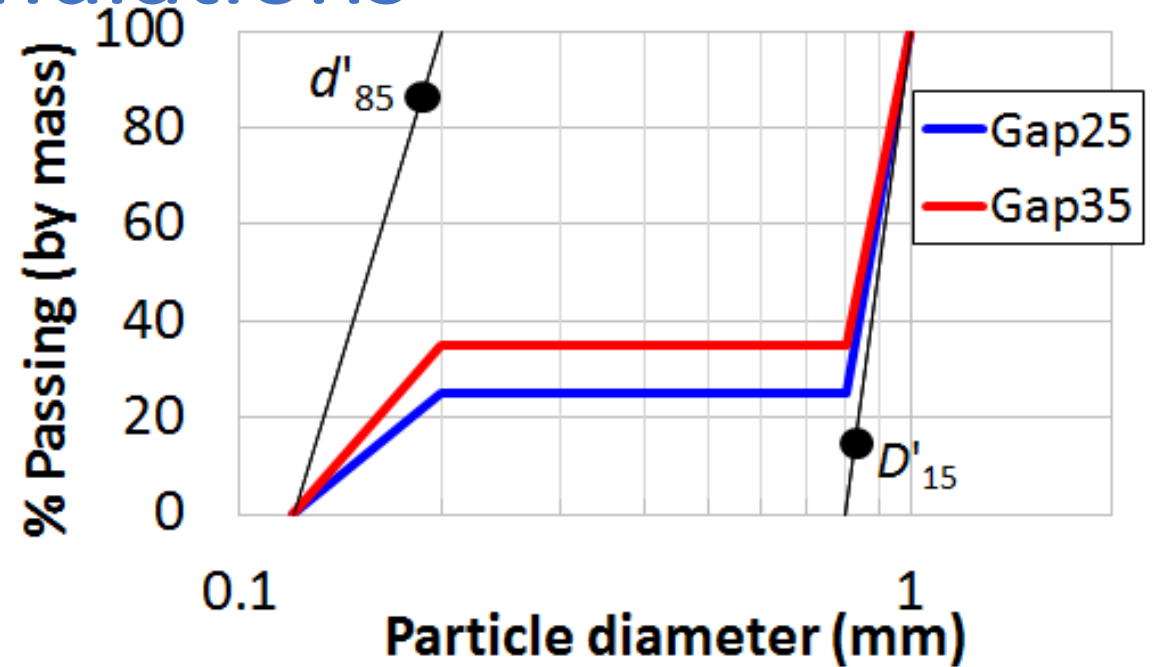


Gap 35 Loose



Permeameter test simulations

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- Circa 30,000 particles
- Di Felice drag expression
- Particle assembly: 6.1 mm cube
- Fluid cell size: 1.2 mm

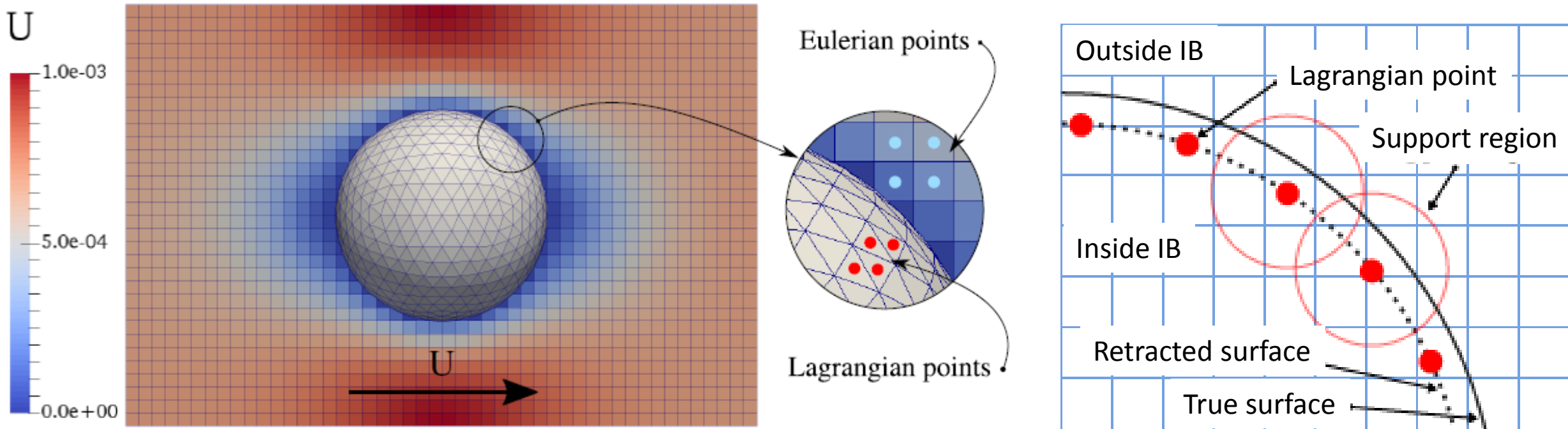


Immersed boundary method (IBM)

Allows simulation of fluid flow in void space (fixed, regular, Eulerian grid)

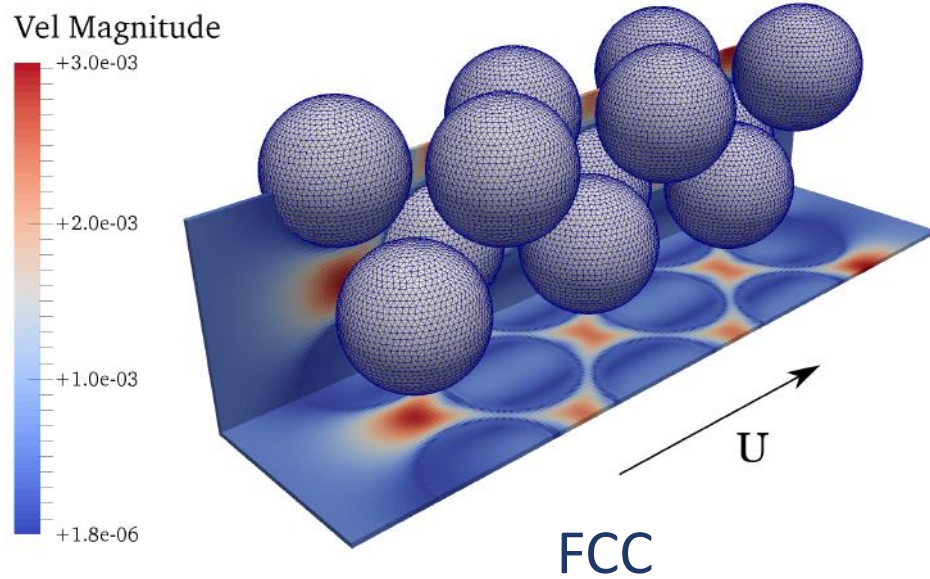
Fluid-particle interaction force can be determined

MultiFlow – B. van Wachem et al.



Verification simulations

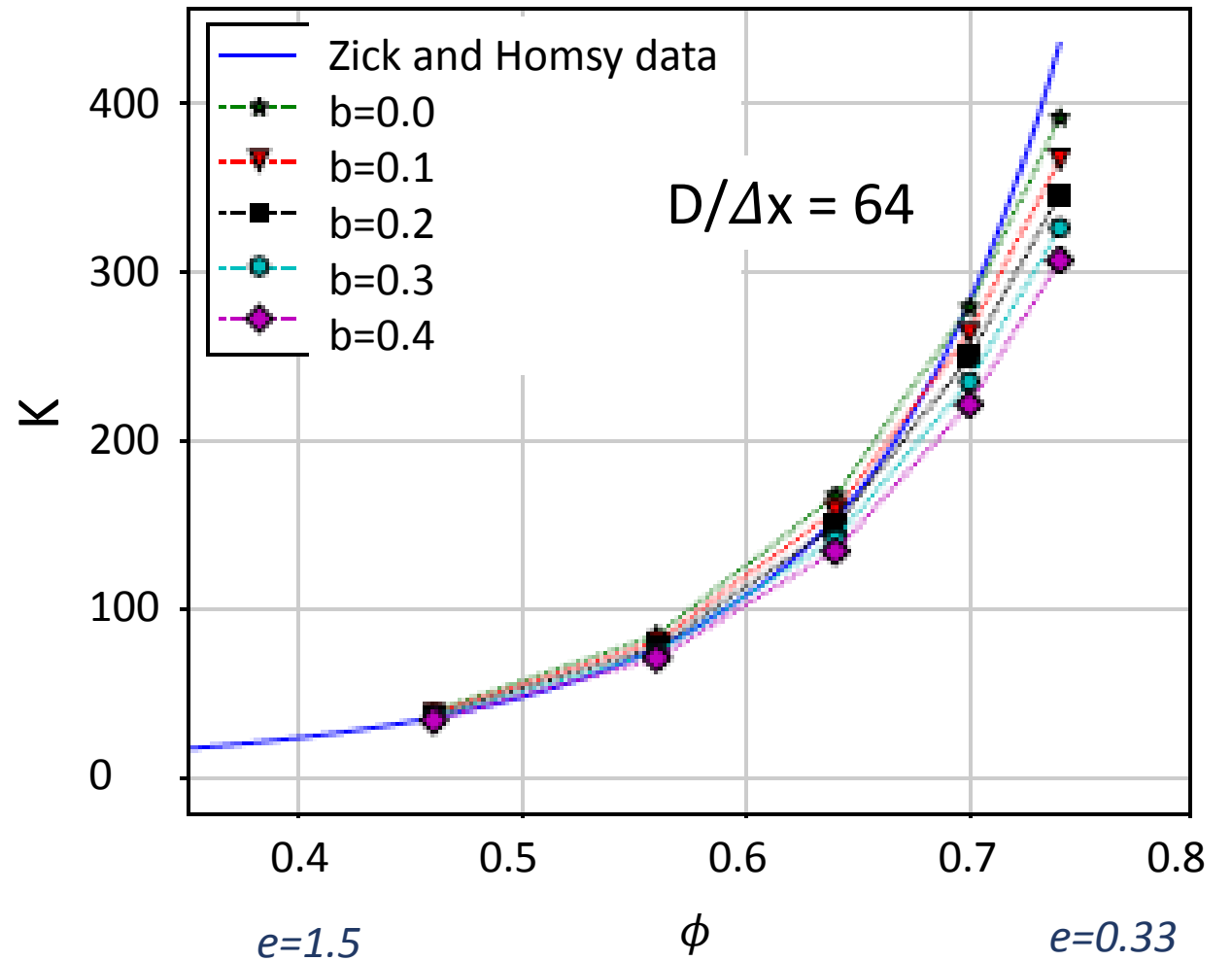
Zick and Homsy – boundary integral method data



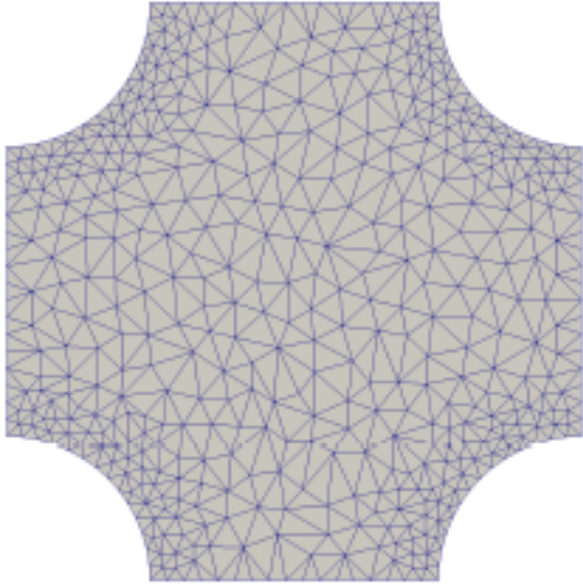
$\phi = \text{solids fraction} = V_{\text{solids}} / V_{\text{total}}$

$b = \text{radius retraction parameter}$
(retraction = $b\Delta x$)

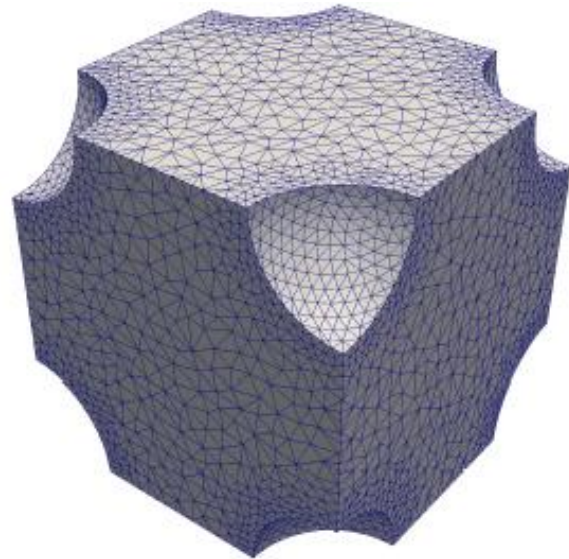
$K = \text{fluid particle interaction force} / \text{Stokes drag}$



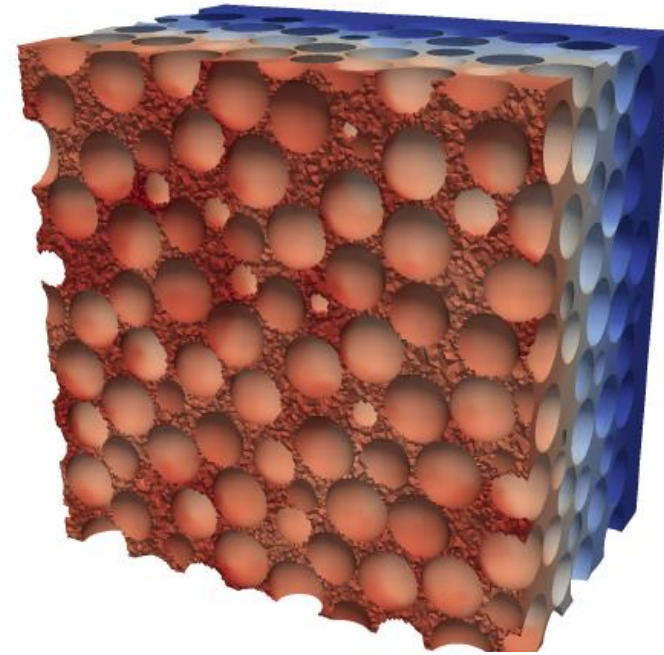
Verification simulations



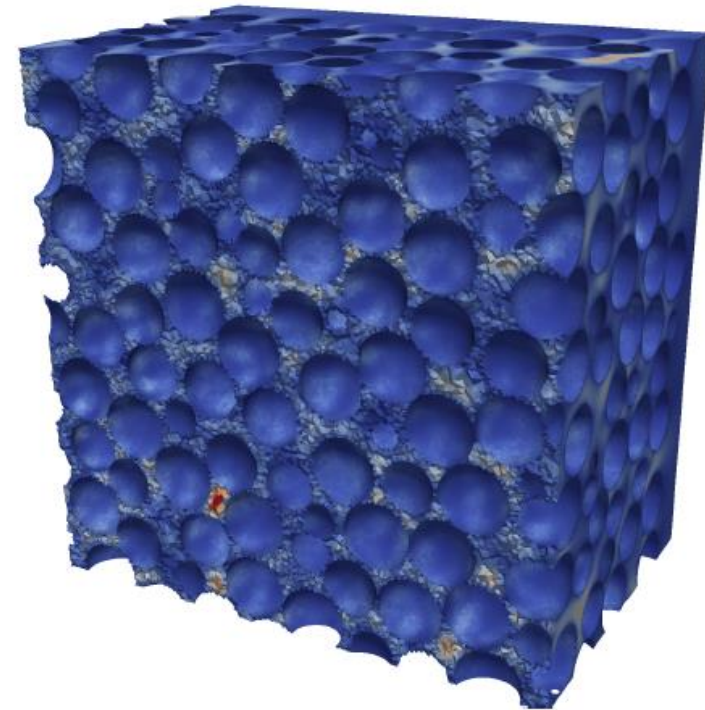
Body fitted mesh



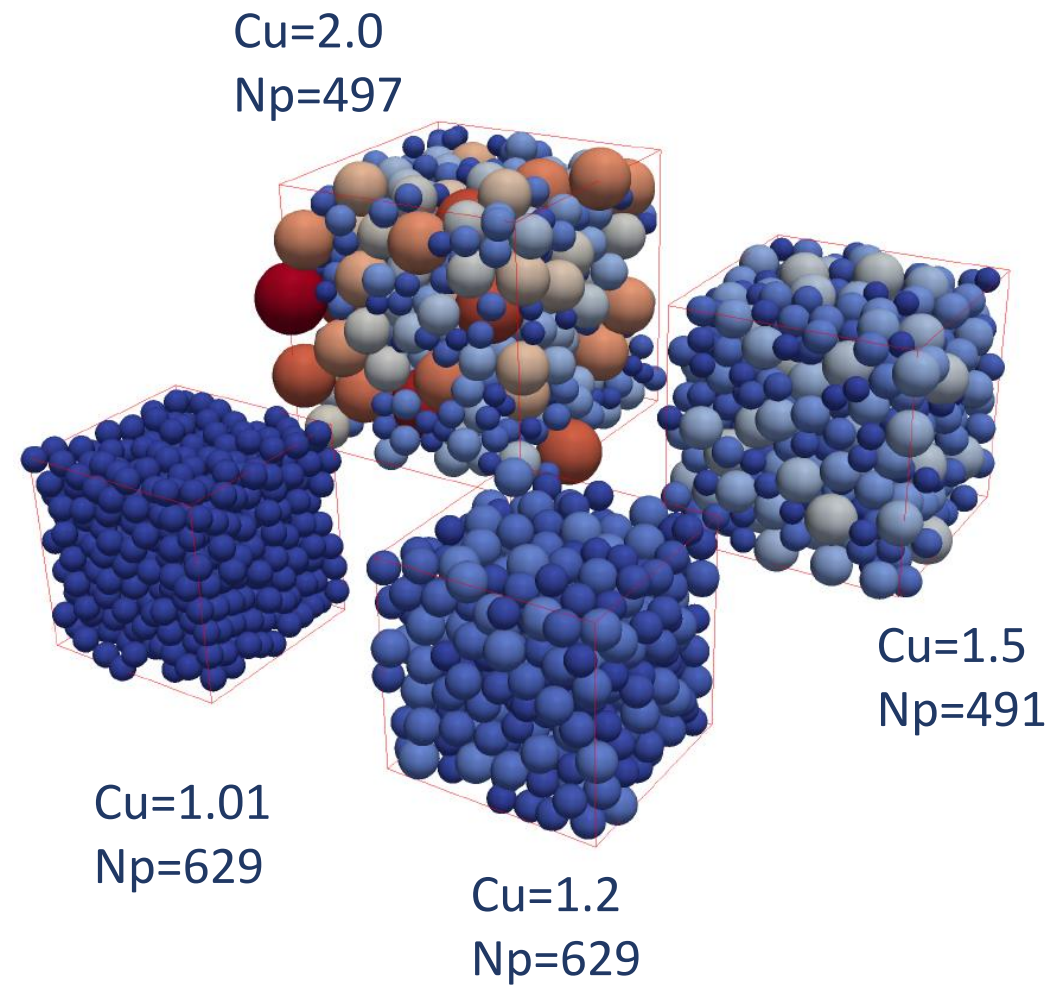
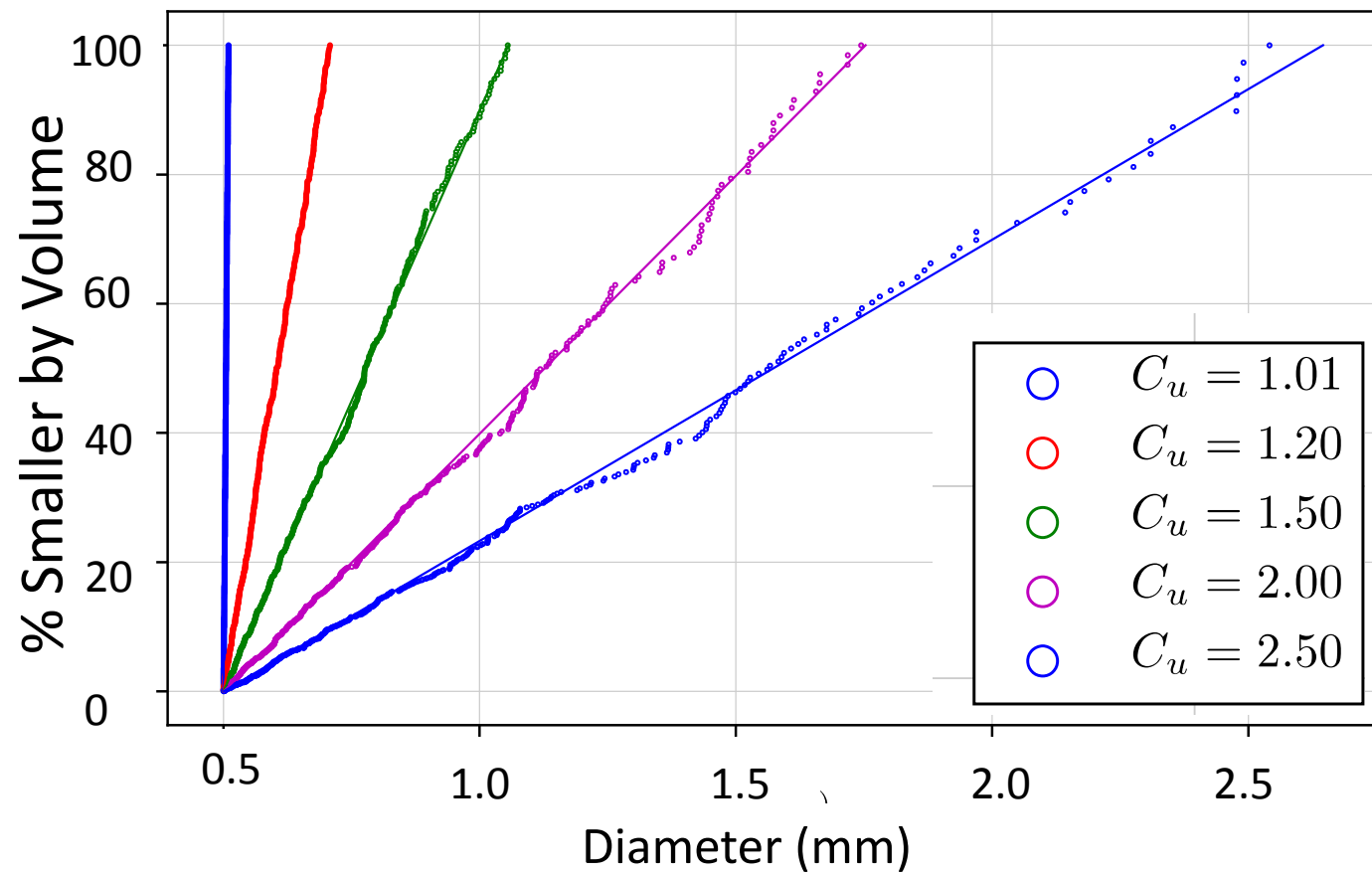
Particles a boundary to fluid flow



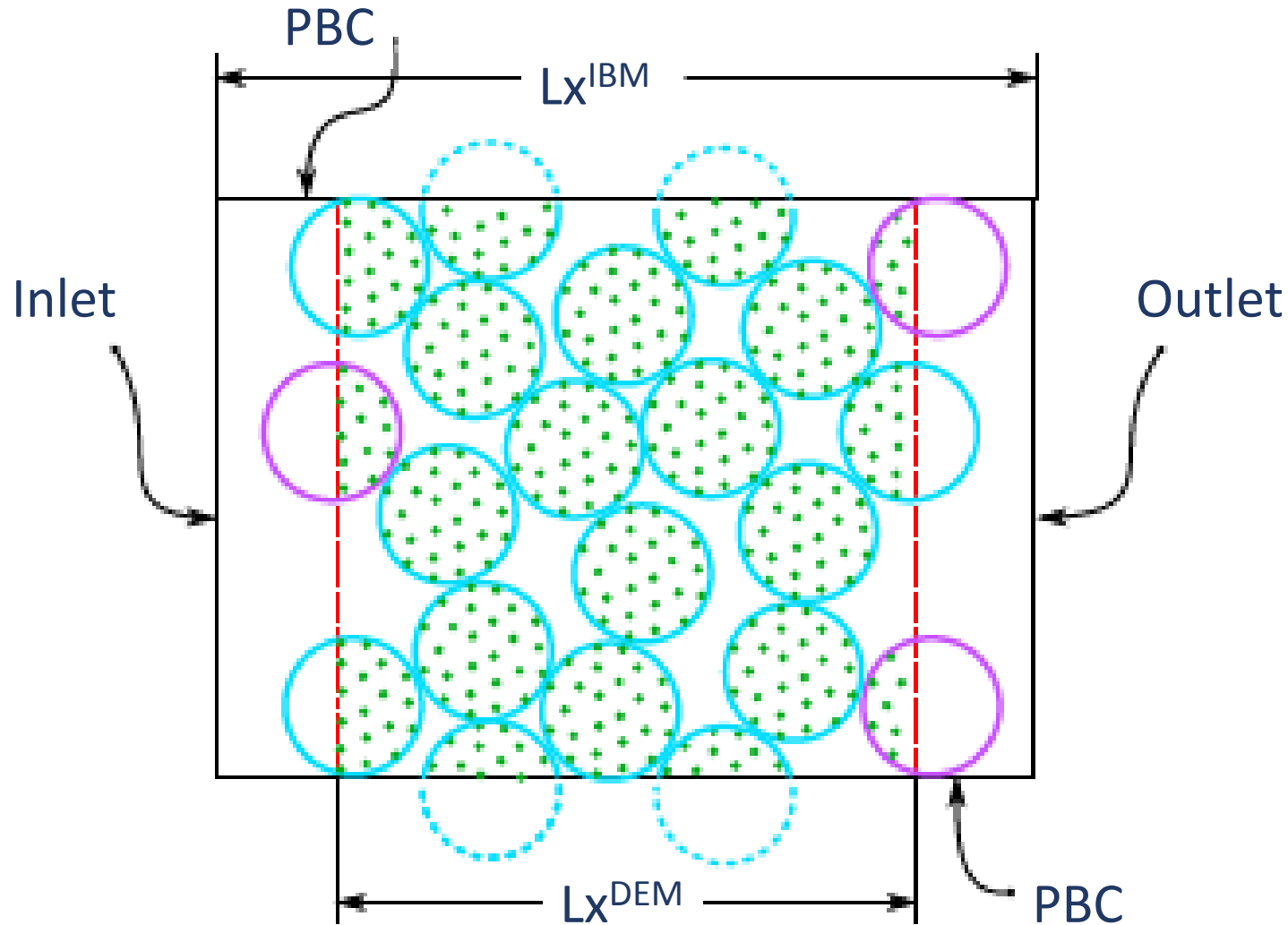
Dense
monodisperse
sample
Low Re



IBM Simulations – linear gradings



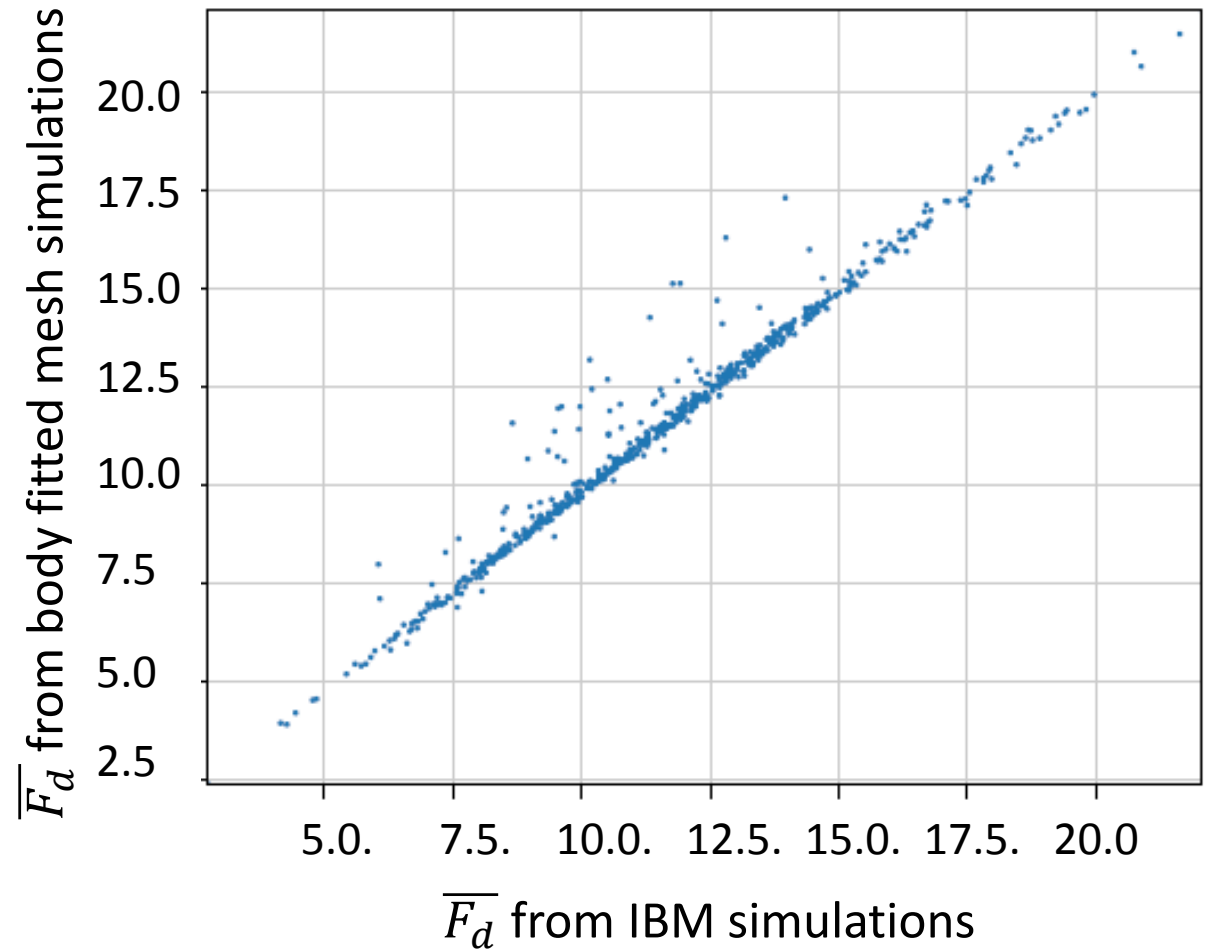
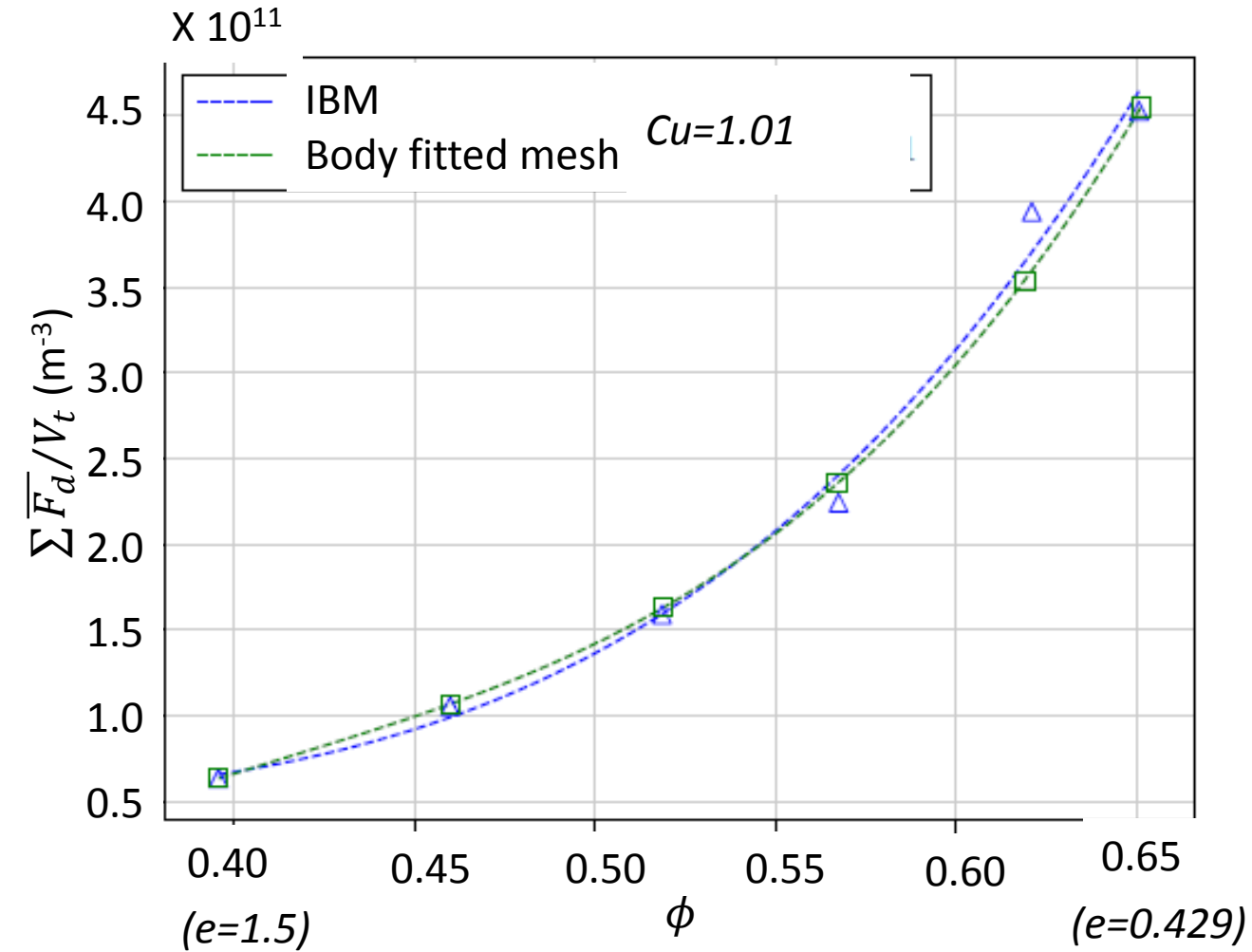
Simulation – configuration



Samples subject to laminar flow

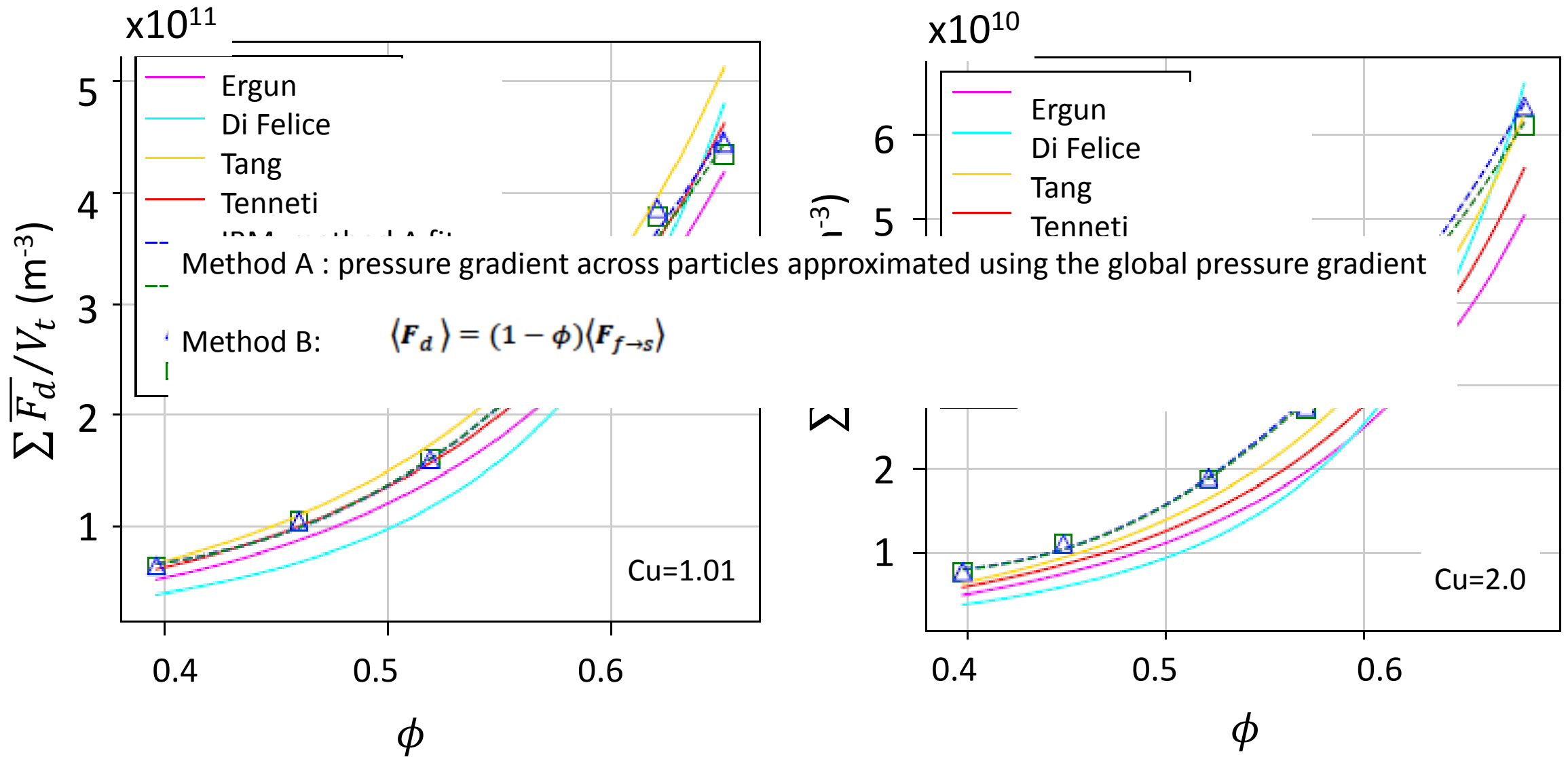
Wide range of packing densities

Variation in drag force with ϕ



\overline{F}_d = drag force normalized by Stokes drag

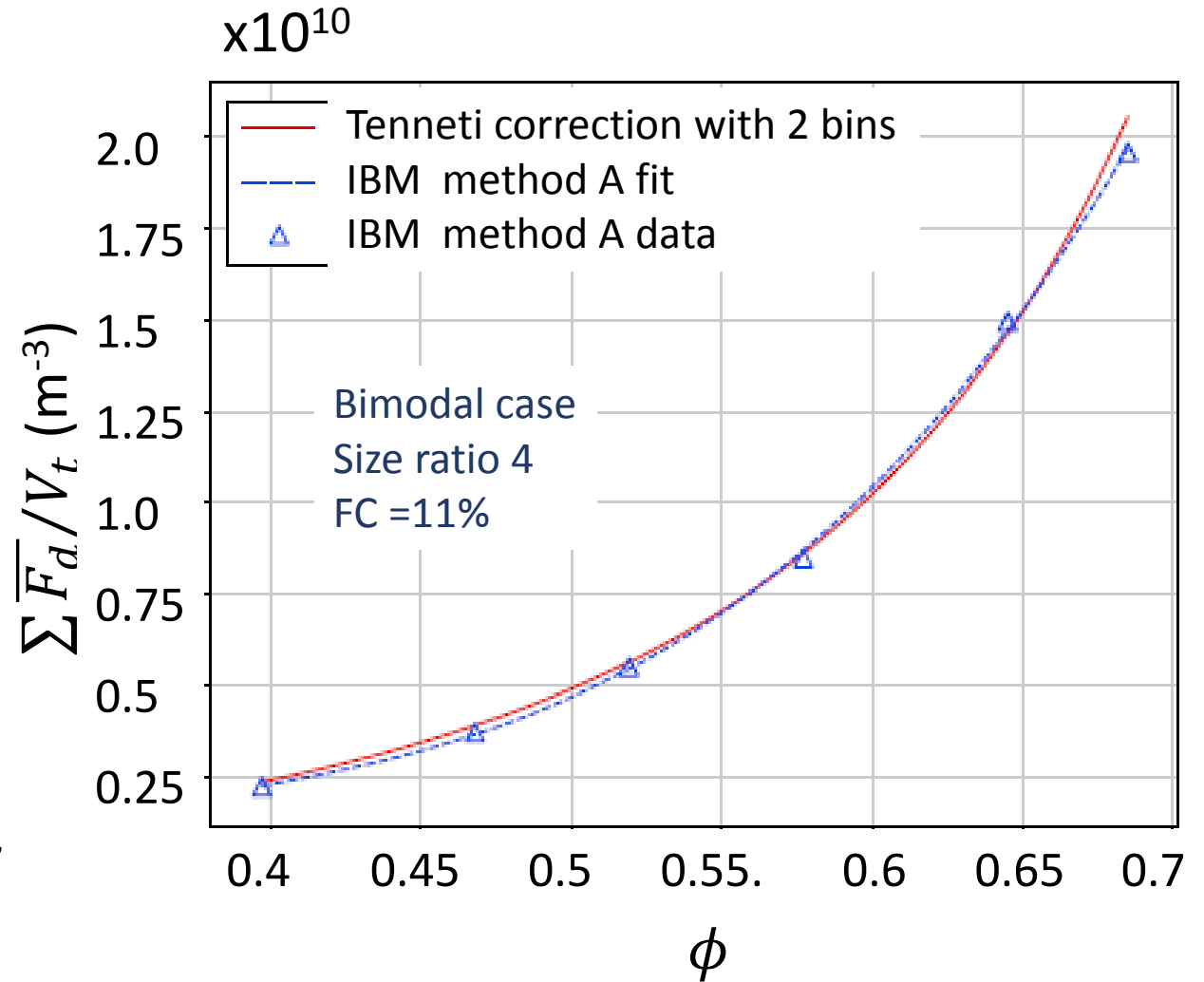
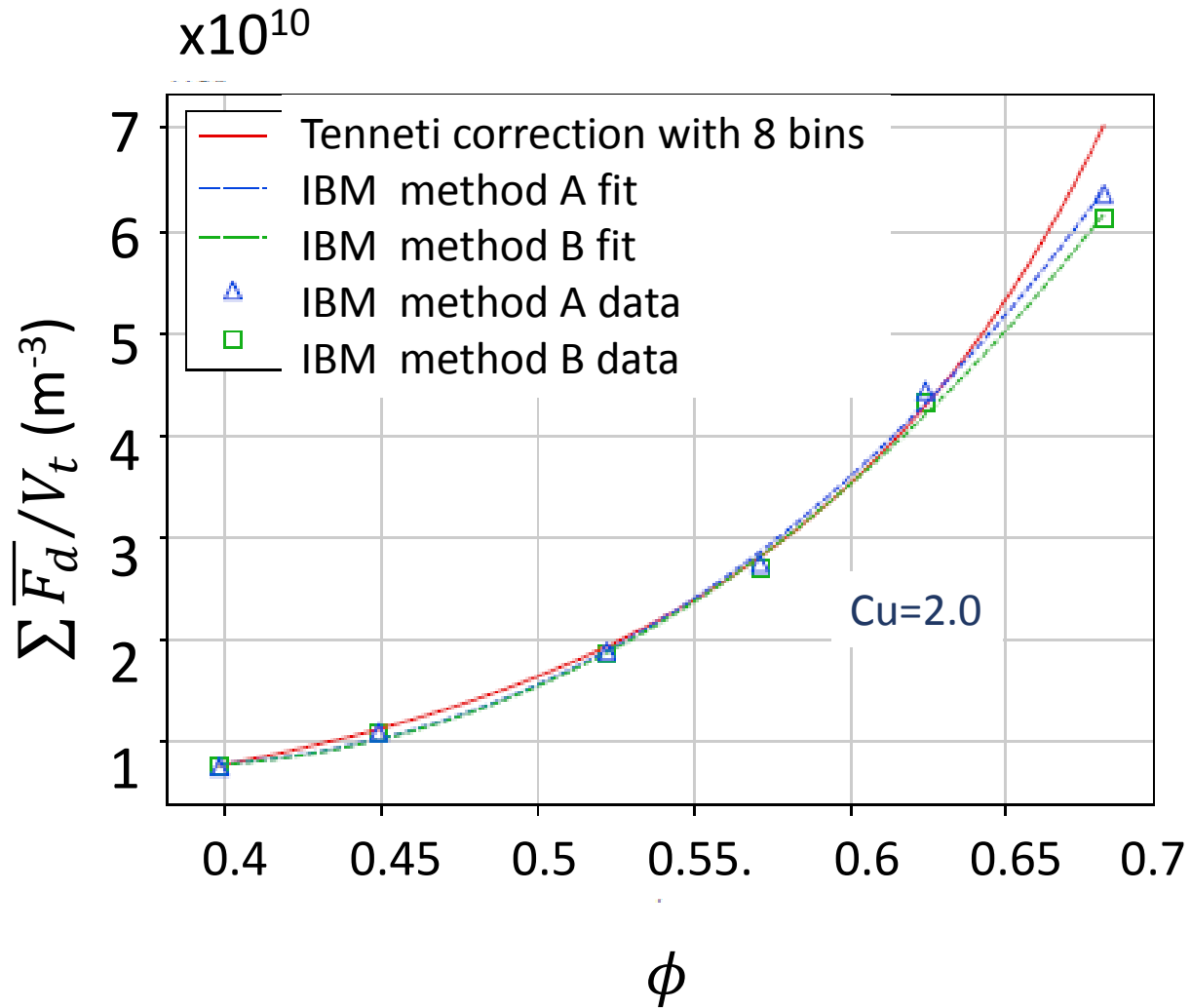
Assessment of semi-empirical expressions



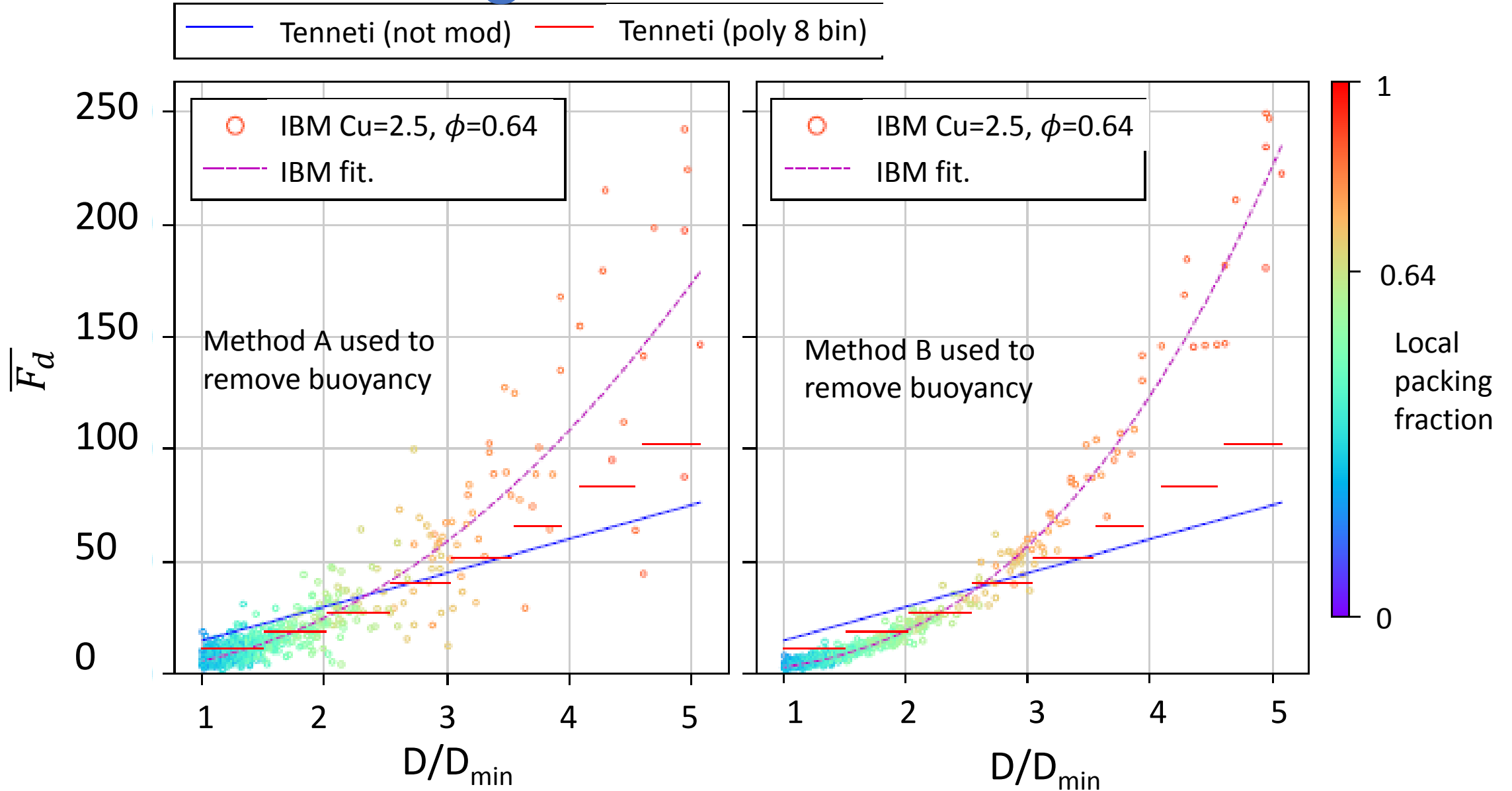
$\overline{F_d}$ = drag force normalized by Stokes drag – total for sample

Method A/B – approach used to extract buoyancy

Assessment of polydispersity correction

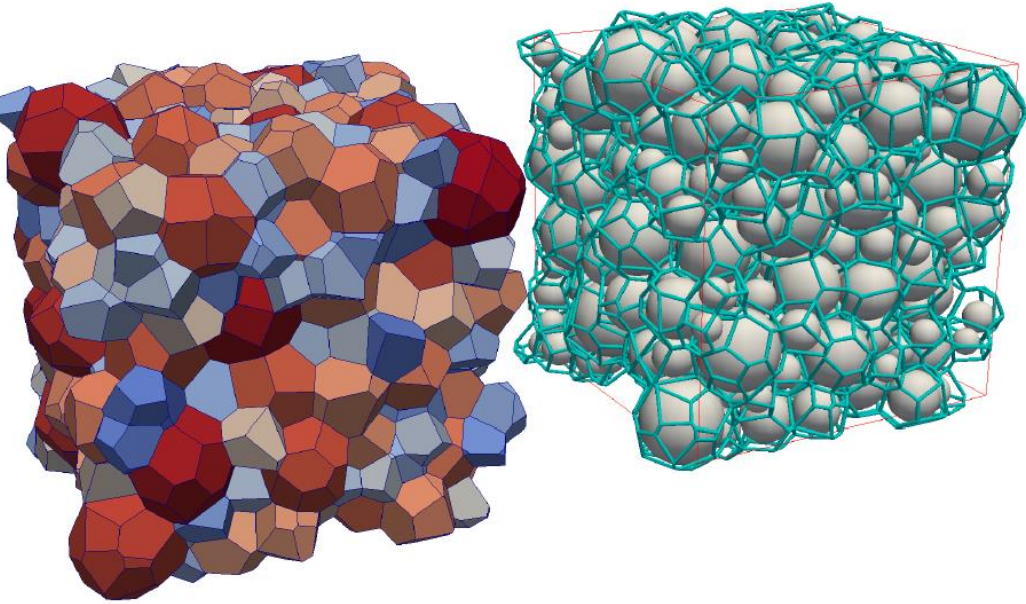


Individual drag forces versus diameter

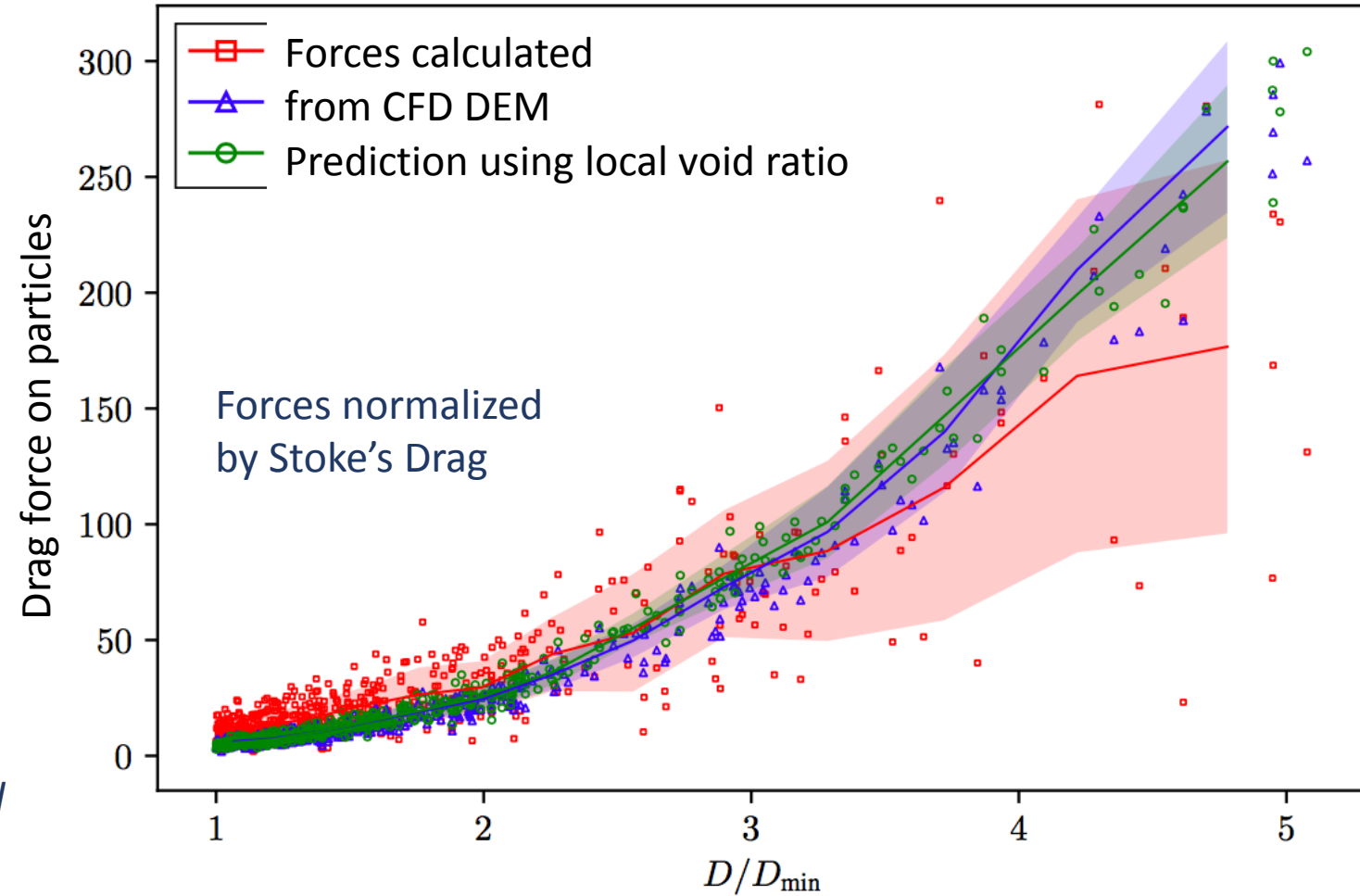


$\overline{F_d}$ = drag force normalized by Stokes drag – total for sample

Fluid particle interaction force : local void ratio

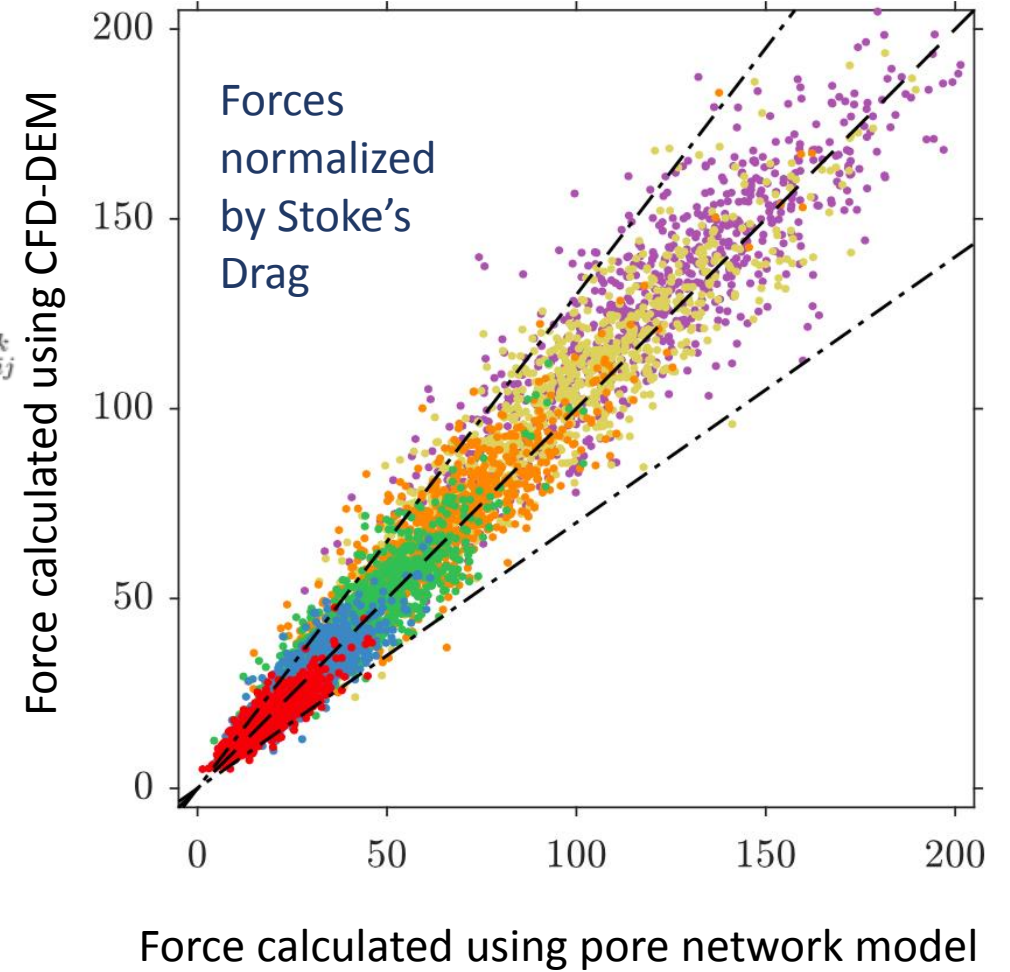
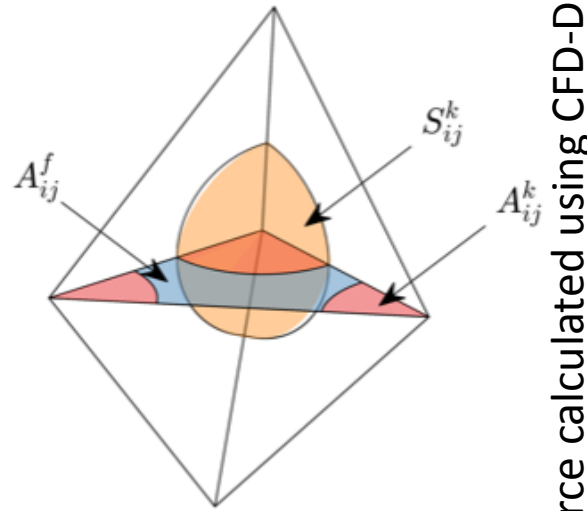
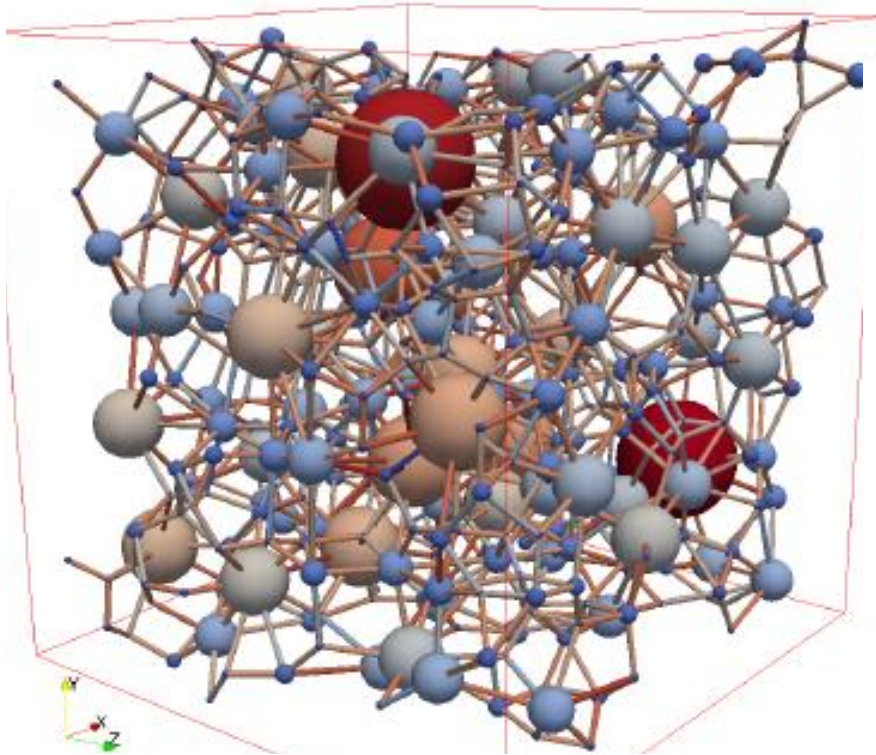


Tenneti et al. polydispersity correction also applied

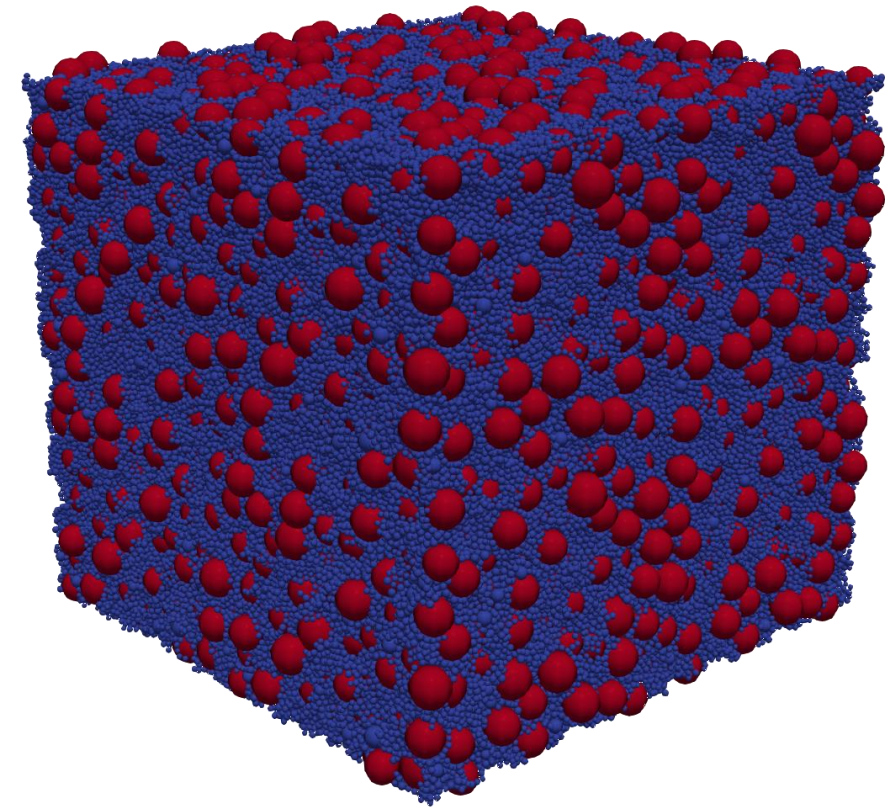


Cu=2.5, Solids fraction 0.701

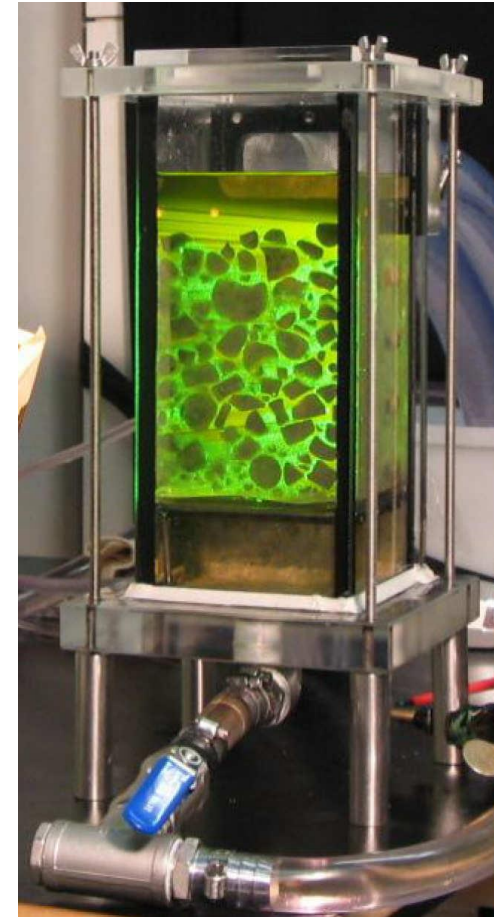
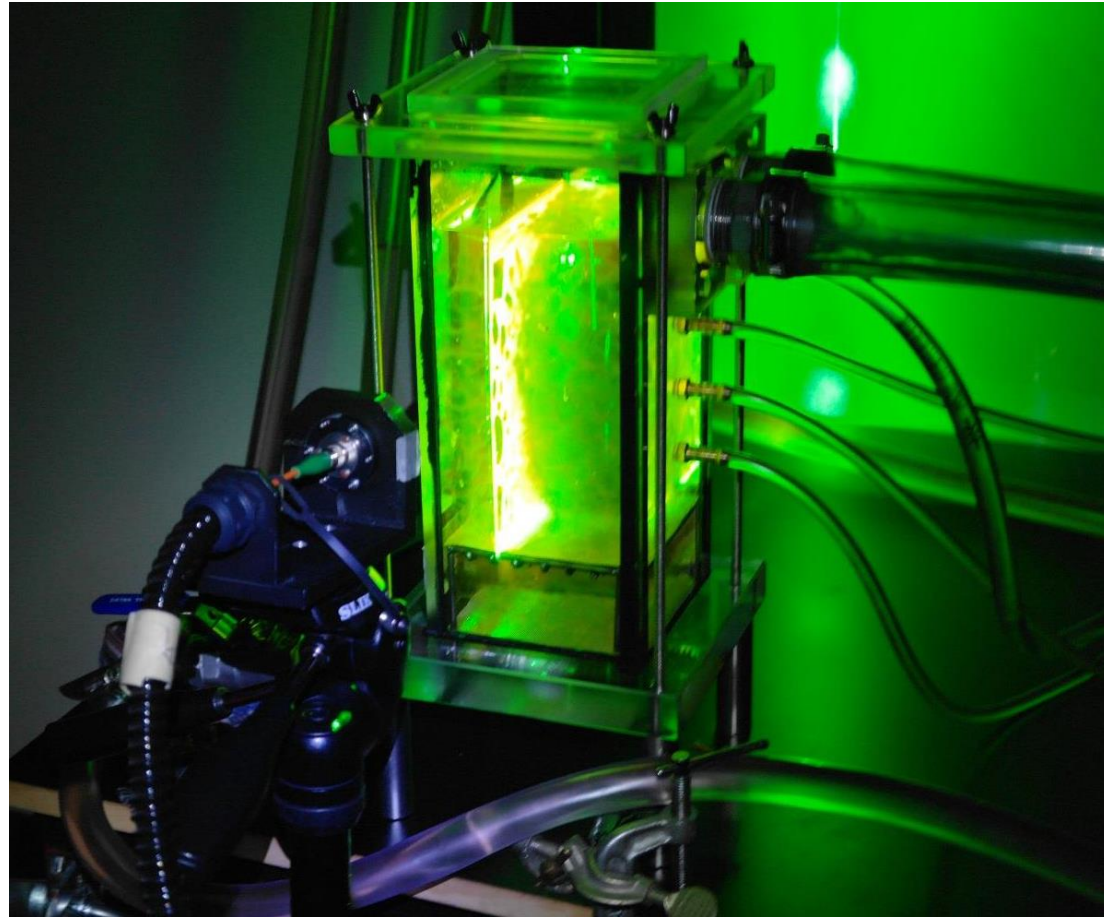
Network based approach to determine forces



Seepage Induced Instability



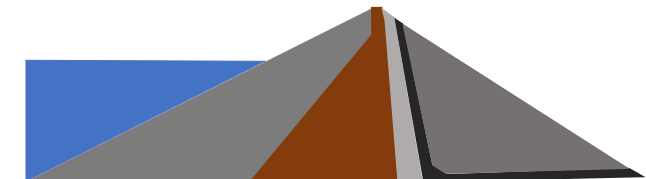
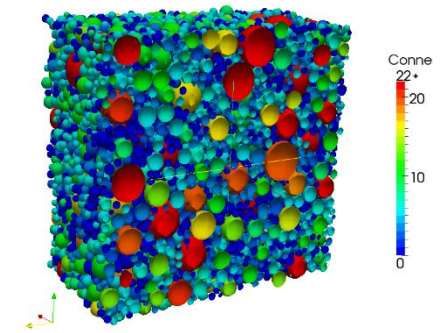
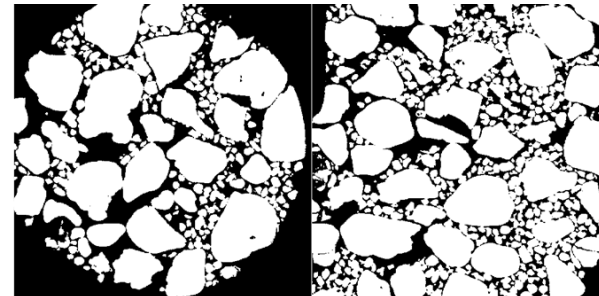
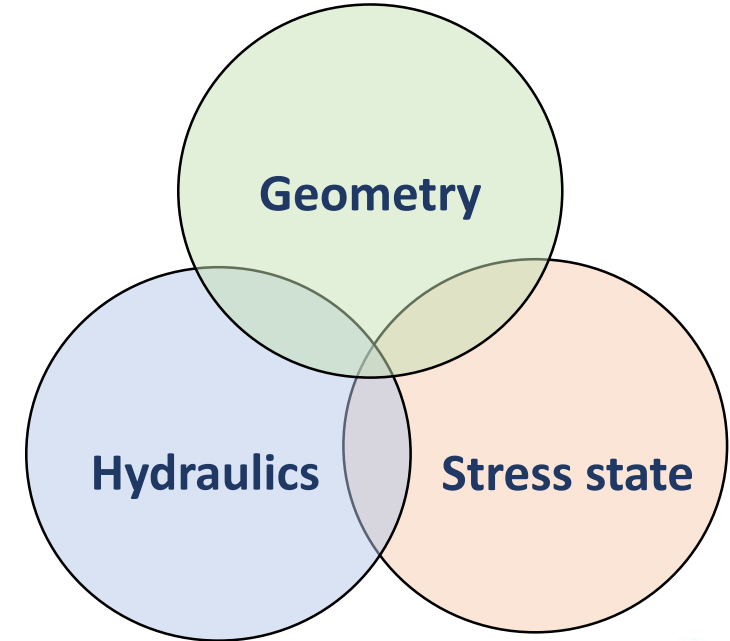
Discrete element method
simulations at Imperial College
London



Transparent soil at the University of Sheffield

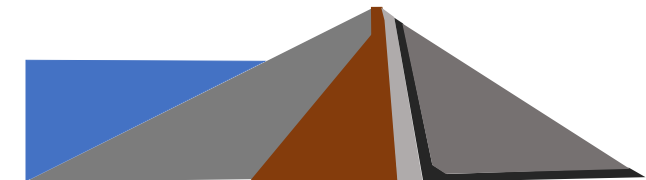
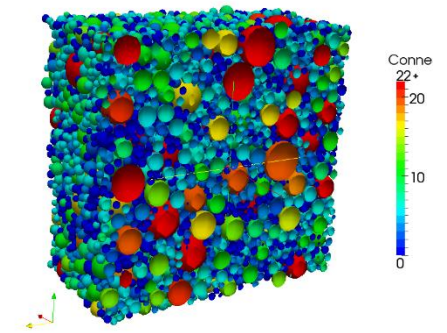
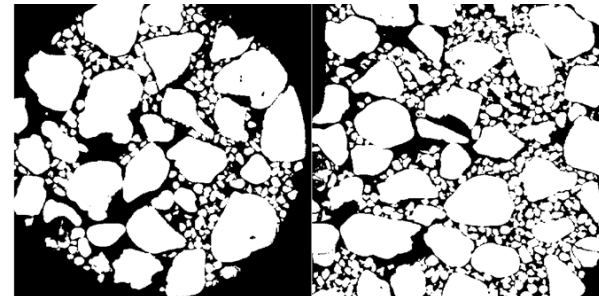
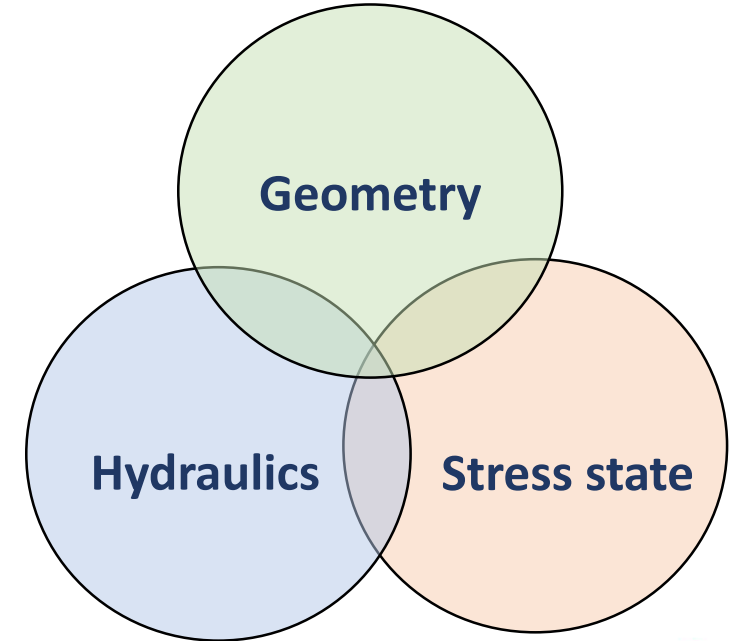
Conclusions

- Considerations of filter compatibility and internal instability are important in dam and embankment design and maintenance
- Geometry / particle scale topology of materials; stress state and fluid:particle interaction determine behaviour
- Particle-scale simulation can improve understanding leading to more robust design guidance

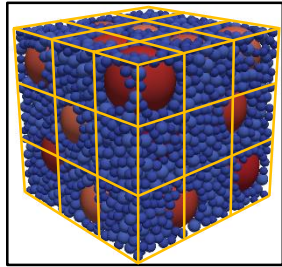


Conclusions

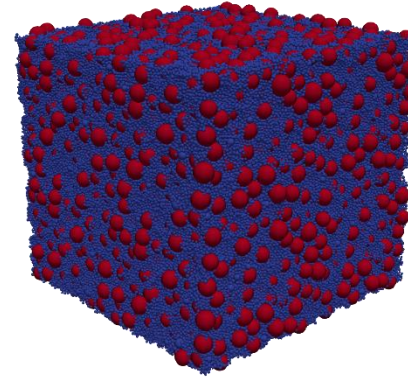
- Simulations with gap-graded materials are challenging – large numbers of particles are needed and low strain rates are required.
- Significant research effort needs to be put into developing accurate drag expressions to enable unresolved DEM-CFD to be used with confidence in geomechanics applications where polydispersity is always an issue.
- Combining network based approaches with DEM datasets can overcome some of the challenges associated with CFD-DEM



Acknowledgements



Kenichi Kawano



Adnan Sufian



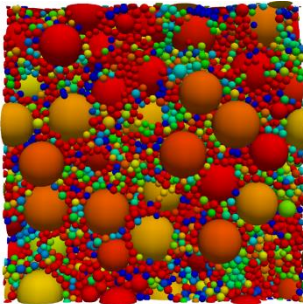
Chris Knight



Berend van Wachem



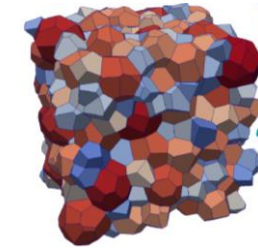
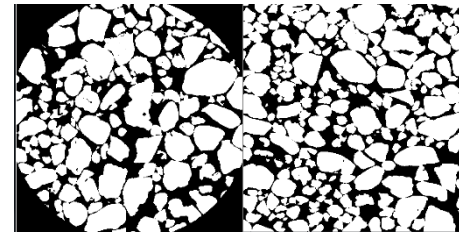
Tom Shire



Kevin Hanley



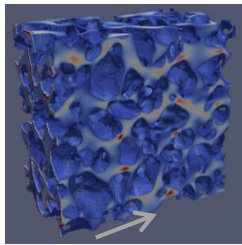
Joana Fonseca



Daniele Dini



Howard Taylor



Way Way Sim

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- Technicians at in Imperial College Soil Mechanics Laboratory
- Engineering and Physical Sciences Research Council
- Dr. Simon Carr, formerly Queen Mary University London

Key references

Ability of a pore network model to predict fluid flow and drag in saturated granular materials Adnan Sufian Christopher Knight; Catherine O'Sullivan; Berend van Wachem; Daniele Dini Computers and Geotechnics,

<https://doi.org/10.1016/j.compgeo.2019.02.007>

Kawano, K., Shire, T., O'Sullivan, C. (2017) Coupled particle-fluid simulations of the initiation of suffusion *Soils and Foundations* <https://doi.org/10.1016/j.sandf.2018.05.008>

Taylor, HF, O'Sullivan, C, Sim, W.W., Carr,S.J. (2017) Sub-Particle-Scale Investigation of Seepage in Sands *Soils and Foundations*, <https://doi.org/10.1016/j.sandf.2017.05.010>

Taylor, H.F., O'Sullivan, C. & Sim, W.W. (2016a) Geometric and Hydraulic Void Constrictions in Granular Filters *ASCE Journal of Geotechnical and Geoenvironmental Engineering*, 142(11) DOI 10.1061/(ASCE)GT.1943-5606.0001547

Shire, T., O'Sullivan,C. and Hanley, K.J. (2016) "The influence of fines content and size-ratio on the micro-scale properties of dense bimodal materials" *Granular Matter* 18(52) doi:10.1007/s10035-016-0654-9

Shire, T., O'Sullivan, C. , Fannin, R.J., Hanley, K. (2014) "Fabric and effective stress distribution in internally unstable soils" *ASCE Journal of Geotechnical and Geoenvironmental Engineering*, 140(12) DOI 10.1061/(ASCE)GT.1943-5606.0001184

Fonseca, J., Sim, W. W., Shire, T., and O'Sullivan, C. (2014) "Micro-structural analysis of sands with varying degrees of internal stability" *Géotechnique* Vol: 64, pp 405-411 doi: 10.1680/geot.13.T.014

HF Taylor, C O'Sullivan, T Shire, WW Moinet (2019) Influence of the coefficient of uniformity on the size and frequency of constrictions in sand filters. *Géotechnique* 69 (3), 274-282

Knight et al. (2019) Computing drag and interactions between fluid and polydisperse particles in saturated granular materials Computers and Geotechnics <https://doi.org/10.1016/j.compgeo.2019.103210>